DESIGN BASIS	
BUILDING CODE	=2018 IBC
RISK CATEGORY	=
ROOF LOADS DEAD LOAD LIVE LOAD	=15 PSF =20 PSF
SNOW LOAD GROUND SNOW LOAD FLAT ROOF SNOW LOAD EXPOSURE FACTOR IMPORTANCE FACTOR THERMAL FACTOR SLOPE FACTOR DRIFT SURCHARGE LOAD WIDTH OF SNOW DRIFT	=78 PSF =55 PSF =1.0 =1.0 =1.0 =1.0 =73 PSF =10 FT
FLOOR LOADS DEAD LOAD LIVE LOAD	=18 PSF =40 PSF
WIND LOAD ULTIMATE WIND SPEED ASD WIND SPEED EXPOSURE INTERNAL PRESSURE COEFFICIENT	=105 MPH =81 MPH =B =+/- 0.18
SEISMIC LOAD IMPORTANCE FACTOR Ss SITE CLASS SDS SD1 SEISMIC DESIGN CATEGORY SEISMIC FORCE-RESISTING SYSTEM DESIGN BASE SHEAR SEISMIC RESPONSE COEFFICIENT (Cs) RESPONSE MODIFICATION FACTOR (R) ANALYSIS PROCEDURE	=1.0 =0.581 =D =0.517 =0.163 =D LIGHT-FRAMED (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE =5.7 KIPS =0.08 =6.5 =EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION

- FOUNDATIONS HAVE BEEN DESIGNED BASED ON PRESUMPTIVE BEARING CAPACITY OF 1500 PSF.
- CONTRACTOR SHALL BE RESPONSIBLE FOR DEWATERING AND PROTECTING ALL EXCAVATION. PERMANENTLY BRACE ALL BASEMENT FOUNDATION WALLS PRIOR TO BACKFILLING.
- DO NO BACKFILL AGAINST CANTILEVER RETAINING WALLS UNTIL CONCRETE HAS REACHED DESIGN STRENGTH. 4.
- FOOTINGS SHALL BE 48" MINIMUM BELOW FINISHED GRADE. UNSUITABLE MATERIAL SHALL BE REMOVED AND REPLACED WITH COMPACTED SOIL.
- FOUNDATIONS SHALL NOT BE PLACED ON FROZEN GROUND OR IN WATER.
- ALL FOOTING EXCAVATIONS SHALL BE FINISHED BY HAND. 9. CONTRACTOR TO COORDINATE ALL FLOOR DRAINAGE AND PLUMBING.
- **REINFORCING**
- ALL CONCRETE SHALL INCLUDE REINFORCEMENT. IF REINFORCING IS NOT SPECIFICALLY INDICATED ON DRAWINGS, CONTACT ENGINEER. REINFORCEMENT SHALL CONFORM TO THE FOLLOWING STANDARDS:

ASTM A706

- MATERIAL ASTM A615 GRADE 60 <u>ITEM</u> DEFORMED BARS
- WELDED WIRE REINFORCEMENT
- WHERE 90, 135, OR 180 DEGREE HOOKS ARE INDICATED ON PLANS, PROVIDE ACI STANDARD HOOK.
- LAP REINFORCEMENT PER LAP SPLICE LENGTH SCHEDULE UNO. PROVIDE NECESSARY ACCESSORIES TO PROPERLY PLACE REINFORCEMENT.
- REINFORCEMENT SHOWN IN DETAILS IS MEANT TO BE TYPICAL UNO.
- 7. DOWELS SHALL MATCH BAR SIZE AND NUMBER OF THE MAIN REINFORCING.

CONCRETE

ALL CONCRETE WORK SHALL COMPLY WITH THE CURRENT VERSION OF ACI 318. 2. CONCRETE COMPRESSIVE STRENGTHS, EXPOSURE CLASSIFICATION AND WEIGHT:

<u>COMPONENT</u>	EXPOSURE CLASS	COMPRESSIVE STRENGTH
FOOTINGS	F0/S0/W0/C1	2500 PSI
FOUNDATION WALLS	F1/S0/W0/C1	3500 PSI
INT. SLAB ON GRADE	F0/S0/W0/C0	2500 PSI
EXT. PIERS/COLUMNS	F1/S0/W0/C1	3500 PSI
INT. SLABS ON METAL DECK	F0/SO/W0/C0	2500 PSI
EXT. SLABS ON METAL DECK	F2/SO/W0/C2	5000 PSI

ALL CONCRETE SHALL BE THOROUGHLY CONSOLIDATED. USE OF CHLORIDE CONTAINING AGENTS AND CALCIUM CHLORIDE IS PROHIBITED. PLACEMENT OF CONCRETE IN CONTACT WITH

- ALUMINUM IS ALSO PROHIBITED.
- CONCRETE SHALL BE NORMAL WEIGHT UNO. ALL EXPOSED EDGES SHALL BE CHAMFERED 3/4" UNO BY ARCHITECT.
- CONCRETE COVER SPECIFIED IN CONCRETE COVER TABLE.
- SUBMIT ENGINEERED CONCRETE MIXES INCLUDING REQUIRED BACKUP DATA FOR EACH TYPE OF CONCRETE TO BE USED FOR ENGINEER REVIEW.
- FORMS SHALL NOT BE STRIPPED UNTIL CONCRETE HAS REACHED DESIGN STRENGTH. 10. CONCRETE COVER PER CONCRETE COVER SCHEDULE

WOOD

- LUMBER AND FASTENERS SHALL CONFORM TO NDS SPECIFICATIONS OF STRESS-GRADE LUMBER AND ITS FASTENING. ALL SHEATHING SHALL BE APA RATED.
- ALL FASTENERS SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS STEEL. SILL PLATES SHALL BE PRESSURE TREATED.
- ALL STRUCTURAL MEMBERS SHALL BE DOUGLAS FIR-LARCH #2 UNO.
- ALL GLULAM MEMBERS SHALL BE DF 24F-V4 UNO. ALL LVL MEMBERS SHALL BE 2.0E OR BETTER UNO.
- ALL PSL HEADERS AND BEAMS SHALL BE 2.0E OR BETTER.
- ALL PSL POSTS AND COLUMNS SHALL BE 1.8E OR BETTER. ALL LSL POSTS AND COLUMNS SHALL BE 1.3E OR BETTER.
- ALL NAILING OF WOOD MEMBERS SHALL BE IN ACCORDANCE WITH TABLE 2304.10.1 OF THE APPLICABLE VERSION OF THE IBC.
- OVERBUILD FRAMING SHALL BE MADE WITH 2X MEMBERS IN ACCORDANCE WITH THE CURRENT VERSION OF THE IBC. POSTS SUPPORTING HEADERS SHALL CONTINUE FROM POINT OF LOAD DOWN TO FOUNDATION.
- FLUSH FRAMING CONNECTIONS SHALL BE MADE WITH HANGER. ALL BEAMS OVER COLUMNS SHALL HAVE METAL POST CAP.
- BOLT HOLES SHALL BE LIMITED TO 1/16" LARGER THAN REQUIRED BOLT DIAMETER. BORED HOLES IN WALL STUDS SHALL BE AT LEAST 5/8" FROM EDGE. 17
- ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE TREATED. 18.
- WOOD STRUCTURAL PANELS SHALL BE INSTALLED WITH 1/8" GAP BETWEEN PANEL EDGES. 19. ALL INTERIOR BEARING WALLS SHALL HAVE AT LEAST ONE MID-HEIGHT ROW OF BLOCKING. UNSHEATHED WALLS SHALL HAVE 20.
- BLOCKING EVERY 4'-0" O.C. ALL NAILS IN NAILING SCHEDULE SHALL BE COMMON. THREADED, HARDENED STEEL NAILS MAY BE SUBSTITUTED FOR COMMON 21. SIZE NAILS OF CORRESPONDING SIZE FOR PLYWOOD. USE ANNULAR-RING, COMMON WIRE, GALVANIZED NAILS FOR PLYWOOD.
- GALVANIZED NAILS SHALL BE HOT-DIP GALVANIZED, ASTM-A153. 22. ALL FASTENERS USED IN PRESSURE TREATED WOOD SHALL BE COATED, TREATED, AND APPROVED FOR USE BY THE
- MANUFACTURER. 23. EDGE DISTANCE FOR NAILS SHALL BE MINIMUM OF 2 TIMES THE WIRE DIAMETER UNLESS UNO ON PLANS.

WOOD JOISTS

NOTES:

- ALL WOOD I-JOISTS SHALL BE STAMPED WITH APA PRI TRADEMARK.
- JOISTS SHALL BE TOE-NAILED TO SUPPORT WITH (2) 10d NAILS.
- JOISTS SHALL HAVE MINIMUM BEARING OF 1-1/2". LAP JOISTS AT BEARING WALL A MINIMUM OF 4" AND NAIL TOGETHER WITH (3) 16d NAILS. 4

BAR COVER	
ITEM	
CAST AGAINST EARTH	3"
SLABS ON GRADE	1 1/2"
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER AT SLABS, WALL, AND JOISTS	3/4"
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER AT BEAMS AND COLUMNS	1 1/2"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER	1 1/2"

LAP SPLICE SCHEDULE					
	F'c=3000 PSI	F'c=3500 PSI	F'c=4000 PSI	F'c=4500 PSI	F'c=5000 PSI
BAR SIZE	TYP/TOP (IN)				
#3	17/22	16/20	15/19	14/18	13/17
#4	22/29	21/27	19/25	18/24	17/23
#5	28/36	26/33	24/31	23/30	22/28
#6	33/43	31/40	29/37	27/35	26/34
#7	48/63	45/58	42/54	40/51	38/49

ALL LAP SPLICES ARE CLASS B 2. HORIZONTAL BARS WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW SHALL BE CONSIDERED "TOP"

		ABBREVIATIONS	
		ACI AN	IERICAN CONCRETE INSTITUTE
		ADDL AL AISC AN	IERICAN INSTITUTE OF STEEL CONSTRUCTION
	ANGERS	ALT AL	TERNATE
		AND AND AL	LOWABLE STRESS DESIGN
FRAMING TYPE	SIMPSON FASTENER	AWS AN BOT BC	IERICAN WELDING SOCIETY)TTOM
SINGL	LE JOIST END HANGER TO WOOD BEAM	BOF BC	
		CIP CA	SARING ST-IN-PLACE
2X4	LUS24	CL CE CLR CL	
		CMU CC	DNCRETE MASONRY UNIT
2X6	LUS26	CONC CC	DNCRETE
2X8	LUS28	CONT CC D&F DF	
		DIA DI	AMETER
2X10	LUS210	DIM DI DN DC	MENSION DWN
2X12	1115212	DWG DF DWLS DC	RAWING DWELS
		(E) EX	(ISTING
DOUB	LE JOIST END HANGER TO WOOD BEAM	EF EA	ICH FACE
		EL EL EL EQ EQ	EVATION QUAL
(2) 2X4	LUS24-2	EF EA	CH FACE
(2) 2X6	LUS26-2	Evv EA (F) FU	ITURE
· · /		FND FC	DUNDATION OOR
(2) 2X8	LUS28-2	FT FE	ET
(2) 2X10		GT GI	RDER TRUSS
(2) 2/10	LUS210-2	HORIZ HO HSS HO	DRIZONTAL DI LOW STRUCTURAL SECTION
(2) 2X12	LUS212-2	IBC IN	TERNATIONAL BUIDLING CODE
			CH P
TRIPL	LE JOIST END HANGER TO WOOD BEAM	LLH LO	
(2) 226		LRFD LC	AD RESISTANCE FACTOR DESIGN
(3) 2/0	LU320-3	LWC LIQ MECH ME	ECHANICAL
(3) 2X8	LUS28-3	MEP ME ME77 ME	ECHANICAL, ELECTRICAL, PLUMBING
(0) 0)(40		(N) NE	EW STAND
(3) 2X10	LUS210-3	NTS NU NWC NC	DI TO SCALE DRMAL WEIGHT CONCRETE
(3) 2X12	1115212-3	OWJ OF	PEN WEB JOIST JI ESS NOTED OTHERWISE
(0) ==		PL PL	
I-J	JOIST END HANGER TO WOOD BEAM	PSF PC PSI PC	DUNDS PER SQUARE FEET DUNDS PER SQUARE INCH
		PT PF REF RE	RESSURE TREATED
ALL SIZES	IUS SERIES	REINF RE	
I	LVL END HANGER TO WOOD BEAM	SIM SIM SI	AB-ON-GRADE
			'EEL IEARWALL
ALL SIZES	HU SERIES	T&B TC	
RA	NETER END HANGER TO WOOD BEAM	TOS TO	P OF STEEL
		TOF TC	P OF FOOTING P OF WALL
2X6	LRU26Z	TYP TY	PICAL
020	101007	VERT VE	IRTICAL
288	LRU28Z	VIF VE W/ WI	:RIFY IN FIELD TH
2X10	LRU210Z	WWF WE	ELDED WIRE FABRIC
2X12	LRU212Z		
RAFTER END H	IANGER TO WOOD BEAM - SLOPED AND SKEWED		
		SHEET	
2X6	LSU26	NUMBER	
27.6	1 201 109	<u> </u>	GENERAL NOTES
۲۸۵	L00U20	<u>S1 0</u>	
2X10	LSSU210	S1 1	
			FRAMINGPLAN
2X12	LSSU210	S2.0	CONCRETE DETAILS I
		S3.0	WOOD DETAILS I
OTES: JOIST HANGER NAILIN	NG SHALL BE IN ACCORDANCE WITH SIMPSON'S	L	
REQUIREMENTS. WHE	ERE A MINIMUM AND MAXIMUM NAILING PATTERN ARE G	N,	
I HE MAXIMUM SHALL INSTALLATION PER M	. BE USED. IANUFACTURER'S REQUIREMENTS.		
SIMPSON ZMAX FINIS	H REQUIRED FOR EXTERIOR APPLICATIONS.		LEGEND

AMERICAN CONCRETE INSTITUTE
ADDITIONAL
AMERICAN INSTITUTE OF STEEL CONSTRUCT
ALTERNATE
ARCHITECT
ALLOWABLE STRESS DESIGN
AMERICAN WELDING SOCIETY
BOTTOM
BOTTOM OF FOOTING
BEARING
CAST-IN-PLACE
CENTERLINE
DIAMETER
DIMENSION
DOWN
DRAWING
DOWELS
EXISTING
EACH
EACH FACE
ELEVATION
EQUAL
EACH FACE
EACH WAY
FUTURE
FOUNDATION
FLOOR
FEET
FOOTING
KID
I ONG LEG HORIZONTAL
LOAD RESISTANCE FACTOR DESIGN
LIGHT WEIGHT CONCRETE
MECHANICAL
MECHANICAL, ELECTRICAL, PLUMBING
MEZZANINE
NEW
NOT TO SCALE
NORMAL WEIGHT CONCRETE
OPEN WEB JOIST
UNLESS NOTED OTHERWISE
PLATE
POUNDS PER SQUARE FEET
POUNDS PER SQUARE INCH
PRESSURE TREATED
REFERENCE
REINFORCING
SLAB-UN-GRADE
TOP OF WALL
TYPICAI
UNLESS NOTED OTHERWISE
VERTICAL
VERIFY IN FIELD
WITH
WELDED WIRE FABRIC
-



HAZE ENGIN Salt La PH: 80 hazelwo CLIENT SOLSTICE DES	Ake City, UT 1-810-5061 od-eng.com
Revi Code C 09/2	ewed for Compliance
MILNER MICROHOUSING	LOT 15 AND LOT 16, MAIN ST, STEAMBOAT SPRINGS, CO
BBC ST	ADO L/CCA NAZEJ 8/20/2023 62108 CONAL ENGINE VONAL ENGINE VONAL ENGINE TATUS DESCRIPTION PERMIT
PROJECT NUMBER DATE DRAWN BY APPROVED BY	23-047 5/5/2023 RRH RRH

DESIGN BASIS	
BUILDING CODE	=2018 IBC
RISK CATEGORY	=
ROOF LOADS	
DEAD LOAD	=15 PSF
LIVE LOAD	-20 PSP
SNOW LOAD	
GROUND SNOW LOAD	=78 PSF
FLAT ROOF SNOW LOAD	=55 PSF
EXPOSURE FACTOR	=1.0
IMPORTANCE FACTOR	=1.0
THERMAL FACTOR	=1.0
SLOPE FACTOR	=1.0
DRIFT SURCHARGE LOAD	=73 PSF
WIDTH OF SNOW DRIFT	=10 F I
FLOOR LOADS	
DEAD LOAD	=18 PSF
LIVE LOAD	=40 PSF
UI TIMATE WIND SPEED	=105 MPH
ASD WIND SPEED	=81 MPH
EXPOSURE	=B
INTERNAL PRESSURE COEFFICIENT	=+/- 0.18
SEISMIC LOAD	
IMPORTANCE FACTOR	=1.0
Ss	=0.581
SITE CLASS	=D
S _{DS}	=0.517
S _{D1}	=0.163
SEISMIC DESIGN CATEGORY	=D
SEISMIC FORCE-RESISTING SYSTEM	LIGHT-FRAMED (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
DESIGN BASE SHEAR	=5.7 KIPS
SEISMIC RESPONSE COEFFICIENT (Cs)	=0.08
RESPONSE MODIFICATION FACTOR (R)	=6.5
ANALYSIS PROCEDURE	=EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION

- FOUNDATIONS HAVE BEEN DESIGNED BASED ON PRESUMPTIVE BEARING CAPACITY OF 1500 PSF. CONTRACTOR SHALL BE RESPONSIBLE FOR DEWATERING AND PROTECTING ALL EXCAVATION.
- PERMANENTLY BRACE ALL BASEMENT FOUNDATION WALLS PRIOR TO BACKFILLING.
- DO NO BACKFILL AGAINST CANTILEVER RETAINING WALLS UNTIL CONCRETE HAS REACHED DESIGN STRENGTH. 4
- FOOTINGS SHALL BE 48" MINIMUM BELOW FINISHED GRADE. UNSUITABLE MATERIAL SHALL BE REMOVED AND REPLACED WITH COMPACTED SOIL.
- FOUNDATIONS SHALL NOT BE PLACED ON FROZEN GROUND OR IN WATER.
- ALL FOOTING EXCAVATIONS SHALL BE FINISHED BY HAND. 9. CONTRACTOR TO COORDINATE ALL FLOOR DRAINAGE AND PLUMBING.
- REINFORCING
- ALL CONCRETE SHALL INCLUDE REINFORCEMENT. IF REINFORCING IS NOT SPECIFICALLY INDICATED ON DRAWINGS, CONTACT ENGINEER. REINFORCEMENT SHALL CONFORM TO THE FOLLOWING STANDARDS:

ASTM A706

- MATERIAL ASTM A615 GRADE 60 ITEM DEFORMED BARS
- WELDED WIRE REINFORCEMENT
- WHERE 90, 135, OR 180 DEGREE HOOKS ARE INDICATED ON PLANS, PROVIDE ACI STANDARD HOOK.
- LAP REINFORCEMENT PER LAP SPLICE LENGTH SCHEDULE UNO. PROVIDE NECESSARY ACCESSORIES TO PROPERLY PLACE REINFORCEMENT.
- REINFORCEMENT SHOWN IN DETAILS IS MEANT TO BE TYPICAL UNO. DOWELS SHALL MATCH BAR SIZE AND NUMBER OF THE MAIN REINFORCING. 7.
- <u>CONCRETE</u>
- ALL CONCRETE WORK SHALL COMPLY WITH THE CURRENT VERSION OF ACI 318. CONCRETE COMPRESSIVE STRENGTHS. EXPOSURE CLASSIFICATION AND WEIGHT:

COMPONENT	EXPOSURE CLASS	COMPRESSIVE STRENGTH
FOOTINGS	F0/S0/W0/C1	2500 PSI
FOUNDATION WALLS	F1/S0/W0/C1	3500 PSI
INT. SLAB ON GRADE	F0/S0/W0/C0	2500 PSI
EXT. PIERS/COLUMNS	F1/S0/W0/C1	3500 PSI
INT. SLABS ON METAL DECK	F0/SO/W0/C0	2500 PSI
EXT. SLABS ON METAL DECK	F2/SO/W0/C2	5000 PSI

ALL CONCRETE SHALL BE THOROUGHLY CONSOLIDATED. USE OF CHLORIDE CONTAINING AGENTS AND CALCIUM CHLORIDE IS PROHIBITED. PLACEMENT OF CONCRETE IN CONTACT WITH

- ALUMINUM IS ALSO PROHIBITED.
- CONCRETE SHALL BE NORMAL WEIGHT UNO.
- ALL EXPOSED EDGES SHALL BE CHAMFERED 3/4" UNO BY ARCHITECT. CONCRETE COVER SPECIFIED IN CONCRETE COVER TABLE.
- SUBMIT ENGINEERED CONCRETE MIXES INCLUDING REQUIRED BACKUP DATA FOR EACH TYPE OF CONCRETE TO BE USED FOR ENGINEER REVIEW.
- FORMS SHALL NOT BE STRIPPED UNTIL CONCRETE HAS REACHED DESIGN STRENGTH.
- CONCRETE COVER PER CONCRETE COVER SCHEDULE 10.

WOOD

- LUMBER AND FASTENERS SHALL CONFORM TO NDS SPECIFICATIONS OF STRESS-GRADE LUMBER AND ITS FASTENING.
- ALL SHEATHING SHALL BE APA RATED. ALL FASTENERS SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS STEEL. SILL PLATES SHALL BE PRESSURE TREATED.
- ALL STRUCTURAL MEMBERS SHALL BE DOUGLAS FIR-LARCH #2 UNO.
- ALL GLULAM MEMBERS SHALL BE DF 24F-V4 UNO. ALL LVL MEMBERS SHALL BE 2.0E OR BETTER UNO.
- ALL PSL HEADERS AND BEAMS SHALL BE 2.0E OR BETTER. ALL PSL POSTS AND COLUMNS SHALL BE 1.8E OR BETTER.
- ALL LSL POSTS AND COLUMNS SHALL BE 1.3E OR BETTER.
- ALL NAILING OF WOOD MEMBERS SHALL BE IN ACCORDANCE WITH TABLE 2304.10.1 OF THE APPLICABLE VERSION OF THE IBC. OVERBUILD FRAMING SHALL BE MADE WITH 2X MEMBERS IN ACCORDANCE WITH THE CURRENT VERSION OF THE IBC.
- POSTS SUPPORTING HEADERS SHALL CONTINUE FROM POINT OF LOAD DOWN TO FOUNDATION. FLUSH FRAMING CONNECTIONS SHALL BE MADE WITH HANGER.
- ALL BEAMS OVER COLUMNS SHALL HAVE METAL POST CAP. BOLT HOLES SHALL BE LIMITED TO 1/16" LARGER THAN REQUIRED BOLT DIAMETER.
- BORED HOLES IN WALL STUDS SHALL BE AT LEAST 5/8" FROM EDGE.
- ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE TREATED. 18 WOOD STRUCTURAL PANELS SHALL BE INSTALLED WITH 1/8" GAP BETWEEN PANEL EDGES.
- ALL INTERIOR BEARING WALLS SHALL HAVE AT LEAST ONE MID-HEIGHT ROW OF BLOCKING. UNSHEATHED WALLS SHALL HAVE 20.
- BLOCKING EVERY 4'-0" O.C. ALL NAILS IN NAILING SCHEDULE SHALL BE COMMON. THREADED, HARDENED STEEL NAILS MAY BE SUBSTITUTED FOR COMMON 21. SIZE NAILS OF CORRESPONDING SIZE FOR PLYWOOD. USE ANNULAR-RING, COMMON WIRE, GALVANIZED NAILS FOR PLYWOOD.
- GALVANIZED NAILS SHALL BE HOT-DIP GALVANIZED, ASTM-A153. 22. ALL FASTENERS USED IN PRESSURE TREATED WOOD SHALL BE COATED, TREATED, AND APPROVED FOR USE BY THE
- MANUFACTURER. 23. EDGE DISTANCE FOR NAILS SHALL BE MINIMUM OF 2 TIMES THE WIRE DIAMETER UNLESS UNO ON PLANS.

WOOD JOISTS

- ALL WOOD I-JOISTS SHALL BE STAMPED WITH APA PRI TRADEMARK.
- JOISTS SHALL BE TOE-NAILED TO SUPPORT WITH (2) 10d NAILS.
- JOISTS SHALL HAVE MINIMUM BEARING OF 1-1/2". LAP JOISTS AT BEARING WALL A MINIMUM OF 4" AND NAIL TOGETHER WITH (3) 16d NAILS.

BAR COVER	
ITEM	
CAST AGAINST EARTH	3"
SLABS ON GRADE	1 1/2"
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER AT SLABS, WALL, AND JOISTS	3/4"
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER AT BEAMS AND COLUMNS	1 1/2"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER	1 1/2"

LAP SPLICE SCHEDULE			
	F'c=3000 PSI	F'c=3500 PSI	
BAR SIZE	TYP/TOP (IN)	TYP/TOP (IN)	
#3	17/22	16/20	
#4	22/29	21/27	
#5	28/36	26/33	
#6	33/43	31/40	
#7	48/63	45/58	

ALL LAP SPLICES ARE CLASS B HORIZONTAL BARS WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW SHALL BE CONSIDERED "TOP"

SHEAR WALL SCHEDULE			
TYPE	SHEATHING	EDGE NAILING	FIELD NA
SW1	7/16" APA RATED OSB	8d@6" O.C.	8d@12" O
SW2	7/16" APA RATED OSB	8d@4" O.C.	8d@12" O
SW3	7/16" APA RATED OSB	8d@3" O.C.	8d@12" O
SW4	7/16" APA RATED OSB	8d@2" O.C.	8d@12" O
NOTES	1		1

ALL STUDS ARE DFL #2 OR BETTER

NOTES:

USE 8d COMMON WIRE NAILS. USE HOT-DIPPED GALVANIZED NAILS WHEN NAILING TO PRESSURE TREATED LUMBER

PROVIDE END POST AT EACH END OF SHEAR WALL. ANCHORS TO BE 1/2" DIA. J-BOLT W/ 7" MIN EMBED UNO.

40/51

F'c=4000 PSI F'c=4500 PSI F'c=5000 PSI TYP/TOP (IN) TYP/TOP (IN) TYP/TOP (IN) 15/19

14/18 13/17 18/24 17/23 23/30 22/28 27/35 26/34

38/49

19/25

24/31

29/37

42/54

BOTTOM PLATE STUD/BLOCKING AT SILL OR BOTTOM MAX 1/2" ANCHOR BOLT AILING MAX 5/8" ANCHOR JOINTS BOLT SPACING NAILING PLATE SPACING 16d@6" O.C. (2) 2x (2) 2x 32" O.C. 32" O.C. 16d@4" O.C. 24" O.C. 32" O.C. (2) 2x (2) 2x 16d@4" O.C. (2) 2x OR 3x (2) 2x OR 3x 16" O.C. 24" O.C. 1/4"x6" SDS@6" O.C. 12" O.C. 16" O.C. (2) 2x OR 3x (2) 2x OR 3x

(2) 2X4 (2) 2X6 (2) 2X8 (2) 2X10 (2) 2X12 TRIPLE JOIST E (3) 2X6 (3) 2X8 (3) 2X10 (3) 2X12 I-JOIST END ALL SIZES LVL END H ALL SIZES RAFTER END 2X6 2X8 2X10 2X12 RAFTER END HANGER T 2X6

2X8 2X10 2X12

NOTES: JOIST HANGER NAILING SHALL BE IN ACCORDANCE WITH SIMPSON'S 1. REQUIREMENTS. WHERE A MINIMUM AND MAXIMUM NAILING PATTERN ARE GIVEN, THE MAXIMUM SHALL BE USED. INSTALLATION PER MANUFACTURER'S REQUIREMENTS. SIMPSON ZMAX FINISH REQUIRED FOR EXTERIOR APPLICATIONS.

LSSU210

RAMING TYPE	SIMPSON FASTENER
SINGL	E JOIST END HANGER TO WOOD BEAM
2X4	LUS24
2X6	LUS26
2X8	LUS28
2X10	LUS210
2X12	LUS212
DOUBL	E JOIST END HANGER TO WOOD BEAM
(2) 2X4	LUS24-2
(2) 2X6	LUS26-2
(2) 2X8	LUS28-2
(2) 2X10	LUS210-2
(2) 2X12	LUS212-2
TRIPL	L JOIST END HANGER TO WOOD BEAM
(3) 2X6	LUS26-3
(3) 2X8	LUS28-3
(3) 2X10	LUS210-3
(3) 2X12	LUS212-3
I-JI	DIST END HANGER TO WOOD BEAM
ALL SIZES	IUS SERIES
L	VL END HANGER TO WOOD BEAM
ALL SIZES	HU SERIES
RA	TER END HANGER TO WOOD BEAM
2X6	LRU26Z
2X8	LRU28Z
2X10	LRU210Z
2X12	LRU212Z
RAFTER END H	ANGER TO WOOD BEAM - SLOPED AND SKEWED
2X6	LSU26
2X8	LSSU28
2X10	LSSU210

ABBREVIATIONS	
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ARCH	ARCHITECT
ASD	ALLOWABLE STRESS DESIGN
AWS	AMERICAN WELDING SOCIETY
BOT	BOTTOM
BOF	BOTTOM OF FOOTING
BRG	BEARING
CIP	CAST-IN-PLACE
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONT	CONTINUOUS
D&E	DRILL AND EPOXY
DIA	DIAMETER
DIM	DIMENSION
DN	DOWN
DWG	DRAWING
DWLS	DOWELS
(E)	EXISTING
EA	EACH
EF	EACH FACE
EL	ELEVATION
EQ	EQUAL
EF	EACH FACE
EW	EACH WAY
(F)	FUIURE
FND	FUUNDATION
FLR	FLOOR
	FEEI
FIG	
IN	
lin K	
	LONG LEG VERTICAL
LWC	LIGHT WEIGHT CONCRETE
MECH	MECHANICAL
MED	
ME77	MEZZANINE
(N)	NEW
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
OWJ	OPEN WEB JOIST
UNO	UNLESS NOTED OTHERWISE
PL	PLATE
PSF	POUNDS PER SQUARE FEET
PSI	POUNDS PER SQUARE INCH
PT	PRESSURE TREATED
REF	REFERENCE
REINF	REINFORCING
SIM	SIMILAR
SOG	SLAB-ON-GRADE
STL	STEEL
SW	SHEARWALL
T&B	TOP AND BOTTOM
TOC	TOP OF CONCRETE
TOS	TOP OF STEEL
TOF	TOP OF FOOTING
TOW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	
VVVVF	

ADDL

AISC

COL

CONT

DRAWING LIST		
SHEET NUMBER	DRAWING NAME	
S0.0	GENERAL NOTES	
S0.1	GENERAL NOTES	
S1.0	FRAMING PLAN	
S1.1	FOUNDATION AND ROOF FRAMINGPLAN	
S2.0	CONCRETE DETAILS I	
S3.0	WOOD DETAILS I	

















FOUNDATION WALL SCHEDULE					
NAME	MAX HEIGHT	WALL THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING	COMMENTS
FW3	3' - 0"	8"	#4 @ 18" O.C.	#4 @ 18" O.C.	

SPOT FOOTING SCHEDULE							
	DIMENSIONS		REINFORCEMENT OPTIONS		REINFORCEMENT		
TYPE	THICKNESS	LENGTH	WIDTH	<u>#4 EW</u>	<u>#5 EW</u>	LOCATION	COMMENTS
SF2.0	10"	2' - 0"	2' - 0"	(3) #4	(3) #5	BOT	

PLAN NOTES:

- SEE S0 SERIES FOR GENERAL NOTES.
 SEE S2 SERIES FOR TYPICAL CONCRETE DETAILS.
- 3. SEE S3 SERIES FOR TYPICAL WOOD DETAILS.
- ALL DIMENSIONS SHALL BE VERIFIED WITH ARCHITECT AND ARCHITECTURAL DRAWINGS.
 SF2.0 INDICATES SPREAD FOOTING TYPE. SEE SCHEDULE FOR SIZE AND REINFORCING. ALL FOOTINGS
- EXPOSED TO FREEZE/THAW SHALL BEAR A MINIMUM OF 2'-6" BELOW GRADE OR MORE DEPENDING ON LOCAL JURISDICTION
- W2.0 INDICATES STRIP FOOTING TYPE. SEE SCHEDULE FOR SIZE AND REINFORCING. ALL FOOTINGS EXPOSED TO FREEZE/THAW SHALL BEAR A MINIMUM OF 2'-6" BELOW GRADE OR MORE DEPENDING ON
- LOCAL JURISDICTION 7. S-12 - INDICATES SONOTUBE FOOTING SIZE. SEE SCHEDULE FOR SIZE AND REINFORCING. ALL FOOTINGS EXPOSED TO FREEZE/THAW SHALL BEAR A MINIMUM OF 2'-6" BELOW GRADE OR MORE DEPENDING ON
- LOCAL JURISDICTION 8. BF20 - INDICATES "BIG FOOT" FOOTING SIZE. SEE "POST ON BIG FOOT" DETAIL IF APPLICABLE. ALL
- FOOTINGS EXPOSED TO FREEZE/THAW SHALL BEAR A MINIMUM OF 2'-6" BELOW GRADE OR MORE
- DEPENDING ON LOCAL JURISDICTION 9. AB32 - INDICATES ANCHOR BOLT SPACING. SEE SHEAR WALL SCHEDULE FOR ALTERNATIVE 5/8" SPACING.
- ANCHORS BOLTS TO BE 1/2" AND SPACED AT 32" O.C. UNO. AB24= 24" O.C. SPACING. AB16=16" O.C. SPACING 10. FW8 - INDICATES FOUNDATION WALL TYPE. SEE FOUNDATION WALL SCHEDULE. MAX HEIGHT GIVEN IS
- FLOOR TO FLOOR HEIGHT.





WALL FOOTING SCHEDULE



 $^{1 \}frac{\text{ROOF FRAMING PLAN}}{1/4" = 1'-0"}$









14 STRUCTURAL FASCIA CONNECTION







10 TYPICAL I JOIST RAFTER



1 OUTRIGGER DETAIL

4 TYPICAL LVL ATTACHMENT

<u>4-PLY</u>



5 TYPICAL BEAM ATTACHMENT



(6) WOOD BEAM TO WOOD COLUMN



7 INTERIOR BEARING WALL





2 ROOF LEDGER DETAIL



+ + + + +

+ + + + +

PER BELOW

1 ROOF LEDGER - PARALLEL



HAZEL ENGINE Salt Lake PH: 801-8 hazelwood CLIENT SOLSTICE DESIGN	HAZELWOOD ENGINEERING Salt Lake City, UT PH: 801-810-5061 hazelwood-eng.com CLIENT SOLSTICE DESIGN BUILD		
Revie Code Co 09/25	wed for ompliance		
MILNER MICROHOUSING	LOT 15 AND LOT 16, MAIN ST, STEAMBOAT SPRINGS, CO		
PROJECT NUMBER 23 DATE 5/12	00 L/C: V 20/2023 0: 0 2108 0: 0 NAL FN0355 NAL FN0355 NAL FN0355 DESCRIPTION RMIT		
DRAWN BY AL	uthor pprover ETAILS I		
S	3.0		