

MAXIMUM	SLUMP, INCHES (+/- 1")	AIR CONTENT PERCENT (+/- 1.5%)	CEMENT TYPE	ADMIXTURES / COMMENTS
3/4" STONE	5	N/A	ASTM C150 I/II OR ASTM C595 IL	
3/4" STONE	4	6%	ASTM C150 I/II OR ASTM C595 IL	
3/4" STONE	4	3%	ASTM C150 I/II OR ASTM C595 IL	
3/4" STONE	4	N/P	ASTM C150 I/II OR ASTM C595 IL	
3/4" STONE	4	N/P	ASTM C150 I/II OR ASTM C595 IL	
3/4" STONE	4	6%	ASTM C150 I/II OR ASTM C595 IL	25% MAX FLY ASH

-, INCHES
TO 6
TO 8
TO 9

A. CONCRETE MIX SUBMITTAL SHALL INCLUDE A SINGLE PAGE LIST OF MIXES, IN TABULAR FORMAT, WITH KEY

SHRINKAGE (0.05% UNLESS NOTED OTHERWISE) AT 28 DAYS (500 MICROSTRAIN) AS MEASURED BY ASTM

SUBCONTRACTORS (INCLUDING BUT NOT LIMITED TO FLOORING) TO ADDRESS ALL POTENTIAL SCHEDULE

REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, EXCEPT BARS SHOWN TO BE FIELD-BENT SHALL

DSED TO EARTH:	3"
SMALLER	2" 1-1/2"
FACT WITH GROUND: RS AND SMALLER	3/4"

IENT	1-1/2"
\LS	1-1/2"

POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE

CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR.

INSTALLATION INFORMATION (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT DEPTH AS INDICATED ON THE DRAWINGS. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MPIL.

CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER; REGISTRATION MUST BE IN THE STATE IN WHICH THE PROJECT IS

TRAINING SHALL BE KEPT ON SITE AND MADE AVAILABLE TO THE EOR/ SPECIAL INSPECTOR AS REQUESTED. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION THAT SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI (ACI 318-11 D 9.2.2, ACI 318-14 17.8.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO

DST-INSTALLED ANCHORS						
	HILTI	SIMPSON				
	KWIK BOLT TZ2 (ICC ESR-4266)	STRONG-BOLT 2 (ICC ESR-3037)				
	KWIK HUS-EZ (ICC ESR-3027)	TITEN HD (ICC ESR-2713)				
HIT HY-200 V3 (ICC ESR-4868)		AT-XP (UES ER-263)				
ST-INSTALLED ANCHORS						
HILTI SIMPSON						

	HILII	SIMPSON
)	KWIK BOLT TZ2 (ICC ESR-4561)	WEDGE-ALL (ICC ESR-1396)
	KWIK HUS-EZ (ICC ESR-3056)	TITEN HD (ICC ESR-1056)
	HIT HY-270 (ICC ESR-4143 / 4144)	AT-XP (UES ER-281)

STRUCTURAL STEEL 1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE

"SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" (AISC 360) AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (AISC 303) BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC). STRUCTURAL STEEL WIDE FLANGE BEAMS AND WTS SHALL CONFORM TO ASTM A992, 50 KSI YIELD. ROLLED STEEL FLOOR PLATES SHALL CONFORM TO ASTM A786, COMMERCIAL GRADE.

OTHER ROLLED SHAPES, INCLUDING PLATES, CHANNELS, AND ANGLES SHALL CONFORM TO ASTM A36, 36 KSI YIFI D 5. HOLLOW STRUCTURAL SECTION (HSS) RECTANGULAR SHAPES SHALL CONFORM TO ASTM A500, GRADE C, 50 KSI

- YIELD. 6. HSS ROUND SHAPES SHALL CONFORM TO ASTM A500, GRADE C, 46 KSI YIELD.
- PIPE SHAPES SHALL CONFORM TO ASTM A53, GRADE B, 35 KSI YIELD.
- EXCEPT AS NOTED, FRAMED BEAM CONNECTIONS SHALL BE BEARING-TYPE WITH 3/4" DIAMETER, SNUG TIGHT, ASTM F3125 BOLTS, DETAILED IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND THE "STEEL CONSTRUCTION MANUAL" BY THE AISC. INSTALL BOLTS IN ACCORDANCE WITH AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS".
- 9. ALL BEAMS SHALL HAVE FULL DEPTH WEB STIFFENERS EACH SIDE OF WEBS ABOVE AND BELOW COLUMNS. 10. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE (36, 55 WITH WELDABILITY SUPPLEMENT S1, AND/OR 105) AS NOTED ON THE STRUCTURAL DRAWINGS.
- 11. HEADED ANCHOR STUDS (HAS) SHALL CONFORM TO ASTM A108 AND SHALL BE CONNECTED TO STRUCTURAL STEEL WITH EQUIPMENT APPROVED BY THE STUD MANUFACTURER ACCORDING TO THE STUD MANUFACTURER'S RECOMMENDATIONS.
- 12. WELDING SHALL BE DONE BY A CERTIFIED WELDER IN ACCORDANCE WITH THE AISC DOCUMENTS LISTED ABOVE, THE AMERICAN WELDING SOCIETY (AWS) D1.1: STRUCTURAL WELDING CODE, AND THE RECOMMENDATIONS FOR USE OF WELD E70 ELECTRODES. WHERE NOT SPECIFICALLY NOTED, MINIMUM WELD SHALL BE 3/16" FILLET BY LENGTH OF CONTACT EDGE.
- 13. GROUT BENEATH COLUMN BASE AND BEAM BEARING PLATES SHALL HAVE A MINIMUM 28-DAY, COMPRESSIVE STRENGTH OF 7,500 PSI AND SHALL BE NON-SHRINK, NON-METALLIC, AND TESTED IN ACCORDANCE WITH ASTM C1107.

CORROSION CONTROL

- ALL STEEL MEMBERS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED PER ASTM A123. FASTENERS AND HARDWARE SHALL BE HOT DIPPED GALVANIZED PER ASTM A153 OR ASTM B695 CLASS 50 (A490 BOLTS SHALL NOT BE HOT DIPPED GALVANIZED). STAINLESS STEEL FASTENERS AND HARDWARE MAY ALSO BE
- 3. ALL FIELD CUT OR DAMAGED SURFACES, FIELD WELDED AREAS AND AUTHORIZED NON-GALVANIZED MEMBERS AS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE REPAIRED WITH (2) COATS OF A 95% ZINC RICH PAINT PER ASTM A780 (ZRC PREFERRED).

STRUCTURAL WOOD FRAMING:

- IN-GRADE BASE VALUES HAVE BEEN USED FOR DESIGN.
- DIMENSIONAL LUMBER FRAMING SHALL BE S4S HEM FIR NO. 2 AND BETTER UNO. SOLID TIMBER BEAMS AND POSTS SHALL BE DOUGLAS FIR-LARCH NO. 1 AND BETTER UNO.
- STUDS SHALL BE DOUG FIR-LARCH OR HEM FIR STUD AND BETTER UNO.
- TOP AND BOTTOM PLATES SHALL BE DOUGLAS FIR-LARCH NO. 2 AND BETTER UNO 6. ALL LUMBER SHALL BE 19% MAXIMUM MOISTURE CONTENT AT THE TIME OF INSTALLATION UNO
- ALL WOOD EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED DOUGLAS FIR-LARCH OR SOUTHERN YELLOW PINE. PRESERVATIVE-TREATED WOOD SHALL BE
- TREATED IN ACCORDANCE WITH AWPA STANDARDS U1 AND M4. TREATMENTS SHALL HAVE NO AMMONIA ADDED AND SHALL BE THE FOLLOWING USE CATEGORY:
- A. UC2 AT INTERIOR
- B. UC3B AT EXTERIOR WITH NO GROUND CONTACT C. UC4B AT EXTERIOR WITH GROUND CONTACT
- 8. FASTENERS FOR USE WITH TREATED WOOD SHALL BE CORROSION RESISTANT IN ACCORDANCE WITH SECTION 2304.9.5 (2012 IBC), 2304.10.5 (2015 & 2018 IBC), 2304.10.6 (2021 IBC).
- 9. ALL CONNECTORS USED WITH PRESSURE-TREATED MATERIAL SHALL BE STAINLESS STEEL ASTM 304 OR 316, OR HAVE A SIMPSON Z-MAX (G185) OR HDG COATING. STANDARD COATING (G90) IS ACCEPTABLE AT INTERIOR CONDITIONS WITH NON PRESSURE-TREATED LUMBER ONLY. CONNECTORS ARE TO BE IN ACCORDANCE WITH ASTM A653 OR ASTM 123.
- 10. ALL IRON AND STEEL PRODUCTS ATTACHED TO TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 OR SHALL BE TYPE 304 OR 316 STAINLESS STEEL
- 11. STRUCTURAL MEMBERS SHALL NOT BE CUT FOR PIPES, ETC. UNLESS SPECIFICALLY NOTED OR DETAILED ON THE STRUCTURAL DRAWINGS.
- 12. ALL BOLTS SHALL BE RETIGHTENED PRIOR TO CLOSING IN OF WALLS, FLOORS, AND ROOFS.
- 13. ALL BOLTS BEARING ON WOOD SHALL HAVE STANDARD CUT WASHERS UNDER HEAD AND/OR NUT. UNO. 14. METAL FRAMING ANCHORS SHOWN OR REQUIRED, SHALL BE SIMPSON STRONG-TIE OR EQUAL CODE APPROVED CONNECTORS AND INSTALLED WITH ALL HOLES FILLED (ROUND AND TRIANGULAR) WITH THE MAXIMUM SIZE NAIL RECOMMENDED BY THE MANUFACTURER TO DEVELOP THE MAXIMUM RATED CAPACITY. 15. CONNECTOR BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME B18.2.1 AND ASTM SAE J429 GRADE 1.
- 16. NAILS AND SPIKES SHALL CONFORM TO ASTM F1667. 17. WOOD SCREWS SHALL CONFORM TO ANSI/ASME B18.6.1.
- 18. LEAD HOLES FOR LAG SCREWS SHALL BE 40%-70% OF THE SHANK DIAMETER AT THE THREADED SECTION AND
- EQUAL TO THE SHANK DIAMETER AT THE UNTHREADED SECTION. 19. CONVENTIONAL LIGHT FRAMING SHALL COMPLY WITH IBC SECTION 2308.
- 20. COLUMNS / MULTIPLE STUDS IN BEARING WALLS SUPPORTING ALL BEAMS AND HEADERS SHALL OCCUR CONTINUOUSLY THROUGH EACH FLOOR LEVEL DOWN TO THE FOUNDATION OR ANOTHER SUPPORT BEAM. SOLID SQUASH BLOCKING EQUIVALENT IN AREA TO THE COLUMN/MULTIPLE STUDS ABOVE SHALL BE PROVIDED WITHIN THE JOIST SPACE BENEATH THE COLUMN/MULTIPLE STUDS.
- 21. ALL BEAMS AND TRUSSES SHALL BE BRACED AGAINST ROTATION AT POINTS OF BEARING. 22. 2X BLOCKING SHALL BE PLACED BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS, UNO.
- 23. CROSS-BRIDGING OR SOLID BLOCKING SHALL BE PROVIDED AT 8'-0" MAX UNO. FOR ALL JOISTS AND RAFTERS MORE THAN 10" IN DEPTH, 2X3 OR APPROVED METAL TYPE BRIDGING MAY BE USED.
- 24. PROVIDE A MINIMUM OF (3) STUDS AT EACH CORNER, UNO. 25. ALL JOISTS AND BEAMS (EXCLUDING I-JOISTS) SHALL BE SEAT-CUT FOR FULL UNIFORM BEARING AT SUPPORTS,
- SEATS, CAPS, ETC. 26. VENTING IS REQUIRED IN ALL ENCLOSED ROOF AND CRAWL SPACE FRAMING CAVITIES, SEE ARCHITECTURAL
- DRAWINGS
- 27. EXCEPT AS NOTED OTHERWISE, MINIMUM NAILING SHALL BE PROVIDED AS SPECIFIED IN TABLE 2304.9.1 (2012 IBC), 2304.10.1 (2015 & 2018 IBC), 2304.10.2 (2021 IBC) "FASTENING SCHEDULE".
- 28. ALL MULTIPLE MEMBER BEAMS SHALL BE NAILED TOGETHER WITH MAX NUMBER OF 10D NAILS VERTICALLY @ 3" AND HORIZONTALLY @ 12" PER PLY. 29. TONGUE AND GROOVE DECKING SHALL BE INSTALLED IN ACCORDANCE WITH THE "STANDARD FOR TONGUE AND
- GROOVE HEAVY TIMBER ROOF DECKING", AITC 112. WHERE DECKING MUST BE NAILED FROM THE BOTTOM SIDE, USE (2) 16D GALVANIZED FINISH NAILS AT EACH SUPPORT, COUNTERSUNK AND FILLED. 30. ALL ROOF RAFTERS, JOISTS, TRUSSES, AND BEAMS SHALL BE ANCHORED TO SUPPORTS WITH H2.5A METAL
- FRAMING ANCHORS UNO. PROVIDE (2) WITHIN 4'-0" OF ALL CORNERS.

WOOD SHEATHING:

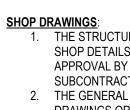
PLYWOOD AND ORIENTED STRAND BOARD (OSB) FLOOR AND ROOF SHEATHING SHALL BE APA RATED WITH

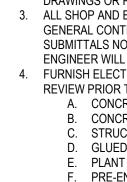
- STAMP INCLUDING APA TRADEMARK AND PANEL SPAN RATING. A. MINIMUM FLOOR SHEATHING: 23/32" APA STURD-I-FLOOR RATED 24 INCH O.C. TONGUE & GROOVE GLUED AND NAILED.
- B. MINIMUM ROOF SHEATHING: 19/32" OSB OR CDX PLYWOOD, APA 40/20, NAILED. MINIMUM WALL SHEATHING: 7/16" OSB OR CDX PLYWOOD, APA 24/16, BLOCKED AND NAILED.
- 2. NAIL WALL SHEATHING WITH MINIMUM 8D COMMON OR 10D BOX AT 6" AT PANEL EDGES, AND 12" AT
- INTERMEDIATE FRAMING EXCEPT AS NOTED. BLOCK AND NAIL ALL EDGES BETWEEN STUDS. 3. MINIMUM (3) 8D NAILS PER STUD. NAIL ALL PLATES USING EDGE NAIL SPACING INDICATED.
- 4. SHEATHE ALL EXTERIOR WALLS. SHEATHE INTERIOR WALLS AS DESIGNATED ON THE DRAWINGS.
- 5. SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO TOP PLATE. CUT IN "L" AND "T" SHAPES AROUND OPENINGS. LAP SHEATHING OVER SINGLE 2X PLATE MEMBER AT RIM JOIST. AT RIM JOIST PROVIDE A MINIMUM OF 3" BETWEEN SHEATHING EDGE AND TOP/BOTTOM EDGE OF RIM.
- MINIMUM HEIGHT OF SHEATHING PANELS SHALL BE 16" TO ENSURE THAT PLATES ARE TIED TO STUDS. ALL SHEATHING SHEETS SHALL HAVE 1/8" GAP AT ALL EDGES AND JOINTS.
- 8. FULLY NAIL FLOOR SHEATHING IMMEDIATELY AFTER GLUING (DO NOT SPOT NAIL).
- 9. PROVIDE (1) PANEL SHEATHING CLIP AT ALL UNSUPPORTED ROOF SHEATHING PANEL EDGES. WHERE SPANS ARE GREATER THAN 32" PROVIDE (2) CLIPS.

STRUCTURAL INSULATED PANELS (SIPS):

- SIPS SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH SECTION R613.
- SIPS MANUFACTURER SHALL BE A MEMBER OF THE STRUCTURAL INSULATED PANEL ASSOCIATION (SIPA). 3. INTERIOR PANELS SHALL BE TREATED WITH BLUEWOOD FUNGUS AND MOISTURE PROTECTION OR EQUIVALENT APPROVED BY ARCHITECT.
- 4. EPS CORES SHALL BE TREATED WITH BORATE PRESERVATIVE.
- 5. ALL DIMENSIONAL LUMBER SPLINES AND PLATES SHALL BE HEM-FIR NO. 2 OR BETTER.

VALUES: A. F_b = 2400 PSI B. F_v = 190 PSI E. E = 1800 KSI SEAL CUT EDGES AND ENDS EXPOSED TO WEATHERING.





STRUCTURAL CAPACITIES OF STRUCTURAL COMPOSITE LUMBER SHALL BE IN CONFORMANCE WITH SECTION 2303.1.9 (2012 IBC), 2303.1.10 (2015, 2018 & 2021 IBC).

MANUFACTURER OF STRUCTURAL COMPOSITE LUMBER PRODUCTS SHALL HAVE PROPER CODE EVALUATION REPORTS FOR ALL PRODUCTS AND SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. THE CONTRACTOR SHALL NOT CUT, NOTCH, OR OTHERWISE ALTER STRUCTURAL COMPOSITE LUMBER MEMBERS WITHOUT WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER AND THE MANUFACTURER; HOWEVER, HOLES MAY BE CUT IN MEMBERS IN ACCORDANCE WITH THE MANUFACTURER'S ALLOWABLE HOLE CHART. MEMBERS NOTED AS LVL (LAMINATED VENEER LUMBER) ON PLAN SHALL BE 1-3/4" WIDE X DEPTH INDICATED, PLANT-FABRICATED, AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:

A. F_b = 2600 PSI B. F_v = 285 PSI C. F_{cPAR} = 2460 PSI D. $F_{CPERP} = 750 PSI$ E. E = 1900 KSI

ENGINEERED LUMBER:

MEMBERS NOTED AS PSL (PARALLEL STRAND LUMBER) ON PLAN SHALL BE PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES: A. F_b = 2900 PSI

C. $F_{cPAR} = 2900 PSI$ D. $F_{cPERP} = 750 PSI$ E. E = 2000 KSI A. F_b = 1700 PSI B. F_v = 400 PSI C. $F_{cPAR} = 1400 PSI$ D. $F_{cPERP} = 680 PSI$ E. E = 1300 KSI

B. F_v = 290 PSI

MEMBERS NOTED AS LSL (LAMINATED STRAND LUMBER) ON PLAN SHALL BE PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:

BRIDGING AND BLOCKING SHALL BE INSTALLED ACCORDING TO THE FABRICATOR'S REQUIREMENTS. WOOD I-JOISTS SHALL HAVE THE DEPTH, SPACING, SPAN, AND LAYOUT SHOWN ON THE DRAWINGS. MEMBERS SHALL BE FACTORY MANUFACTURED WITH ORIENTED STRAND BOARD (OSB) WEBS, LAMINATED VENEER LUMBER (LVL) OR MACHINE STRESS RATED (MSR) LUMBER FLANGES PER CODE APPROVAL BY ICB OR NER. STRUCTURAL WOOD FLANGES AND WEBS SHALL BE DESIGNED FOR STRUCTURAL CAPACITIES AND DESIGN PROVISIONS ACCORDING TO ASTM D 5055. SUBSTITUTION OF EQUIVALENT SERIES BY OTHERS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL. JOISTS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS. HOLES IN WEBS SHALL NOT

EXCEED MANUFACTURER'S PUBLISHED LIMIT CRITERIA. 10. OPEN WEB TRUSSES SHALL HAVE THE DEPTH, SPACING, SPAN, AND LAYOUT SHOWN ON THE DRAWINGS. MEMBERS SHALL BE FACTORY MANUFACTURED WITH TUBULAR STEEL WEBS, AND LAMINATED VENEER LUMBER (LVL) OR MACHINE STRESS RATED (MSR) LUMBER CHORDS PER CODE APPROVAL BY ICB OR NER.

11. OPEN WEB JOISTS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED TO CARRY THE LOADS INDICATED ON THE STRUCTURAL DRAWINGS. 12. MEMBER FORCES SHALL BE DETERMINED BY THE FABRICATOR. STRESSES SHALL NOT EXCEED THOSE ALLOWED

13. DEFLECTION LIMITS FOR WOOD I-JOISTS AND OPEN WEB JOISTS SHALL NOT EXCEED THE FOLLOWING DEFLECTION CRITERIA:

Α.	ROOF LIVE LOAD =	L/360
В.	ROOF TOTAL LOAD =	L/240 (1" MAXIMUM
C.	FLOOR LIVE LOAD =	L/480
D.	FLOOR TOTAL LOAD =	L/240 (1" MAXIMUM

TONGUE AND GROOVE DECKING

BY THE IBC.

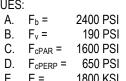
TONGUE AND GROOVE DECKING SHALL BE DOUGLAS FIR-LARCH AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES: A. F_b = 2000 PSI

B. $F_v = 165 PSI$ C. E = 1800 KSI TONGUE AND GROOVE DECKING SHALL COMPLY WITH SECTION 2304.8 OF THE IBC INSTALLED IN A <SIMPLE SPAN><TWO SPAN CONTINUOUS><COMBINATION SIMPLE SPAN TWO SPAN CONTINUOUS> <CANTILEVERED PIECES INTERMIXED> <CONTROLLED RANDOM> LAYUP PATTERN.

TONGUE AND GROOVE DECKING SHALL BE INSTALLED IN ACCORDANCE WITH THE "STANDARD FOR TONGUE AND GROOVE HEAVY TIMBER ROOF DECKING", AITC 112. WHERE DECKING MUST BE NAILED FROM THE BOTTOM SIDE, USE (2) 16D GALVANIZED FINISH NAILS AT EACH SUPPORT, COUNTERSUNK AND FILLED.

STRUCTURAL GLUED LAMINATED TIMBER:

MATERIALS, MANUFACTURE, AND QUALITY CONTROL SHALL BE IN CONFORMANCE WITH ANSI/AITC A 190.1 "STRUCTURAL GLUED LAMINATED TIMBER" AND AITC 117 "STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED LAMINATED TIMBER OF SOFTWOOD SPECIES, DESIGN AND MANUFACTURING REQUIREMENTS." INTERIOR GLUED LAMINATED DOUGLAS FIR BEAMS SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN



SIMPLE SPAN BEAMS SHALL BE COMBINATION SYMBOL 24F-V4 WITH NO CAMBER.

CONTINUOUS AND CANTILEVERED MEMBERS SHALL BE COMBINATION SYMBOL 24F-V8 WITH NO CAMBER.

COLUMNS SHALL BE COMBINATION #2 OR BETTER. MEMBERS SHALL BE ARCHITECTURAL APPEARANCE GRADE.

ADHESIVES SHALL MEET THE REQUIREMENTS FOR WET CONDITIONS OF SERVICE.

9. THE FABRICATOR SHALL FURNISH ALL ITEMS OF CONNECTION STEEL AND HARDWARE FOR JOINING TIMBER MEMBERS TO EACH OTHER AND TO THEIR SUPPORTS, EXCLUSIVE OF ANCHORAGE EMBEDDED IN MASONRY, SETTING PLATES, AND ITEMS FIELD-WELDED TO STRUCTURAL STEEL.

THE STRUCTURAL DRAWINGS ARE COPYRIGHTED AND SHALL NOT BE COPIED FOR USE AS ERECTION PLANS OR SHOP DETAILS. USE OF JVA'S ELECTRONIC FILES AS THE BASIS FOR SHOP DRAWINGS REQUIRES PRIOR APPROVAL BY JVA, A SIGNED RELEASE OF LIABILITY BY THE GENERAL CONTRACTOR AND/OR HIS SUBCONTRACTORS, AND DELETION OF JVA'S NAME AND LOGO FROM ALL SHEETS SO USED.

THE GENERAL CONTRACTOR SHALL SUBMIT IN WRITING ANY REQUESTS TO MODIFY THE STRUCTURAL DRAWINGS OR PROJECT SPECIFICATIONS.

3. ALL SHOP AND ERECTION DRAWINGS SHALL BE CHECKED AND STAMPED (AFTER HAVING BEEN CHECKED) BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION FOR STRUCTURAL ENGINEER'S REVIEW: SHOP DRAWING SUBMITTALS NOT CHECKED BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION TO THE STRUCTURAL

ENGINEER WILL BE RETURNED WITHOUT REVIEW. FURNISH ELECTRONIC VERSION (PDF) OF SHOP AND ERECTION DRAWINGS TO THE STRUCTURAL ENGINEER FOR **REVIEW PRIOR TO FABRICATION FOR:**

A. CONCRETE MIX DESIGNS B. CONCRETE REINFORCING STEEL

STRUCTURAL STEEL

GLUED-LAMINATED TIMBER PLANT FABRICATED WOOD JOISTS

PRE-ENGINEERED SIP ROOF AND WALL PANELS

SUBMIT IN A TIMELY MANNER TO PERMIT 10 WORKING DAYS FOR REVIEW BY THE STRUCTURAL ENGINEER. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "REQUEST FOR CHANGE IN WRITING" UNLESS SPECIFIC SUGGESTED CHANGES ARE CLEARLY MARKED. IN ANY EVENT, CHANGES MADE BY MEANS OF THE SHOP DRAWING SUBMITTAL PROCESS BECOME THE RESPONSIBILITY OF THE ONE INITIATING THE CHANGE.

STRUCTURAL DRAWING LIST					
SHEET NO	SHEET TITLE				
S-0.1	GENERAL NOTES				
S-0.2	GENERAL NOTES				
S-0.3	ABBREVIATION, SYMBOLS KEY & 3D SCHEMATIC VIEWS				
S-1.1	FOUNDATION PLAN				
S-1.2	MAIN LEVEL FLOOR FRAMING PLAN				
S-1.3	ROOF FRAMING PLAN				
S-1.4	SHEAR WALL PLAN				
S-4.1	FRAME ELEVATIONS				
S-5.1	TYPICAL CONCRETE DETAILS & SCHEDULES				
S-5.2	TYPICAL STEEL DETAILS & SCHEDULES				
S-5.3	TYPICAL WOOD DETAILS & SCHEDULES				
S-5.4	TYPICAL WOOD DETAILS				
S-5.5	TYPICAL WOOD DETAILS - SHEAR WALL				
S-5.6	TYPICAL PE TRUSS JOIST DETAILS				
S-5.7	TYPICAL SIP PANEL DETAILS				
S-5.8	FOUNDATION SECTIONS				
S-5.9	ROOF SECTIONS				

Reviewed for Code Compliance

05/09/2024



CONSULTING ENGINEERS

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No.	Description	Date
	Permit Submission	04/19/2024
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Warning: It is a violation for any person, unless he is acting under the direction of a licensed Architect or Professional Engineer, to alter this item in any way.

PROJECT NAME

INGRAM STAGECOACH

DRAWING TITLE

GENERAL NOTES

SEAL & SIGNATURE DATE 04/19/2024 PROJ NO. 240401 DRAWN BY HMH, LAB CHECKED BYPES, MES DWG NO. S-0.

STRUCTURAL GENERAL NOTES

STRUCTURAL ERECTION AND BRACING REQUIREMENTS: 1. THE STRUCTURAL DRAWINGS ILLUSTRATE AND DESCRIBE THE COMPLETED STRUCTURE WITH ELEMENTS IN

- THEIR FINAL POSITIONS, PROPERLY SUPPORTED, CONNECTED, AND/OR BRACED. 2. THE STRUCTURAL DRAWINGS ILLUSTRATE TYPICAL AND REPRESENTATIVE DETAILS TO ASSIST THE GENERAL CONTRACTOR. DETAILS SHOWN APPLY AT ALL SIMILAR CONDITIONS UNLESS OTHERWISE INDICATED. ALTHOUGH DUE DILIGENCE HAS BEEN APPLIED TO MAKE THE DRAWINGS AS COMPLETE AS POSSIBLE, NOT
- EVERY DETAIL IS ILLUSTRATED AND NOT EVERY EXCEPTIONAL CONDITION IS ADDRESSED. 3. ALL PROPRIETARY CONNECTIONS AND ELEMENTS SHALL BE INSTALLED IN ACCORDANCE WITH THE
- MANUFACTURERS' RECOMMENDATIONS. 4. ALL WORK SHALL BE ACCOMPLISHED IN A WORKMANLIKE MANNER AND IN ACCORDANCE WITH THE APPLICABLE CODES AND LOCAL ORDINANCES.
- 5. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL WORK, INCLUDING LAYOUT AND DIMENSION VERIFICATION, MATERIALS COORDINATION, SHOP DRAWING REVIEW, AND THE WORK OF SUBCONTRACTORS. ANY DISCREPANCIES OR OMISSIONS DISCOVERED IN THE COURSE OF THE WORK SHALL BE IMMEDIATELY REPORTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR RESOLUTION.
- 6. CONTINUATION OF WORK WITHOUT NOTIFICATION OF DISCREPANCIES RELIEVES THE ARCHITECT AND STRUCTURAL ENGINEER FROM ALL CONSEQUENCES.
- UNLESS OTHERWISE SPECIFICALLY INDICATED, THE STRUCTURAL DRAWINGS DO NOT DESCRIBE METHODS OF CONSTRUCTION.
- 8. VERIFY ALL OPENINGS THROUGH FLOORS, ROOFS AND WALLS WITH ARCHITECTURAL, MECHANICAL AND
- ELECTRICAL REQUIREMENTS. COORDINATE DIMENSIONS WITH SUPPLIERS PRIOR TO FABRICATION. 9. SIGNIFICANT PERMANENT EQUIPMENT SIZES, WEIGHTS AND LOCATIONS ARE INDICATED ON THE DRAWINGS AS PROVIDED TO THE ENGINEER DURING DESIGN. CHANGES IN SIZE, WEIGHT, OR LOCATION MUST BE SUBMITTED IN WRITING FOR REVIEW BY THE ENGINEER. REQUIRED CURBS, SUPPORTS, OR BRACES NOT SHOWN ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE EQUIPMENT SUPPLIER.
- 10. THE GENERAL CONTRACTOR, IN THE PROPER SEQUENCE, SHALL PERFORM OR SUPERVISE ALL WORK NECESSARY TO ACHIEVE THE FINAL COMPLETED STRUCTURE, AND TO PROTECT THE STRUCTURE, WORKMEN, AND OTHERS DURING CONSTRUCTION. SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO TEMPORARY BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR EXCAVATION, FORMWORK, SCAFFOLDING, SAFETY DEVICES AND PROGRAMS OF ALL KINDS, SUPPORT AND BRACING FOR CRANES AND OTHER ERECTION EQUIPMENT.
- 11. DO NOT BACKFILL AGAINST BASEMENT OR RETAINING WALLS UNTIL SUPPORTING SLABS AND FLOOR FRAMING ARE IN PLACE AND SECURELY ANCHORED, UNLESS ADEQUATE TEMPORARY BRACING IS PROVIDED. 12. TEMPORARY BRACING SHALL REMAIN IN PLACE UNTIL ALL FLOORS, WALLS, ROOFS AND ANY OTHER
- SUPPORTING ELEMENTS ARE IN PLACE. 13. THE ARCHITECT AND STRUCTURAL ENGINEER BEAR NO RESPONSIBILITY FOR THE ABOVE ITEMS, AND OBSERVATION VISITS TO THE SITE DO NOT IN ANY WAY INCLUDE INSPECTIONS OF THESE ITEMS.

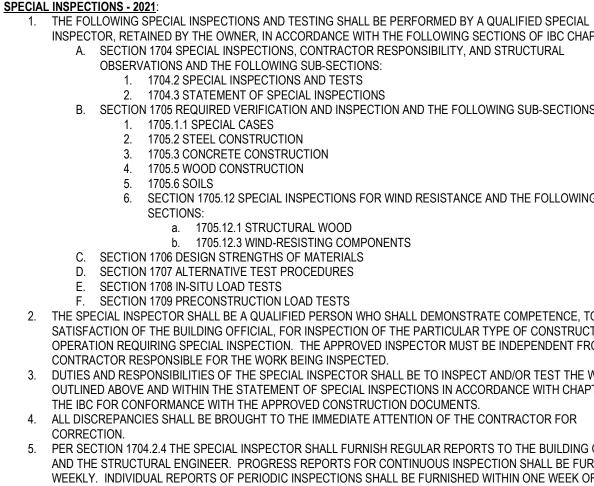
PRECAUTIONARY NOTES ON STRUCTURAL BEHAVIOR:

- INTERIOR ARCHITECTURAL FINISH DETAILING MUST ACCOMMODATE THE RELATIVE DIFFERENTIAL MOVEMENTS OF SUPPORTING STRUCTURAL ELEMENTS. 2. WHERE THE ROOF FRAMING ELEMENT SPANS ARE LONG, APPLIED LOADING WILL NATURALLY CAUSE
- SUBSTANTIAL DEFLECTION. INTERIOR ELEMENTS HUNG FROM THE ROOF STRUCTURE WILL DEFLECT WITH THE ROOF
- 3. THE FLOOR IS A FLOATING CONCRETE SLAB-ON-GRADE AND MAY EXPERIENCE MOVEMENTS INDEPENDENT OF THE STRUCTURAL FOUNDATIONS. INTERIOR ELEMENTS SUPPORTED ON THE SLAB-ON-GRADE FLOOR WILL MOVE WITH THE FLOOR. INTERIOR ELEMENTS SUPPORTED ON FOUNDATIONS AND COLUMNS WILL NOT EXPERIENCE SIMILAR OR MEASURABLE MOVEMENTS.
- 4. EXTERIOR/PERIMETER WALL ASSEMBLIES HUNG FROM THE EDGE OF THE BUILDING STRUCTURE WILL BE DIRECTLY AFFECTED (TO SOME DEGREE) BY CHANGES IN EXTERNAL TEMPERATURE AND FLOOR DEFLECTION. 5. EXTERIOR/PERIMETER AND INTERIOR ARCHITECTURAL FINISH DETAILS SHOULD ALLOW FOR RELATIVE MOVEMENTS BETWEEN ELEMENTS WITH DIFFERENT SUPPORT CONDITIONS.

- DEFERRED SUBMITTALS: PORTIONS OF THE STRUCTURE HAVE ELEMENTS OF PROPRIETARY DESIGN AND FABRICATION, WHICH SHALL BE SUBMITTED BY THE SUPPLIER FOR APPROVAL AFTER AWARD OF CONTRACT.
- 2. THESE ITEMS SHALL CONFORM TO THE LOAD, CAPACITY, SIZE, GEOMETRY, CONNECTION, AND SUPPORT CRITERIA NOTED ON THE STRUCTURAL DRAWINGS.
- 3. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED BY AN ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. FINAL SHOP DRAWING SUBMITTALS SHALL BE STAMPED AND SIGNED.
- 4. FURNISH DEFERRED SUBMITTALS FOR:
- A. CANOPIES, SUNSCREENS, AND SUNSHADES
- B. STAIRS, HANDRAILS, AND GUARDRAILS C. STRUCTURAL INSULATED PANELS
- D. STRUCTURAL STEEL CONNECTIONS
- 5. DEFERRED SUBMITTALS WILL BE REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD FOR COMPLIANCE WITH THE SPECIFIED DESIGN REQUIREMENTS, STAMPED AS "REVIEWED," AND RETURNED TO THE CONTRACTOR. THE GENERAL CONTRACTOR SHALL FORWARD THE REVIEWED DEFERRED SUBMITTALS TO THE LOCAL BUILDING AUTHORITY FOR REVIEW AND APPROVAL BEFORE INSTALLATION OF DEFERRED SUBMITTAL ITEMS. 6. FINAL ISSUE OF THE BUILDING PERMIT MAY, AT THE APPROVAL AUTHORITY'S OPTION, BE CONTINGENT ON ITS
- APPROVAL OF THE DEFERRED SUBMITTAL DOCUMENTS. DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN CALCULATIONS AND DRAWINGS HAVE BEEN REVIEWED BY THE ARCHITECT, STRUCTURAL ENGINEER, AND/OR LOCAL BUILDING AUTHORITY AS REQUIRED.

LETTERS OF CONSTRUCTION COMPLIANCE:

- 1. THE GENERAL CONTRACTOR SHALL DETERMINE FROM THE LOCAL BUILDING AUTHORITY, AT THE TIME THE BUILDING PERMIT IS OBTAINED, WHETHER ANY LETTERS OF CONSTRUCTION COMPLIANCE WILL BE REQUESTED FROM THE STRUCTURAL ENGINEER. 2. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ALL SUCH REQUIREMENTS IN WRITING PRIOR
- TO THE START OF CONSTRUCTION.
- 3. TWO-DAY ADVANCE NOTICE SHALL BE GIVEN WHEN REQUESTING SITE VISITS NECESSARY AS THE BASIS FOR THE COMPLIANCE LETTER. 4. THE GENERAL CONTRACTOR SHALL PROVIDE COPIES OF ALL THIRD-PARTY TESTING AND INSPECTION REPORTS
- TO THE ARCHITECT AND STRUCTURAL ENGINEER A MINIMUM OF ONE WEEK PRIOR TO THE DATE THAT THE COMPLIANCE LETTER IS NEEDED.



- STRUCTURAL ENGINEER OF RECORD.
- RFPORT
- STATEMENT OF SPECIAL INSPECTIONS PER SECTION 1705. PROVIDED BY THE STRUCTURAL ENGINEER.

INSPECTOR, RETAINED BY THE OWNER, IN ACCORDANCE WITH THE FOLLOWING SECTIONS OF IBC CHAPTER 17: A. SECTION 1704 SPECIAL INSPECTIONS, CONTRACTOR RESPONSIBILITY, AND STRUCTURAL

B. SECTION 1705 REQUIRED VERIFICATION AND INSPECTION AND THE FOLLOWING SUB-SECTIONS:

6. SECTION 1705.12 SPECIAL INSPECTIONS FOR WIND RESISTANCE AND THE FOLLOWING SUB-

2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THE APPROVED INSPECTOR MUST BE INDEPENDENT FROM THE

3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR SHALL BE TO INSPECT AND/OR TEST THE WORK OUTLINED ABOVE AND WITHIN THE STATEMENT OF SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF

5. PER SECTION 1704.2.4 THE SPECIAL INSPECTOR SHALL FURNISH REGULAR REPORTS TO THE BUILDING OFFICIAL AND THE STRUCTURAL ENGINEER. PROGRESS REPORTS FOR CONTINUOUS INSPECTION SHALL BE FURNISHED WEEKLY. INDIVIDUAL REPORTS OF PERIODIC INSPECTIONS SHALL BE FURNISHED WITHIN ONE WEEK OF INSPECTION DATES. THE REPORTS SHALL NOTE UNCORRECTED DEFICIENCIES, CORRECTION OF PREVIOUSLY REPORTED DEFICIENCIES, AND CHANGES TO THE APPROVED CONSTRUCTION DOCUMENTS AUTHORIZED BY THE

6. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT WITHIN 10 DAYS OF THE FINAL SPECIAL INSPECTION STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC. WORK NOT IN COMPLIANCE SHALL BE NOTED IN THE

7. THE CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON A MAIN WIND- OR SEISMIC-FORCE-RESISTING SYSTEM PER SECTION 1704.4. THE STATEMENT SHALL ACKNOWLEDGE THE AWARENESS OF THE SPECIAL LISTED REQUIREMENTS OF DESIGNATED SEISMIC SYSTEM OR A WIND- OR SEISMIC-RESISTING COMPONENT IN THE

8. EXCEPT AS NOTED, THE SPECIAL INSPECTIONS OUTLINED ABOVE ARE IN ADDITION TO, AND BEYOND THE SCOPE OF, PERIODIC STRUCTURAL OBSERVATIONS AS DEFINED IN SECTION 1704.6. STRUCTURAL OBSERVATIONS ARE INCLUDED IN THE STRUCTURAL ENGINEERING DESIGN AND CONSTRUCTION ADMINISTRATION SERVICES

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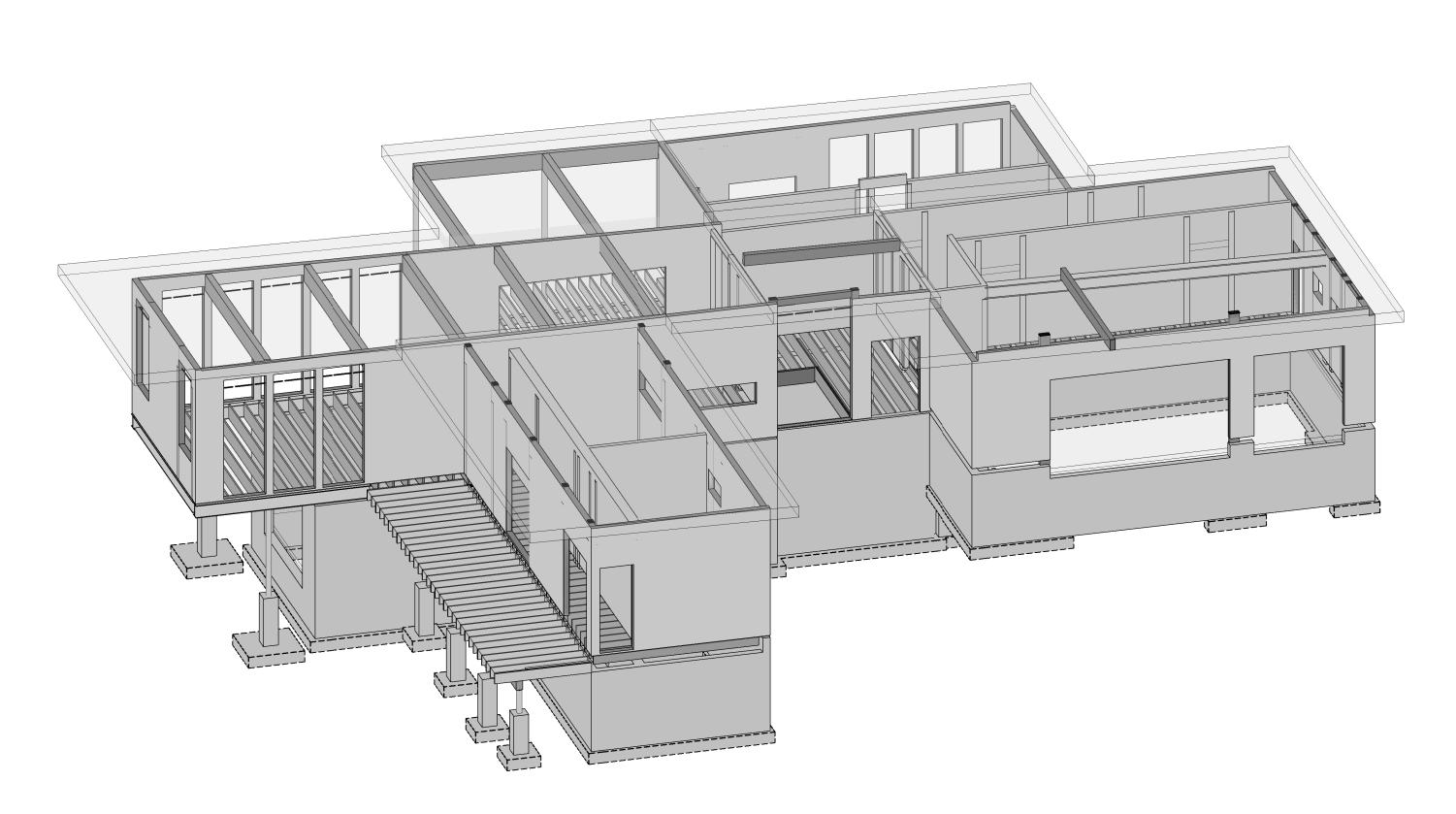
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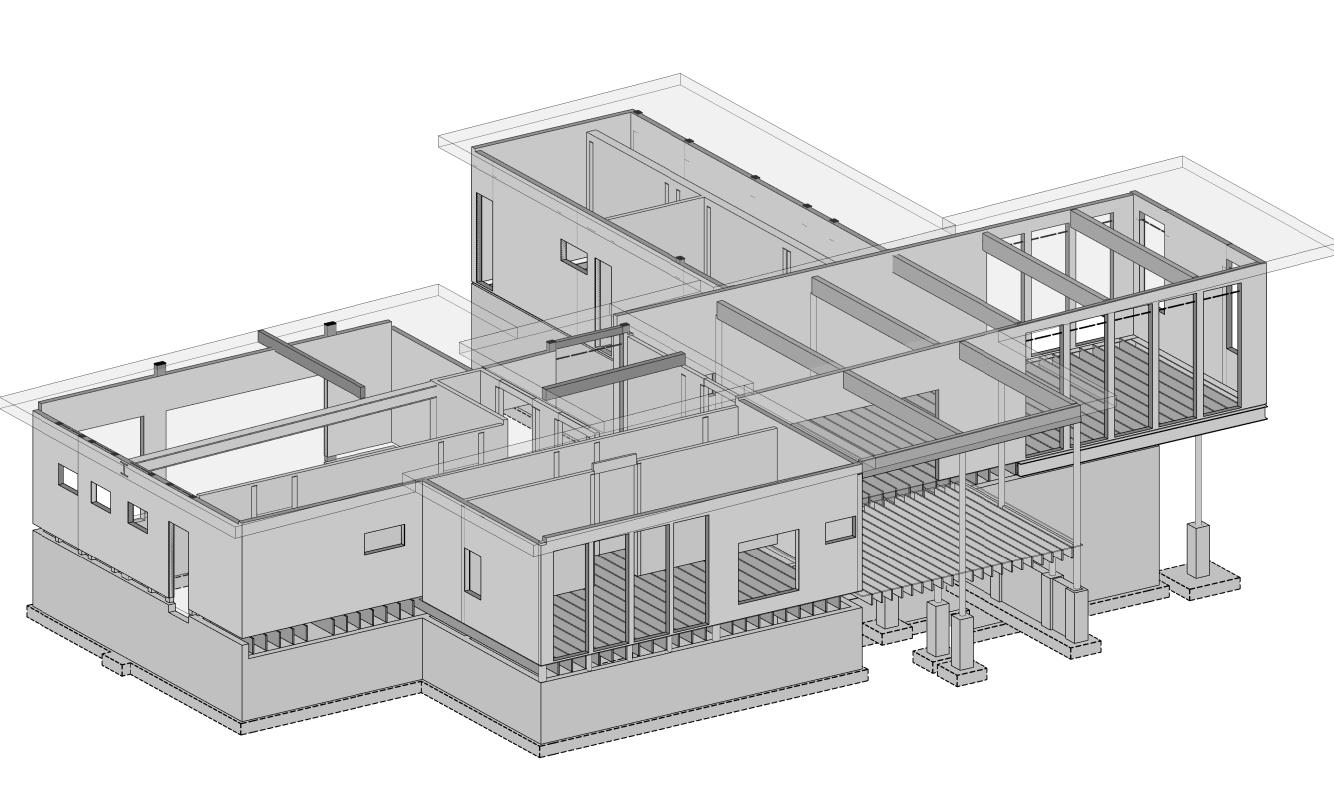
DRAWING TITLE

GENERAL NOTES

SEAL & SIGNATURE DATE 04/19/2024 PROJ NO. 240401 DRAWN BY HMH, LAB CHECKED BY PES DWG NO. S-0.2







3D SCHEMATIC VIEW-BACK

TYPICAL INTERIOR STAIR ASSEMBLY: CONCRETE-FILLED STEEL PANS WITH CLOSED RISERS BETWEEN STRINGERS; DESIGN & DETAILING OF STRINGERS, HEADERS, INTERMEDIATE LANDINGS, & RAILINGS BY STAIR SUPPLIER; COORDINATE STAIR ASSEMBLIES & DETAILS WITH FRAMING ELEMENTS SHOWN ON THESE DRAWINGS AND ARCH DRAWINGS

TYPICAL STAIR ASSEMBLY (EXCEPT AS NOTED): • CONCRETE-FILLED PANS WITH CLOSED RISERS AND

- STRINGERS PER ARCHITECTURAL DRAWINGS
 FRAME LANDINGS WITH CHANNELS OR ANGLES AS REQUIRED. SUPPORT LANDING WITH PIPE OR TUBE STEEL COLUMNS OR HANGERS FROM FOUNDATION OR BEAMS AS REQUIRED TO AVOID INTERFERENCE WITH STRUCTURAL/ARCHITECTURAL ELEMENTS. FRAMING
- SHOWN IS FOR SCHEMATIC PUROPOSES ONLY
 STAIR FABRICATOR SHALL DESIGN & DETAIL ALL MEMBERS,
- CONNECTIONS AND ASSEMBLIES REQIURED FOR FRAMING AND SUPPORT OF STAIRS WHERE NOT SHOWN
- CALCULATIONS, STAMPED AND SIGNED BY A REGISTERED COLORADO PROFESSIONAL ENGINEER, SHALL BE
- SUBMITTED WITH THE STAIR SHOP DRAWINGS
 COORDINATE ALL STAIR ASSEMBLIES AND DETAILS WITH ARCHITECTURAL AND STRUCTURAL DRAWINGS.

			ABBREVIA	FIONS KE	Ϋ́Υ		
@	ON CENTER SPACING	DWG	DRAWING	LGS	LIGHT GAGE STEEL		
(E)	EXISTING	DWL	DOWEL	LL	LIVE LOAD	REINF	REINFORCE, -ED, -ING
(N)	NEW	EA	EACH		LONG LEG HORIZONTAL	REQ	REQUIRED
(R)	REMOVE	ECC	ECCENTRIC	LLV	LONG LEG VERTICAL	REQMT	REQUIREMENT
AB	ANCHOR ROD (BOLT)	E-E	END TO END	LOC	LOCATION	RET	RETAINING
ADDL	ADDITIONAL	EF	EACH FACE	LOU	LOW POINT	RM	ROOM
ADJ	ADJUSTABLE	EJ	EXPANSION JOINT	LSL	LAMINATED STRAND LUMBER (GENERIC TERM)	RMO	ROUGH MASONRY OPENING
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	EL	ELEVATION	LT	LIGHT	RO	ROUGH OPENING
AFF	ABOVE FINISHED FLOOR	ELEC	ELECTRIC, ELECTRICAL	LVL	LAMINATED VENEER LUMBER (GENERIC TERM)	SC	SLIP-CRITICAL
ALT	ALTERNATE	EMBED	EMBEDMENT	MACH	MACHINE	SCH	SCHEDULE
AMT	AMOUNT	ENGR	ENGINEER	MASY	MASONRY	SDST	SELF-DRILLING/
AIVIT	AMOUNT	EOR	ENGINEER OF RECORD	IVIA5 I	MASONRT	3031	SELF-TAPPING
ANCH	ANCHOR, ANCHORAGE	EQ	EQUAL	MATL	MATERIAL	SECT	SECTION
APPROX	APPROXIMATE	EQUIP	EQUIPMENT	MAX	MAXIMUM	SF	SQUARE FEET, SUB-FLOOR
ARCH	ARCHITECT, -URAL	EQUIV	EQUIVALENT	MB	MACHINE BOLT	SHT	SHEET
ATR	ALL THREAD ROD	ES	EACH SIDE	MECH	MECHANICAL	SHTG	SHEATHING
AVG	AVERAGE	EST	ESTIMATE	MEZZ	MEZZANINE	SIM	SIMILAR
BC	BOTTOM OF CONCRETE	E-W	EAST TO WEST	MFR	MANUFACTURE, -ER, -ED	SLH	SHORT LEG HORIZONTAL
BL	BRICK LEDGE	EXC	EXCAVATE	MIN	MINIMUM	SLV	SHORT LEG VERTICAL
					MICROLLAM (TRUS-JOIST		
BLK	BLOCK	EXP	EXPANSION	ML	BRAND LVL), MASONRY LINTEL	SOG	SLAB ON GRADE
BLKG	BLOCKING	EXT	EXTERIOR	MO	MASONRY OPENING	SP	SPACES, SPACED
BM	BEAM	FD	FLOOR DRAIN	MTL	METAL	SPEC	SPECIFICATIONS
BOT	BOTTOM	FDN	FOUNDATION	NF	NEAR FACE	SQ	SQUARE
BRG	BEARING	FF	FINISHED FLOOR, FAR FACE	NIC	NOT IN CONTRACT	SSR	SHEAR STUD RAIL
BW	BOTTOM OF WALL	F-F	FACE TO FACE	NS	NEAR SIDE	ST	SNUG-TIGHT
CB	COUNTERBORE	FIG	FIGURE	N-S	NORTH TO SOUTH	STD	STANDARD
CF	CUBIC FOOT	FL	FLUSH	NTS	NOT TO SCALE	STIFF	STIFFENER
CFS	COLD-FORMED STEEL	FLG	FLANGE	OCJ	OSHA COLUMN JOIST	STL	STEEL
CG	CENTER OF GRAVITY	FLR	FLOOR	OD	OUTSIDE DIAMETER		STRUCTURE, -AL
CIP	CAST-IN-PLACE	FO	FACE OF	OF	OUTSIDE FACE	SUPT	SUPPORT
CJ	CONSTRUCTION JOINT, CONTROL JOINT	FP	FULL PENETRATION	OH	OPPOSITE HAND	SY	SQUARE YARD
CJP	COMPLETE JOINT PENETRATION	FS	FOOTING STEP, FAR SIDE	OPNG	OPENING	SYM	SYMMETRICAL
CL	CENTER LINE	FTG	FOOTING	OPP	OPPOSITE	T&B	TOP AND BOTTOM
CLG	CEILING	GA	GAGE, GAUGE	OSB	ORIENTED STRAND BOARD	T&G	TONGUE AND GROOVE
CLR	CLEAR	GALV	GALVANIZED	PAF	POWDER ACTUATED FASTENER	TB	TOP OF BEAM
СМ	CONSTRUCTION MANAGER, -MENT	GC	GENERAL CONTRACTOR	PC	PRECAST	TC	TOP OF CONCRETE TORQUE-CONTROLLED
CMU	CONCRETE MASONRY UNIT	GEN	GENERAL	PCF	POUNDS PER CUBIC FOOT	TCA	ANCHOR
COL	COLUMN	GL	GLUED LAMINATED, GLULAM		PRE-ENGINEERED	TD	TOP OF DECK
COM	COMMON	GND	GROUND	PEN	PENETRATION	THD	THREAD
COMB	COMBINATION	GR	GRADE	PERP	PERPENDICULAR	THK	THICK, -NESS
CONC	CONCRETE	GT	GIRDER TRUSS	PJP	PARTIAL JOINT PENETRATION	TJ	TOP OF JOIST
CONN	CONNECTION	GYP BD	GYPSUM BOARD	PL	PLATE	TL	TOTAL LOAD
CONT	CONTINUOUS, CONTINUE	HAS	HEADED ANCHOR STUD	PLF	POUND PER LINEAR FOOT	TPG	TOPPING
COORD	COORDINATE, COORDINATION	HDG	HOT-DIP GALVANIZED	PNL	PANEL	TRANS	
CS	COUNTERSINK	HDR	HEADER	PP	PANEL POINT	TW	TOP OF WALL
CTR	CENTER	HORIZ	HORIZONTAL	PS	PRESTRESSED	TYP	TYPICAL
CY	CUBIC YARD	HP	HIGH POINT	PSF	POUNDS PER SQUARE FOOT	ULT	
DAB DET	DEFORMED ANCHOR BAR	HT ID	HEIGHT INSIDE DIAMETER	PSI PSL	POUNDS PER SQUARE INCH PARALLEL STRAND LUMBER	UNO VERT	UNLESS NOTED OTHERWISE VERTICAL
DEV	DEVELOP	IF	INSIDE FACE	PT	(GENERIC TERM) POST TENSIONED, DRESSURE TREATED	VIF	VERIFY IN FIELD
					PRESSURE TREATED		
DIAG	DIAGONAL	INT		PTN	PARTITION	WP	WORK POINT
DIM	DIMENSION	IT		PWD	PLYWOOD	WT	WEIGHT
DL	DEAD LOAD	JB	JOIST BEARING	QTY	QUANTITY	WWF	WELDED WIRE FABRIC
DN	DOWN	JST	JOIST	R	RADIUS	XS	EXTRA STRONG
DP	DRILLED PIER	JT	JOINT	RE	REFERENCE, REFER TO	XSECT	CROSS SECTION
DT	DOUBLE TEE	K	KIP (1,000 LBS)	RECT	RECTANGLE	XXS	DOUBLE EXTRA STRONG

SYMBOLS KEY								
	DIRECTION OF DECK SPAN		/ XXX'-X	TOP OF CONCRETE OR MASONRY ELEVATION			WOOD BEARING WALL	
(GRID)	GRID DESIGNATION	-					WOOD SHEAR WALL	
	REVISION	-		STEP TOP OF WALL		A	COLUMN <u>ABOVE</u>	
	REVISION		BL XXX'-X	BRICK LEDGE ELEVATION		B	COLUMN BELOW	
	INDICATES STRUCTURAL ELEVATION	-	(XXX'-X)	TOP OF FOOTING ELEVATION				
SWx	SHEAR WALL	-	$\Phi_{\underline{XXX-X}}$	TOP OF FLOOR ELEVATION	NS	CXX	COLUMN OR OTHER ELEMENT BELOW SEE SCHEDULES & NOTES	
\bigtriangleup	SHORING		(XXX'-X)	TOP OF FOOTING ELEVATION	IATIO			
77777	STEP IN FLOOR ELEVATION		B	INDICATES BRACED BAY MARK	DESIGN		Cx = COLUMN BPx = BASE PLATE	
	BRICK		X SX	INDICATES BRACED BAY ELEVATION	COLUMN DESIGNATIONS	CONT	COLUMN CONTINUOUS FROM LEVEL BELOW	
	CIP CONCRETE	SYMBOLS	\	INDICATES BRACED MEMBER (ON PLAN)	BUILDING C	XXX	"X" NUMBER OF KING STUDS BELOW "Y" NUMBER OF TRIMMER STUDS BELOW	
			\square	INDICATES CONFIGURATION OF INVERTED CHEVRON-TYPE BRACED BAY WITH HSS DIAGONAL BRACES	BU	WPXY	"WP" = WOOD POST "X" = NUMBER OF STUDS	
1 4 4 · · · A	EXISTING CONCRETE	ZAME		INDICATES CONFIGURATION OF			"Y" = NOMINAL STUD DIMENSION "LVL" = LAMINATED VENEER LUMBER	
	EARTH	BRACED/FRAME BAY		SINGLE DIAGONAL BRACED BAY WITH HSS DIAGONAL BRACE		LVLXY	"X" = NUMBER OF PLY'S "Y" = WIDTH OF LVL	
FX.X	ISOLATED SPREAD FOOTING MARK	BR	RF	INDICATES RIGID (MOMENT) FRAME WITH FULL PENETRATION WELDED BEAM FLANGE		□◆ HDx		
FXX	SPREAD FOOTING MARK			TO COLUMN CONNECTIONS			HOLDOWN	
STEP	STEP IN BOTTOM OF WALL/GRADE BEAM	X SX	X SX	INDICATES RIGID (MOMENT) FRAME ELEVATION W/ FULL PENETRATION WELDED BEAM FLANGE TO COLUMN CONNECTIONS		 ا ک	WOOD HEADER	
XX:12	ROOF SLOPE			INDICATES BRACED BAY OR FRAMED			WOOD JOIST OR BEAM SUPPORTED BY METAL	
SLOPE	DIRECTION OF SLOPE (DOWN)		SLIDING S	BAY COLUMN BASE			HANGER	
	STAIR OR RAMP DIRECTION	TOTAL WIDTH,		W			WOOD JOIST CONTINUOUS	
		p _f + p _d - XX psf SL		XX psf SL			OVER INTERMEDIATE SUPPORT	
		SLIDING w=X XX psf SL BALANCED XX psf DL SNOW LOAD,pf					WOOD JOIST BEARING ON TOP OF SUPPORT	

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No.	Description	Date
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PROJECT NAME

INGRAM STAGECOACH

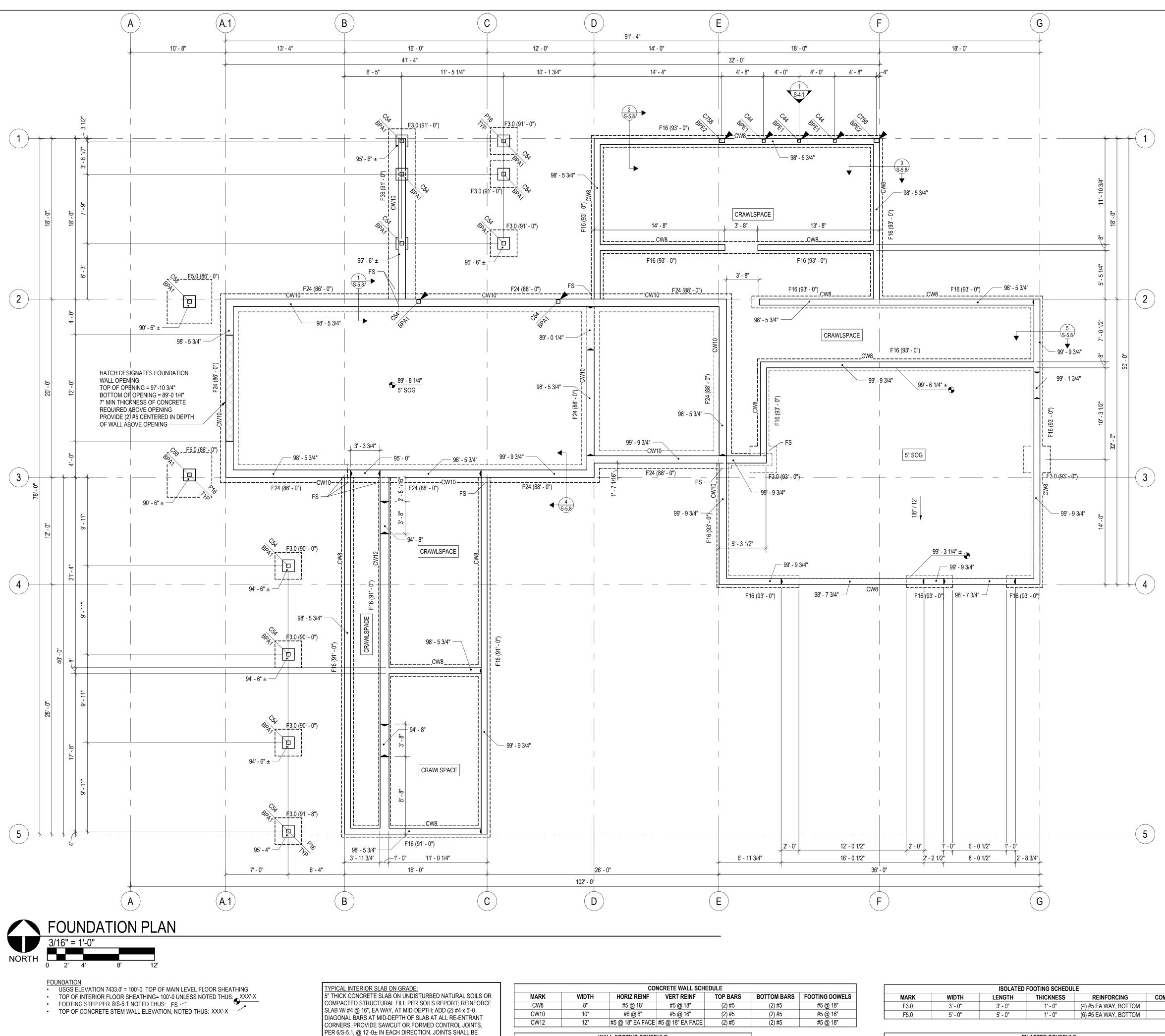
DRAWING TITLE

ABBREVIATION, SYMBOLS KEY & 3D SCHEMATIC VIEWS

SEAL & SIGNATURE

DATE04/19/2024PROJ NO.240401DRAWN BYHMH, LABCHECKED BYPES, MESDWG NO.

S-0.3



TYPICAL INTERIOR SLAB ON GRADE:			CO	NCRETE WALL SCH	IEDULE						ISOLATED	FOOTING SCHEDU	LE	
5" THICK CONCRETE SLAB ON UNDISTURBED NATURAL SOILS OR	MARK	WIDTH	HORIZ REINF	VERT REINF	TOP BARS	BOTTOM BARS	FOOTING DOWELS	MARK		WIDTH	LENGTH	THICKNESS	REINFORCING	COMMENTS
COMPACTED STRUCTURAL FILL PER SOILS REPORT; REINFORCE	CW8	8"	#5 @ 18"	#5 @ 18"	(2) #5	(2) #5	#5 @ 18"	F3.0		3' - 0"	3' - 0"	1' - 0"	(4) #5 EA WAY, BOTTOM	
SLAB W/ #4 @ 16", EA WAY, AT MID-DEPTH; ADD (2) #4 x 5'-0 DIAGONAL BARS AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT	CW10	10"	#6 @ 8"	#5 @ 16"	(2) #5	(2) #5	#5 @ 16"	F5.0		5' - 0"	5' - 0"	1' - 0"	(6) #5 EA WAY, BOTTOM	
CORNERS. PROVIDE SAWCUT OR FORMED CONTROL JOINTS,	CW12	12"	#5 @ 18" EA FACE	#5 @ 18" EA FACE	(2) #5	(2) #5	#5 @ 18"							
PER 6/S-5.1, @ 12'-0± IN EACH DIRECTION. JOINTS SHALL BE						-							_	
		WA	ALL FOOTING SCHEI	DULE]					Р	ILASTER SCHEDUL	.E	
CONTINUOUS, NOT STAGGERED OR OFFSET. ASPECT RATIO SHALL	MARK	WA WIDTH	ALL FOOTING SCHEE THICKNESS	DULE	COMMENTS	-		MARK	WIDTH	DEPTH	P VERTICAL RE		.e Ties	TIE TY
CONTINUOUS, NOT STAGGERED OR OFFSET. ASPECT RATIO SHALL	MARK F16				COMMENTS			MARK P16	WIDTH 1' - 4"	DEPTH 1' - 4"		EINF		
PER 6/S-5.1, @ 12'-0± IN EACH DIRECTION. JOINTS SHALL BE CONTINUOUS, NOT STAGGERED OR OFFSET. ASPECT RATIO SHALL BE A MAXIMUM OF 1.5 TO 1.		WIDTH	THICKNESS	REINFORCING	COMMENTS					-	VERTICAL RE	EINF	TIES	

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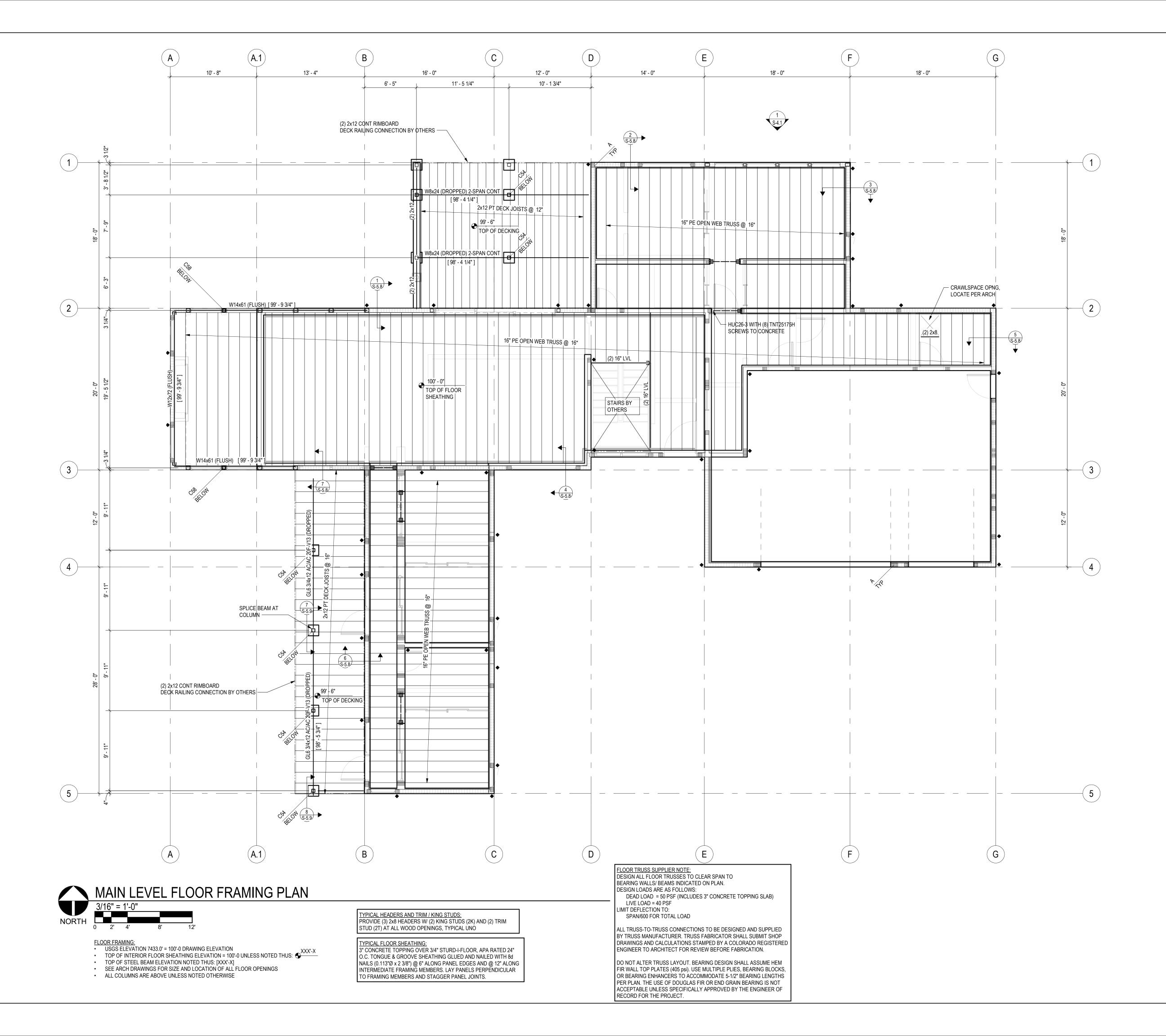
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FOUNDATION PLAN

SEAL & SIGNATURE	DATE	04/19/2024
	PROJ NO.	240401
	DRAWN BY	HMH, LAB
	CHECKED E	BY PES
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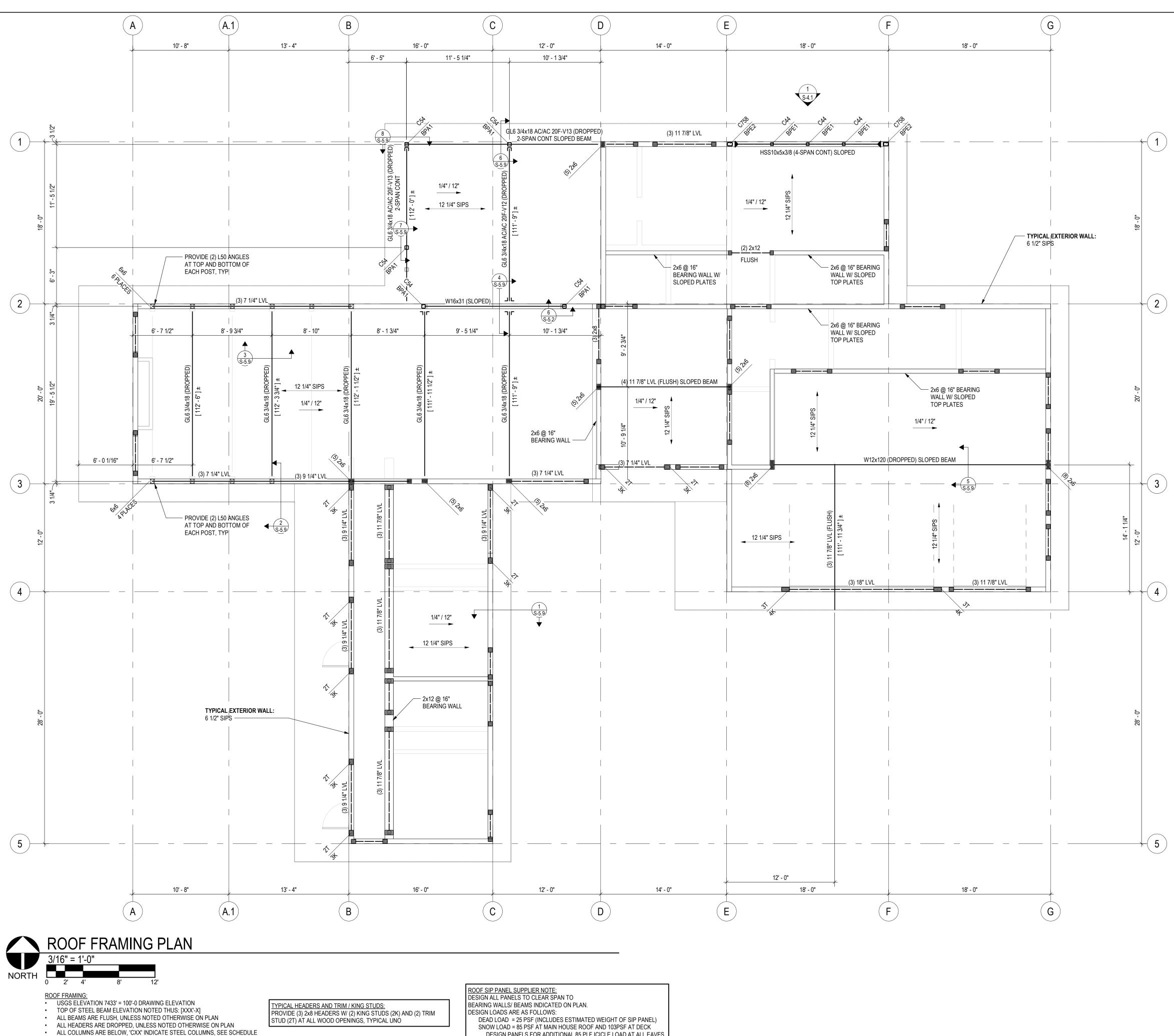
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MAIN LEVEL FLOOR FRAMING PLAN

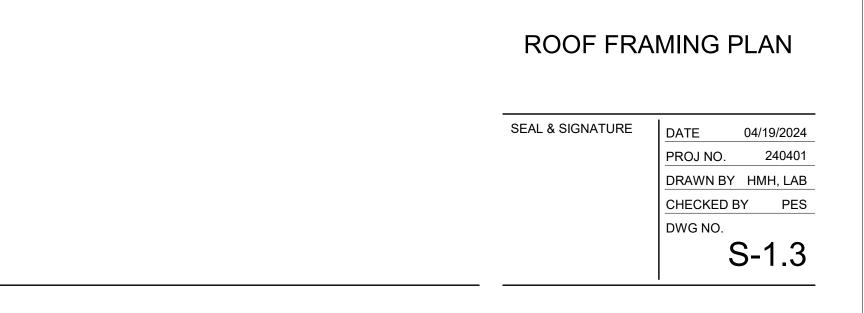
SEAL & SIGNATURE	DATE	04/19/2024
	PROJ NO.	240401
	DRAWN BY	HMH, LAB
	CHECKED E	Y PES
	DWG NO.	
		5-12



DESIGN PANELS FOR ADDITIONAL 85 PLF ICICLE LOAD AT ALL EAVES LIMIT DEFLECTION TO: SPAN/360 FOR SNOW LOAD SPAN/240 (LIMIT TO 1" MAXIMUM) FOR TOTAL LOAD ALL PANEL-TO-PANEL CONNECTIONS TO BE DESIGNED AND SUPPLIED BY

SIP MANUFACTURER.

ON SHEET 2/S-5.2





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STAGECOACH

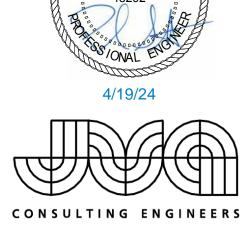
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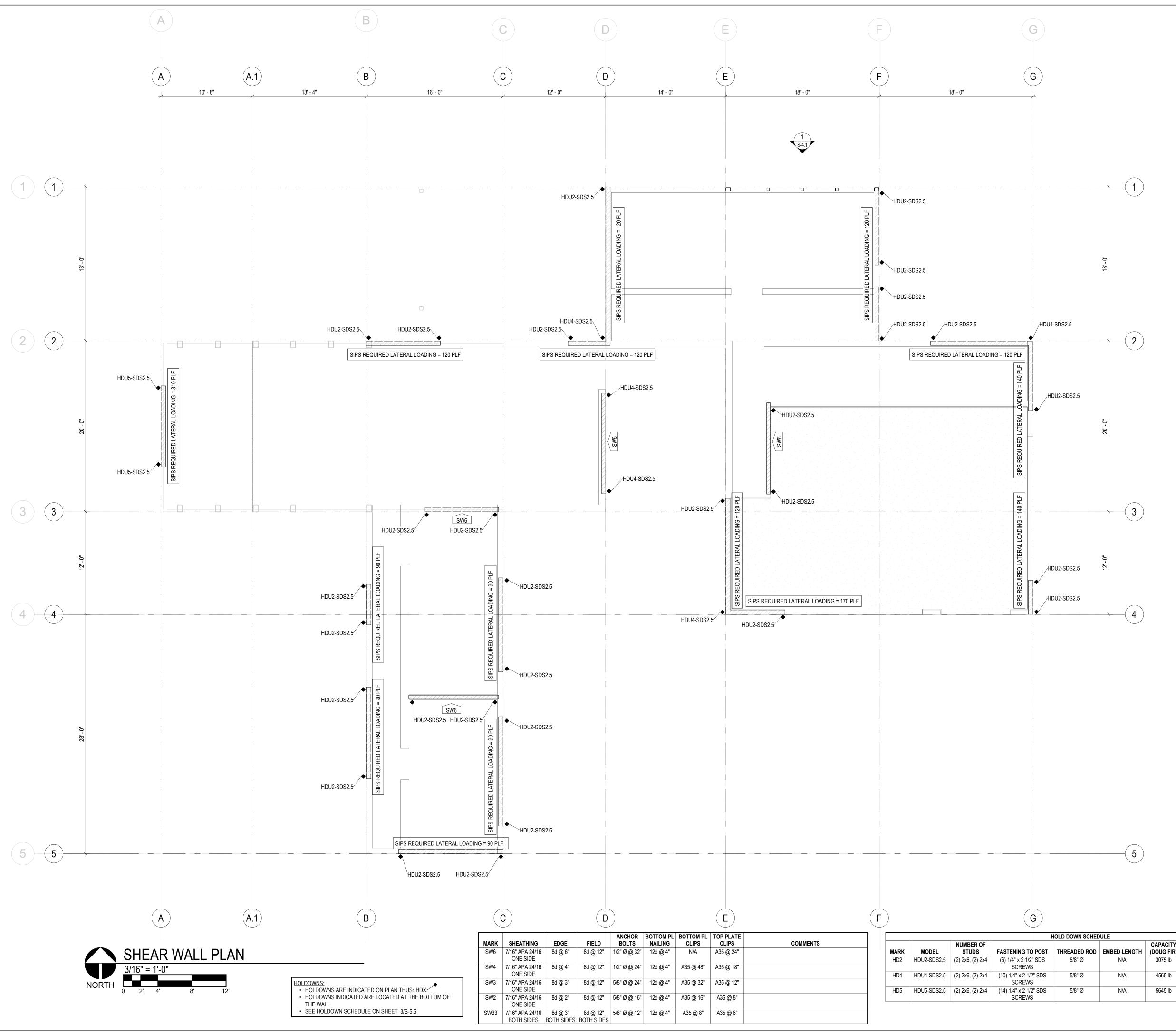
Boulder

Fort Collins
Winter Park

JVA #240401

Reviewed for Code Compliance 05/09/2024





	H	OLD DOWN SCHED	ULE			
BER OF UDS	FASTENING TO POST	THREADED ROD	EMBED LENGTH	CAPACITY (DOUG FIR)	CAPACITY (HEM FIR)	REMARKS
ô, (2) 2x4	(6) 1/4" x 2 1/2" SDS SCREWS	5/8" Ø	N/A	3075 lb	2215 lb	EMBED PL, SEE 4/S-5.5
ô, (2) 2x4	(10) 1/4" x 2 1/2" SDS SCREWS	5/8" Ø	N/A	4565 lb	3285 lb	EMBED PL, SEE 4/S-5.5
ô, (2) 2x4	(14) 1/4" x 2 1/2" SDS SCREWS	5/8" Ø	N/A	5645 lb	4340 lb	EMBED PL, SEE 4/S-5.5

05/09/2024



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JVA #240401

No.	Description	Date
	Permit Submission	04/19/2024
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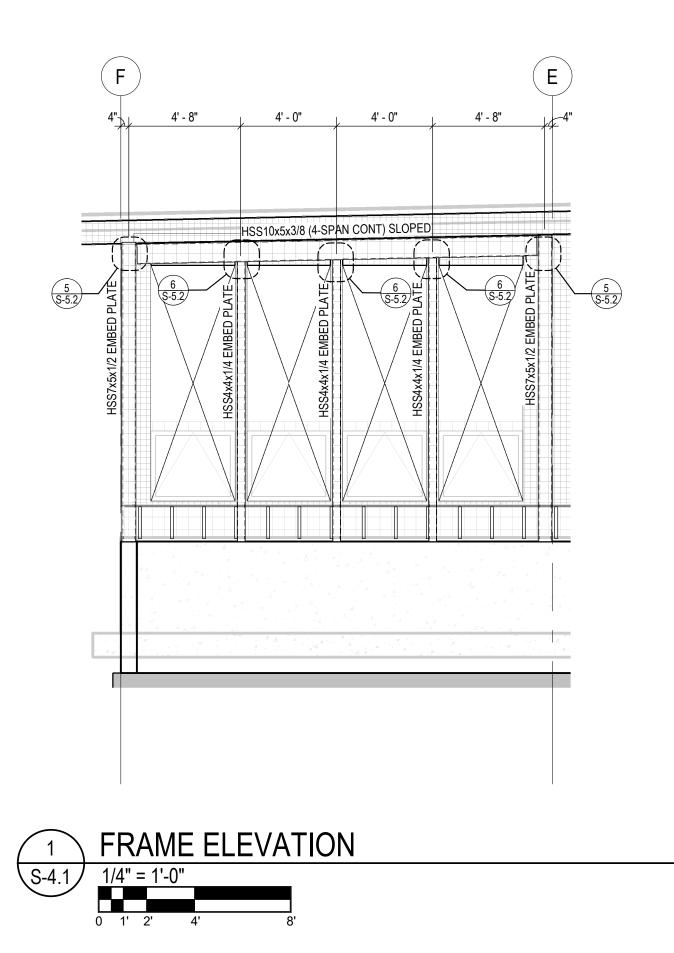
PROJECT NAME

INGRAM STAGECOACH

DRAWING TITLE

SHEAR WALL PLAN

SEAL & SIGNATURE	DATE 04/19/2024
	PROJ NO. 240401
	DRAWN BY HMH, LAB
	CHECKED BY PES
	DWG NO.
	S-1.4



05/09/2024



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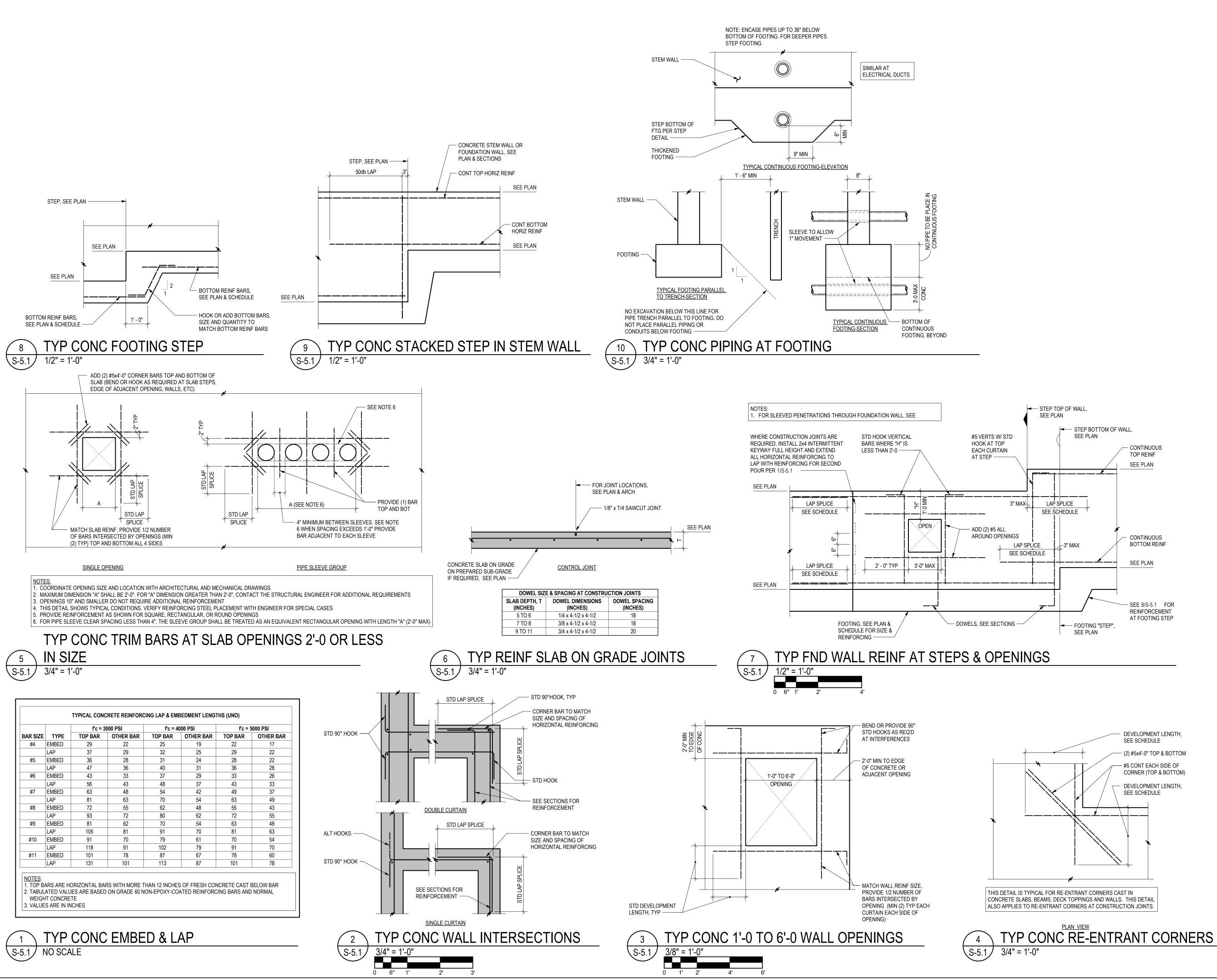
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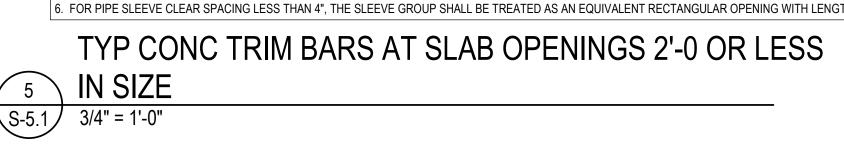
INGRAM STAGECOACH

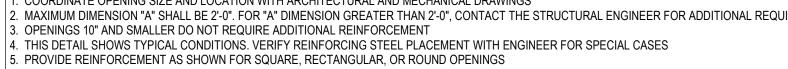
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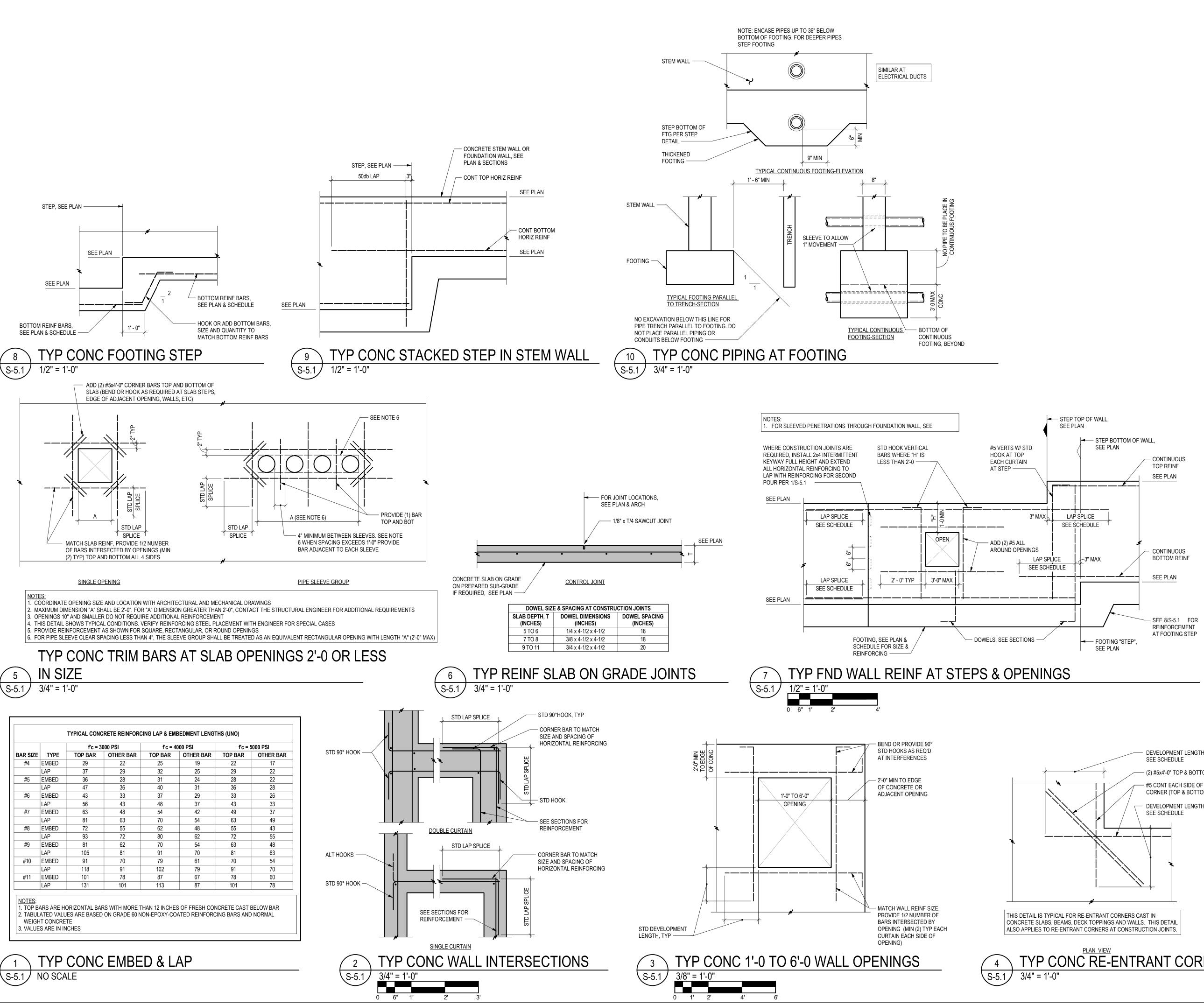
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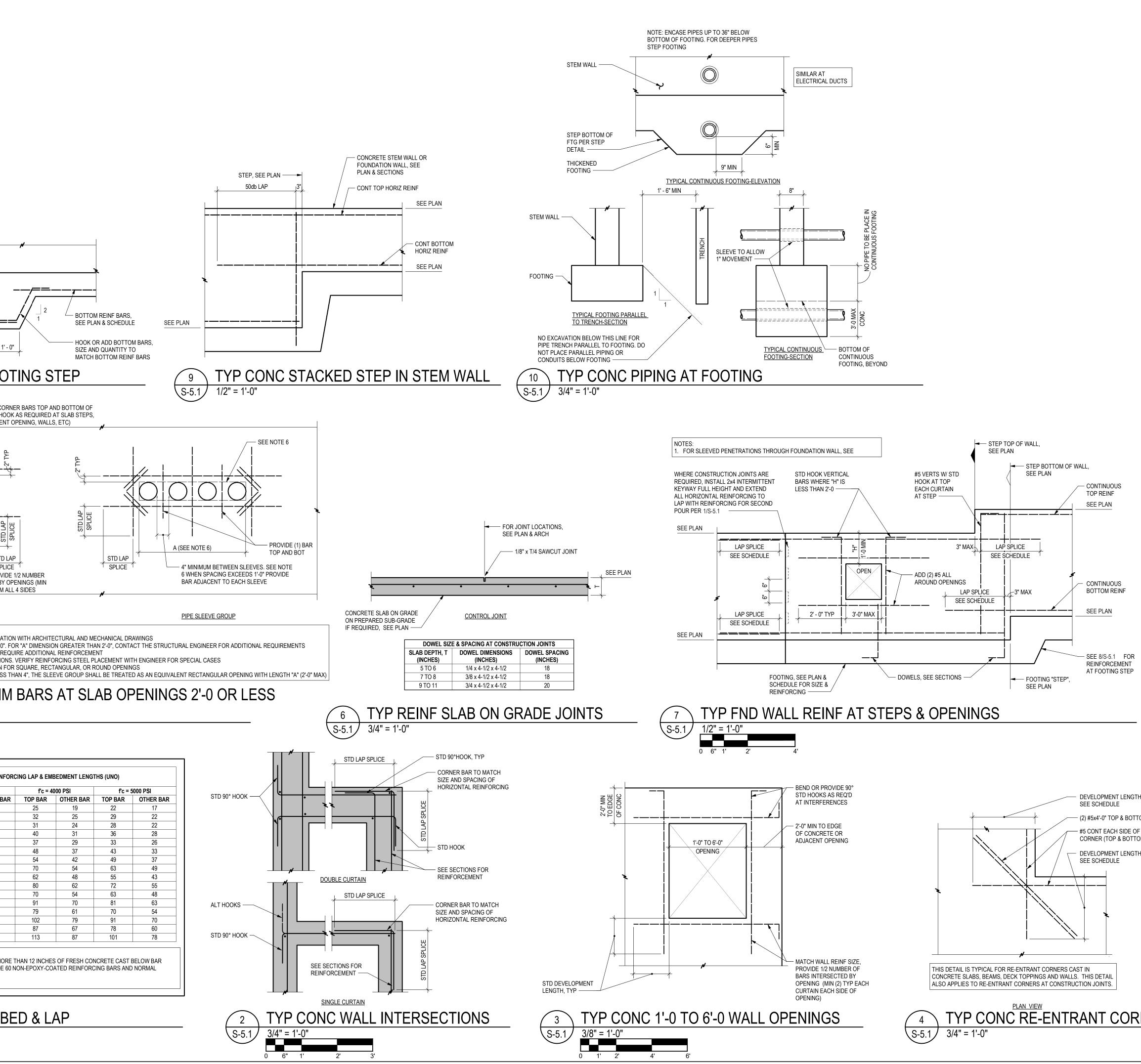
SEAL & SIGNATURE DATE 04/19/2024 PROJ NO. 240401 DRAWN BY HMH, LAB CHECKED BY PES DWG NO. S-4.1













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Description

Permit Submission

JVA #240401

Date

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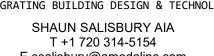
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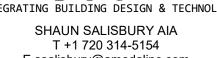
05/09/2024





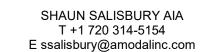












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INGRAM

STAGECOACH

TYPICAL CONCRETE

DETAILS &

SCHEDULES

DATE

PROJ NO.

DWG NO.

DRAWN BY HMH, LAB

CHECKED BY PES

S-5.

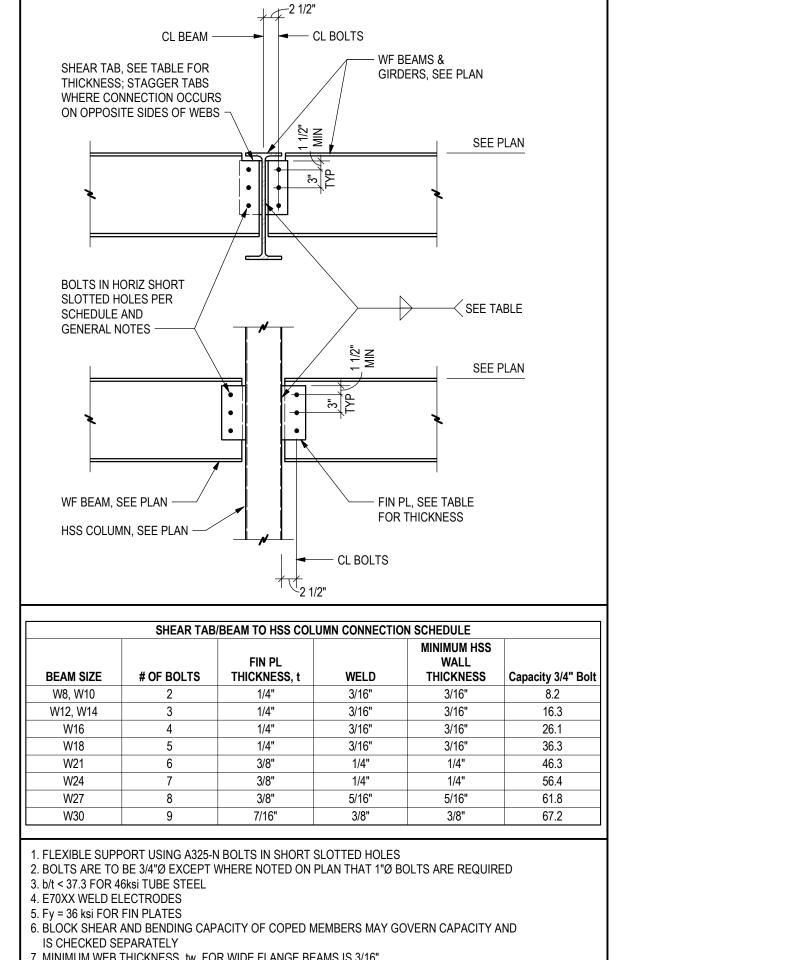
04/19/2024

240401

PROJECT NAME

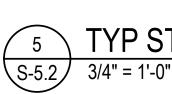
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SEAL & SIGNATURE



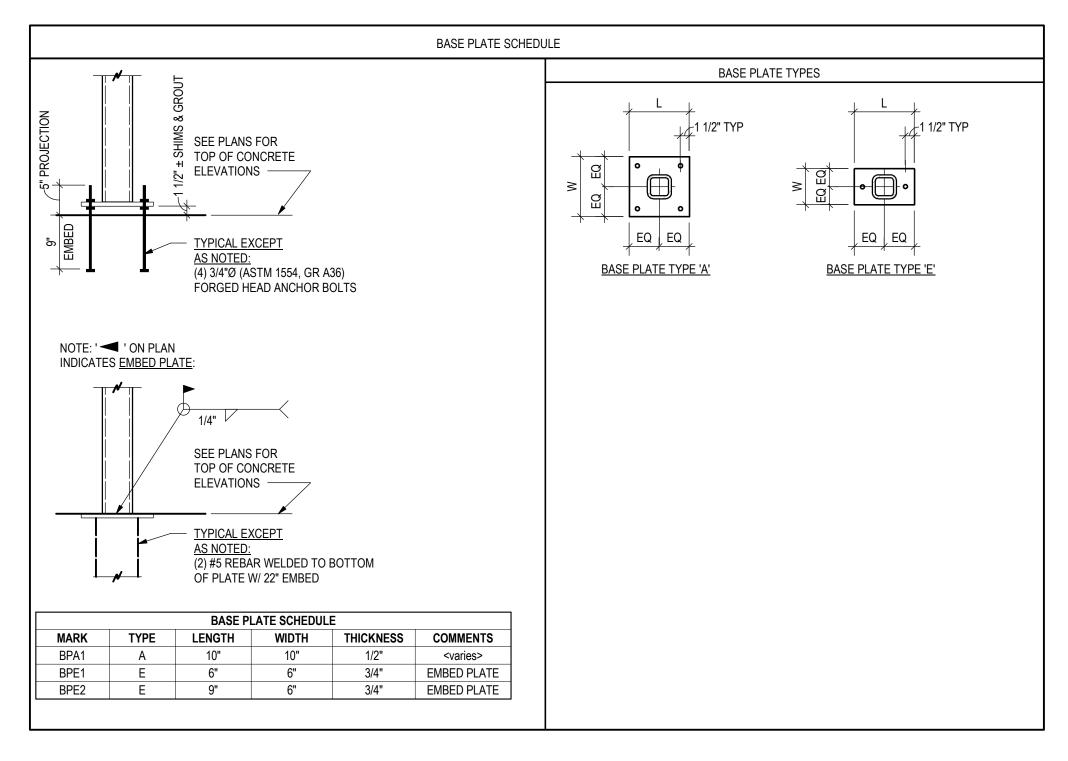
7. MINIMUM WEB THICKNESS, tw, FOR WIDE FLANGE BEAMS IS 3/16" 8. FIN PL THICKNESS IN SCHEDULE SHALL NOT BE INCREASED FOR CONVENIENCE OF FABRICATOR

TYP STL SHEAR TAB/ BM TO HSS COL CONN S-5.2 3/4" = 1'-0"

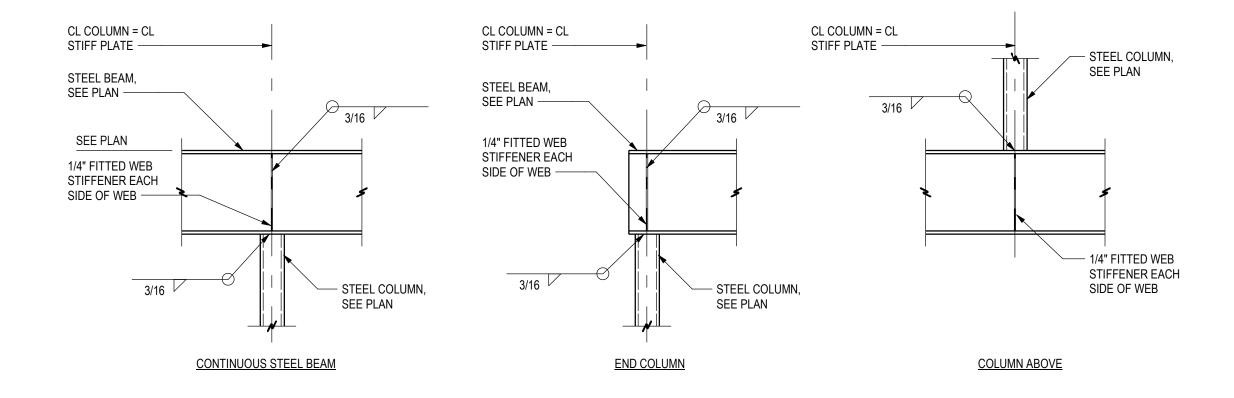


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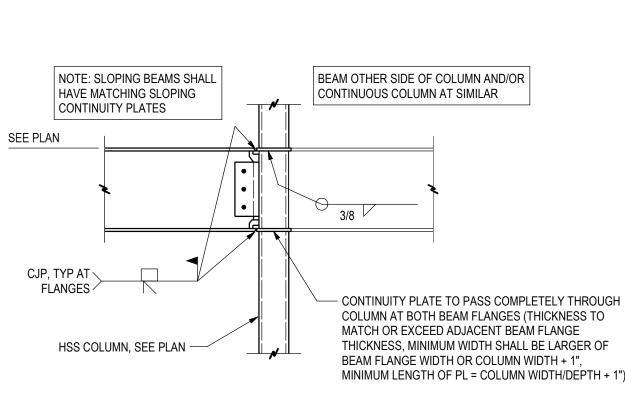
S-5.2/



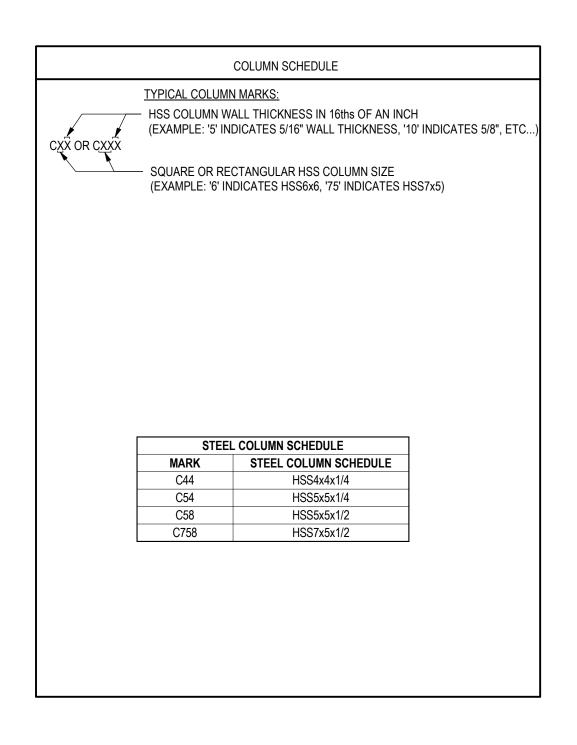
TYP STL BASE PLATE TYPES & SCHEDULE S-5.2/ 3/4" = 1'-0"



STEEL BEAM TO STEEL COLUMN CONNECTIONS 3/4" = 1'-0"



TYP STL RIGID FRAME BM TO HSS COL CONN



OF 3/4"Ø BOLT LENGTH, SEE **BEAM SIZE** NOTE 14 SIZE ROWS 1/4" 3/8" x 6" W8 1/4" W10 3/8" x 6" W12* 3/8" x 9" 1/4" 1/4" W14 3/8" x 9" W16** 3/8" x 12" 1/4" 1/4" W18 3/8" x 12" W21 3/8" x 15" 1/4" 1/4" W24 3/8" x 18" 1/4" W27 3/8" x 21" 3/8" x 24" 1/4" W30 W33 3/8" x 27" 1/4" 1/4" W36 3/8" x 30" W40 3/8" x 33" 1/4" DESIGN BASIS OF SCHEDULE BASED ON AISC 14TH EDITION (ASD) MINIMUM YIELD STRESS FY: PLATES - 36 KSI, W-SHAPES - 50 KSI, HSS SHAPES MINIMUM TENSILE STRESS, FU: PLATES - 58 KSI, W-SHAPES - 65 KSI, HSS SHAP MINIMUM EDGE DISTANCE: VERTICAL EDGE DISTANCE Lev = 1 1/2", HORIZONTAL EDGE DISTANCE Leh = 1 1/2" MINIMUM HSS WALL THICKNESS = 1/4" ALL WELDS ARE TO USE E70XX ELECTRODES. ALL BOLTS ARE TO BE A325-N BEARING TYPE BOLTS, EXCEPT CONNECTIONS DESIGNATED AS 'SC' ON PLANS ARE TO BE CLASS 'A' SLIP-CRITICAL JOINTS WITH F1852 TENSION-CONTROL BOLTS. NOTE THAT VALUES LISTED DO NOT APPLY TO THE SLIP-CRITICAL CONNECTIONS. A325-N BOLTS ARE TO BE FULLY THREADED BEARING TYPE CONNECTIONS AND ARE TO BE INSTALLED TO A 'SNUG-TIGHT' CONDITION AS DEFINED IN THE AISC MANUAL OF STEEL CONSTRUCTION, UNLESS NOTED OTHERWISE. STANDARD HOLES ARE TO BE PROVIDED IN SUPPORTED WEB. SHORT-SLOTTED HOLES TO BE PROVIDED IN SHEAR TAB PLATES, UNLESS NOTED OTHERWISE . BOLT SPACING = 3". 2. BEAM WEB DEPTH FOR DOUBLE COPE CONDITION MUST BE GREATER THAN OR EQUAL TO SHEAR TAB PLATE LENGTH. COPE BEAM ACCORDINGLY. (2" MAXIMUM AT W8-W24 AND 3" MAXIMUM AT W24-W40). . DETAILER TO INFORM ENGINEER OF COPE CONDITIONS NOT COMPLYING WITH CONDITIONS SHOWN IN DETAILS BELOW. 14. FOR SKEWED SHEAR TAB PLATE CONNECTIONS. WELD DETAILS ARE TO FOLLOW TABLE 10-14C, AISC 14TH EDITION. 5. SEE DETAILS BELOW FOR ADDITIONAL DETAILING REQUIREMENTS. THE CONNECTION ENGINEER IS TO BE NOTIFIED OF ANY FRAMING CONDITION WHICH DOES NOT MEET THE CRITERIA PROVIDED IN THIS SCHEDULE 5. * PROVIDE 2 BOLTS WITH 6" LONG SHEAR TAB PLATE FOR W12 BEAM AT 'SINGLE COPE 2" CONDITION ONLY 2. ** PROVIDE 3 BOLTS WITH 9" LONG SHEAR TAB PLATE FOR W16 BEAM AT 'SINGLE COPE 2" CONDITION ONLY. 18. N/A INDICATES CONDITION WHERE CONNECTION FRAMING TYPES OUTLINED IN THIS SCHEDULE TO NOT APPLY. 19. FOR ANY COLUMNS LARGER THAN A W18, THE ALTERNATE BEAM TO COLUMN WEB EXTENDED PLATE CONNECTION IS NOT APPLICABLE. 20. AT THE FOLLOWING COLUMNS PROVIDE 3/4" STIFFENER PLATES TOP AND BOTTOM AT THE ALTERNATE BEAM TO COLUMN WEB EXTENDED PLATE CONNECTION: W8X13, W8X15, W10X26, W10X30, W14X43, W14X48, W14X53, W16X57, W18X65 AND W18X71. SINGLE COPE 1 **SINGLE COPE 2** 6 1/2" MAX AT W8-W24 6 1/2" MAX AT W8-W24 6 1/2" MAX AT W8-W24 8 1/2" MAX AT W27-W40 8 1/2" MAX AT W27-W40 8 1/2" MAX AT W27-W40 TO FACE OF COPE -TO FACE OF COPE -TO FACE OF COPE - \rightarrow \rightarrow FACE OF SUPPORTING FACE OF SUPPORTING FACE OF SUPPORTING MEMBER -MEMBER — MEMBER — COPE DEPTH: 2" MAX AT W8-W24 3" MAX AT W27-W40 -SEE NOTE 12

TYP STL BM SHEAR TAB CONN SCHED (ASD)- 3/4 DIA BOLT 3 S-5.2 3/4" = 1'-0"

3" MAX TO FACE

-OF SUPPORTING

+ MEMBER

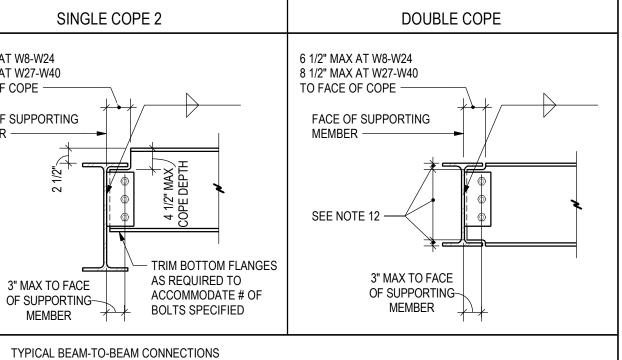
TYP STL COL MARKS & SCHEDULE 2 S-5.2 NO SCALE

AR TAB						
KNESS x GTH, SEE DTE 14	FILLET WELD SIZE	UNCOPED	SINGLE COPE 1	SINGLE COPE 2	DOUBLE COPE	REMARKS
8" x 6"	1/4"	16.5	8.5	N/A	4.7	
8" x 6"	1/4"	16.5	13.4	N/A	5.3	
8" x 9"	1/4"	28.9	21.7	10.3	12.5	
8" x 9"	1/4"	28.9	28.9	19.5	16.7	
5" x 12"	1/4"	41.7	41.7	28.9	27.7	
5" x 12"	1/4"	41.7	41.7	41.7	41.7	
5" x 15"	1/4"	54.2	54.2	54.2	54.2	
5" x 18"	1/4"	66.7	66.7	66.7	66.7	
5" x 21"	1/4"	78.7	78.7	78.7	78.7	
5" x 24"	1/4"	91.3	91.3	91.3	91.3	
5" x 27"	1/4"	103.3	103.3	103.3	103.3	
5" x 30"	1/4"	115.3	115.3	115.3	115.3	
5" x 33"	1/4"	126.7	126.7	126.7	126.7	

; -	46	KS	
PE	S -	58	KS

SHEAR TAB

THICKNESS x



INTEGRATING BUILDING DESIGN & TECHNOLOGY
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PROJECT NAME

INGRAM STAGECOACH

DRAWING TITLE



SEAL & SIGNATURE DATE 04/19/2024 PROJ NO. 240401 DRAWN BY HMH, LAB CHECKED BY PES DWG NO. S-5.2

Reviewed for Code Compliance

05/09/2024



CONSULTING ENGINEERS

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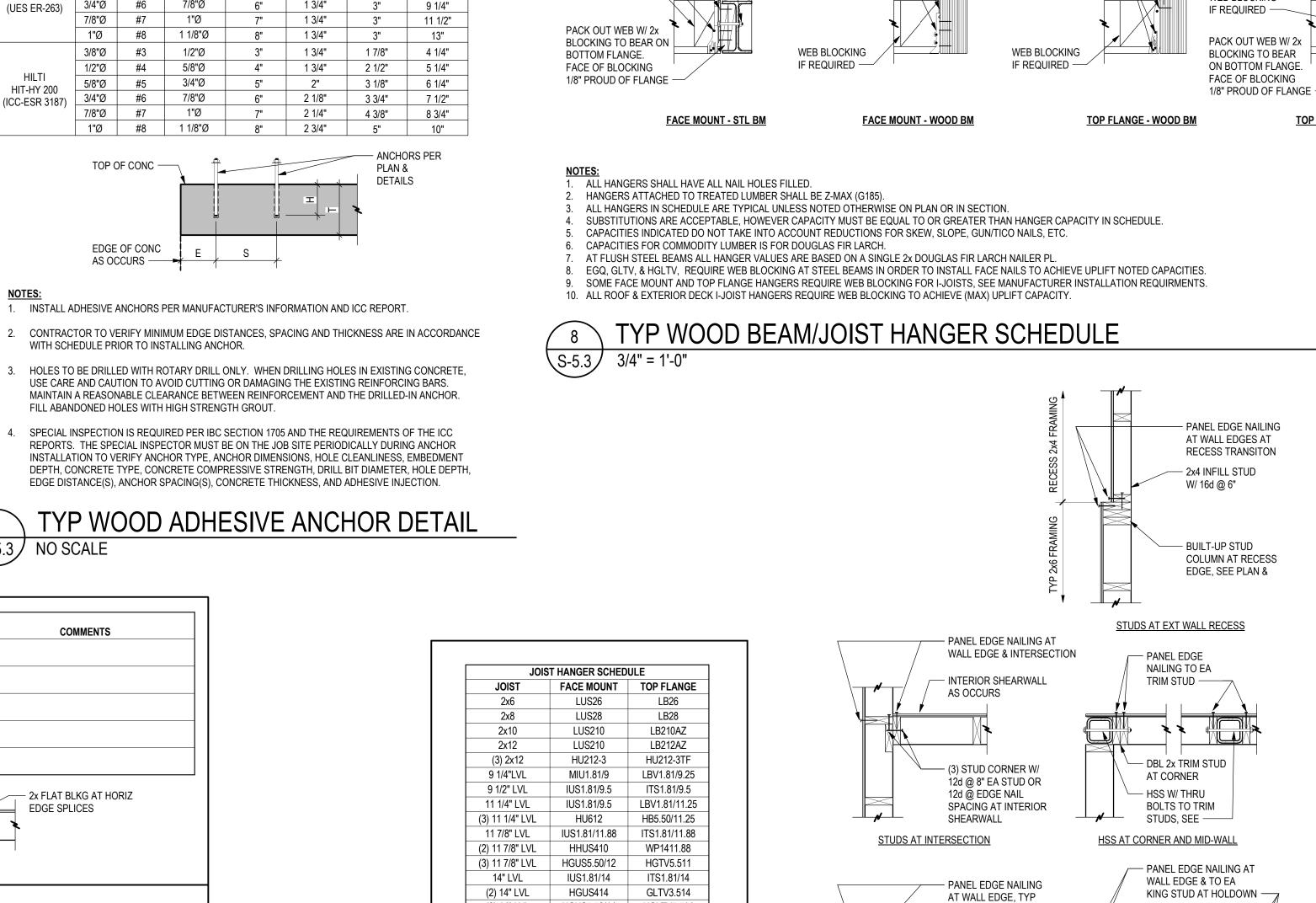
TYP WOOD FASTENING SCHED 6 \<u>S-</u>; S-5.3/ NO SCALE ANCHOR | BOTTOM PL | BOTTOM PL | TOP PLATE FIELD COMMENTS MARK SHEATHING EDGE BOLTS CLIPS NAILING CLIPS SW6 7/16" APA 24/16 8d @ 6" 8d @ 12" 1/2" Ø @ 32" N/A 12d @ 4" A35 @ 24' ONE SIDE SW4 7/16" APA 24/16 8d @ 4" | 8d @ 12" | 1/2" Ø @ 24" | 12d @ 4" | A35 @ 48" | A35 @ 18' ONE SIDE SW3 7/16" APA 24/16 8d @ 3" 8d @ 12" | 5/8" Ø @ 24" | 12d @ 4" | A35 @ 32" | A35 @ 12' ONE SIDE SW2 7/16" APA 24/16 8d @ 2" 8d @ 12" 5/8" Ø @ 16" 12d @ 4" A35 @ 16" A35 @ 8" ONE SIDE SW33 7/16" APA 24/16 8d @ 3" 8d @ 12" 5/8" Ø @ 12" 12d @ 4" A35 @ 8" A35 @ 6" BOTH SIDES BOTH SIDES BOTH SIDES EDGE NAIL EA STUD - 2x FLAT BLKG AT HORIZ EDGE SPLICES DBL STUD AT VERT ⋬┲ EDGE SPLICES -____ V V 16d @ 2" -AT VERT EDGES AT HORIZ EDGES 'SW33' PANEL EDGE NAILING DETAIL TYPICAL FOR ALL SHEAR WALL NAILING: PER IBC / AWC SDPWS, SHEATHING NAILS SHALL BE DRIVEN FLUSH BUT SHALL NOT FRACTURE THE SURFACE OF THE SHEATHING. SHEATHING PANEL NAILING NOT CONFORMING TO THIS SECTION WILL NOT BE ACCEPTABLE AND WILL HAVE TO BE REINSTALLED. IT IS THE CONTRACTOR'S RESPONSIBILITY ENSURE THAT THE NAIL GUNS USED FOR FASTENING ARE SET AT THE PROPER DEPTH AND/OR AIR PRESSURE TO ACHIEVE THE REQUIRED PENETRATION GENERAL NOTES 229"x3" PLATE WASHER DETAIL 1. VALUES ARE BASED ON DOUGLAS FIR-LARCH FRAMING, SEE GENERAL NOTES - PLATE WASHER, 2. SEE PLAN FOR HOLDOWN TYPE AND LOCATION DIAGONALLY 3. UNLESS NOTED OTHERWISE, NUMBER OF STUDS AT EACH END OF SHEAR WALLS IS CALLED OUT ON PLAN SLOTTED HOLES ARE ACCEPTABLE 4. NO PENETRATIONS GREATER THAN 12"x12" IN SHEAR WALLS, BLOCK AND NAIL ALL EDGES UP TO 3/16 " 5. NO MECHANICAL OR PLUMBING PENETRATIONS IN TOP AND BOTTOM PLATES LARGER THAN 6. ALL EDGES SHALL BE BLOCKED WITH 2x MEMBERS AT PLYWOOD/OSB SHEATHED WALLS ALL WALLS HAVE (2) 2x TOP PLATES AND (1) 2x BOTTOM PLATE EQUAL TO WIDTH OF STUD SIZE, TYP UNO - HOLE DIAMETER, SLOT LENGTH NOT 8. MINIMUM WIDTH OF SHEATHING PANELS AT ENDS OF SHEAR WALLS SHALL BE 4'-0 TO ENSURE END STUDS TO EXCEED 1 3/4" ARE ENGAGED BP OR BPS MAY BE 9. SEE DETAILS FOR ATTACHMENT OF DIAPHRAGMS TO SHEARWALL PLATES, TYPICAL USED

CONNECTION	FASTENING	LOCATION
1. JOIST/RAFTER/BLOCKING TO BEARING SUPPORT	(3) 8d COMMON [OR] (3) 12d SINKER	TOENAIL
2. BRIDGING/BLOCKING TO JOIST	(2) 8d COMMON [OR] (2) 12d SINKER	TOENAIL, EACH END
6. SOLE PLATE TO JOIST OR BLOCKING	16d COMMON @ 16" [OR] 12d SINKER @ 8"	FACE NAIL
7. STUD TO TOP AND SOLE PLATE	(2) 16d COMMON [OR] (3) 12d SINKER	END NAIL
8. STUD TO SOLE PLATE	(4) 8d COMMON [OR] (4) 12d SINKER	TOENAIL
9. DOUBLE STUDS AND BUILT-UP CORNER STUDS	16d COMMON @ 24" [OR] 12d SINKER @ 8"	FACE NAIL, EACH STUD
10. DOUBLE TOP PLATES	16d COMMON @ 16" [OR] 12d SINKER @ 12"	TYPICAL FACE NAIL
	(8) 16d COMMON [OR] (12) 12d SINKER	LAP SPLICE FACE NAIL
12. RIM JOIST AND JOIST BLOCKING TO TOP PLATE	8d COMMON @ 6" [OR] 12d SINKER @ 6"	TOENAIL
13. TOP PLATE INTERSECTION	(2) 16d COMMON [OR] (3) 12d SINKER	FACE NAIL
14. BUILT UP HEADER	16d COMMON @ 16" [OR] 12d SINKER @ 12"	FACE NAIL ALONG EACH EDGE, EACH 2x
16. HEADER TO KING STUD	(4) 8d COMMON" [OR] (4) 12d SINKER	TOENAIL
	.9.1 FOR MORE INFORMATION PICAL UNLESS NOTED OTHER	

WOOD FRAMING FASTENING SCHEDULE

7	TYP WOOD ADHESIVE ANCI
5-5.3	NO SCALE

- FILL ABANDONED HOLES WITH HIGH STRENGTH GROUT.
- WITH SCHEDULE PRIOR TO INSTALLING ANCHOR.
- NOTES:



ADHESIVE	ANCHOR		PILOT	MIN EMBED	MIN EDGE
TYPE	THRD ROD	REBAR	HOLE	UNO H	DISTANCE
	3/8"Ø	#3	1/2"Ø	3"	1 3/4"
	1/2"Ø	#4	5/8"Ø	4"	1 3/4"
SIMPSON AT-XP	5/8"Ø	#5	3/4"Ø	5"	1 3/4"
(UES ER-263)	3/4"Ø	#6	7/8"Ø	6"	1 3/4"
	7/8"Ø	#7	1"Ø	7"	1 3/4"
	1"Ø	#8	1 1/8"Ø	8"	1 3/4"
	3/8"Ø	#3	1/2"Ø	3"	1 3/4"
	1/2"Ø	#4	5/8"Ø	4"	1 3/4"
HILTI HIT-HY 200	5/8"Ø	#5	3/4"Ø	5"	2"
(ICC-ESR 3187)	3/4"Ø	#6	7/8"Ø	6"	2 1/8"
	7/8"Ø	#7	1"Ø	7"	2 1/4"
	1"Ø	#8	1 1/8"Ø	8"	2 3/4"

ADHESIVE ANCHOR IN 2500 PSI MIN & 21 DAY AGE MIN CONCRETE

MIN

S

3"

3"

MIN CONC

Т

5"

6 1/2"

8 1/4"

(3) 14" LVL

9 1/2" TJI 210

11 7/8" TJI 210

(2) 11 7/8" TJI 210

14" TJI 210

16" TJI 210

14" TJI 360

11 7/8" TJI 360

16" TJI 360

GENERAL NOTES:

S-5.3 NO SCALE

2

HGUS5.50/14

IUS2.06/9.5

IUS2.06/11.88

HU4.28/11

IUS2.06/14

IUS2.06/16

IUS2.37/14

IUS2.37/11.88

1. ALL HANGERS SHALL HAVE ALL NAIL HOLES FILLED

IUS2.37/16

2. HANGERS ATTACHED TO TREATED LUMBER SHALL BE Z-MAX (G185)

TYP WOOD JOIST HANGER

B. TABLE APPLIES TO JOISTS ONLY SEE PLANS FOR BEAM HANGERS

HGLTV5.514

ITS2.06/9.5

ITS2.06/11.88

LBV4.28/11.88

ITS2.06/14

ITS2.06/16

ITS2.37/14

ITS2.37/11.88

ITS2.37/16

SPACING THICKNESS

TYP WOOD SHEAR WALL NAILING & SCHEDULE

S-5.3 NO SCALE

FACE MOUNT HANGERS				TOP FLANGE HANGE	RS			
-			FAST	ENERS			FASTE	NERS
BEAM/JOIST	MODEL	CAPACITY	FACE	BEAM/JOIST	MODEL	CAPACITY	HEADER	BEAM/JOIST
(2) 2x8	LUS28-2	910 lbs (UPLIFT) / 1280 lbs (DOWN)	(6) 0.162 x 3 1/2"	(4) 0.162 x 3 1/2"	HUS28-2TF	17655 lbs (UPLIFT) / 3400 lbs (DOWN)	(8) 0.162 x 3 1/2"	(6) 0.162 x 3 1/2"
(2) 2x12	LUS210-2	1245 lbs (UPLIFT) / 1785 lbs (DOWN)	(8) 0.162 x 3 1/2"	(6) 0.162 x 3 1/2"	HUS212-2TF	2080 lbs (UPLIFT) / 4375 lbs (DOWN)	(10) 0.162 x 3 1/2"	(8) 0.162 x 3 1/2"
2x12 PT DECK JOISTS	LUS210	1165 lbs (UPLIFT) / 1530 lbs (DOWN)	(8) 0.162 x 3"	(4) 0.162 x 3"	HU212TF	700 lbs (UPLIFT) / 3070 lbs (DOWN)	(14) 0.162 x 3 1/2"	(6) 0.148 x 1 1/2"
6" PE OPEN WEB TRUSS	LUS210	1165 lbs (UPLIFT) / 1530 lbs (DOWN)	(8) 0.162 x 3"	(4) 0.162 x 3"	HU210TF	700 lbs (UPLIFT) / 2970 lbs (DOWN)	(12) 0.162 x 3 1/2"	(4) 0.148 x 1 1/2"
(2) 16" LVL	HUCQ412-SDS	2265 lbs (UPLIFT) / 5045 lbs (DOWN)	(14) 1/4 x 2 1/2" SDS	(6) 1/4" x 2 1/2" SDS	BA3.56/18 (MAX)	1250 lbs (UPLIFT) / 4715 (DOWN)	(16) 0.162 x 3 1/2"	(8) 0.148 x 1 1/2"
(3) 7 1/4" LVL	HU68 (MAX)	1135 lbs (UPLIFT) / 2350 lbs (DOWN)	(14) 0.162 x 3 1/2"	(4) 0.148 x 3 1/2"	N/A	N/A	N/A	N/A
(3) 9 1/4" LVL	HU610	1795 lbs (UPLIFT) / 3020 lbs (DOWN)	(18) 0.162 x 3 1/2"	(8) 0.162 x 3 1/2"	HB5.5/9.25	2075 lbs (UPLIFT) / 5815 lbs (DOWN)	(22) 0.162 x 3 1/2"	(10) 0.162 x 3 1/2
(3) 11 7/8" LVL	HGUS5.50/12	5205 lbs (UPLIFT) / 11835 lbs (DOWN)	(56) 0.162 x 3 1/2"	(20) 0.162 x 3 1/2"	ECQ5.37-SDS, H=11.875	7670 lbs (UPLIFT) / 19800 lbs (DOWN)	(28) 1/4 x 3" SDS	(12) 1/4" x 3" SDS
(4) 11 7/8" LVL	HGUS7.25/12	5205 lbs (UPLIFT) / 11835 lbs (DOWN)	(56) 0.162 x 3 1/2"	(20) 0.162 x 3 1/2"	EGQ7.25-SDS	7670 lbs (UPLIFT) 19800 lbs (DOWN)	(28) 1/4 x3 SDS	(12) 1/4 x 3 SDS
GL6 3/4x12	HGUS6.88/12	5205 lbs (UPLIFT) / 13215 lbs (DOWN)	(56) 0.162 x 3 1/2"	(20) 0.162 x 3 1/2"	HGLT7	2450 lbs (UPLIFT) / 10720 lbs (DOWN)	(18) N54A	(6) N54A
GL6 3/4x18	HGUS6.88/12	5205 lbs (UPLIFT) / 13215 lbs (DOWN)	(56) 0.162 x 3 1/2"	(20) 0.162 x 3 1/2"	HGLT7	2450 lbs (UPLIFT) / 10720 lbs (DOWN)	(18) N54A	(6) N54A

HEADER FASTENERS -

BEAM/JOIST

FASTENERS

SHEARWALL

SHTG, TYP -

- (3) 2x6 STUD CORNER

Ŵ/ 12d @ 8" EA STUD -

WOOD STUD

WALL, TYP -

STUDS AT CORNER

3/4" = 1'-0'

3

S-5.3

HOLDOWN TO (2)

KING STUDS MIN -

* *

KING STUDS -

TRIM STUD(S)

OR BLOCKING

HOLDOWN AT CORNER AND END

TYP WOOD STUD WALL PLAN DETAILS

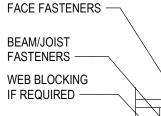
- DRYWALL STOP

CONNECT WOOD PACKING TO WEB W/ 1/2"Ø THRU-BOLTS @ 24" UNLESS NOTED OTHERWISE

FACE FASTENERS -

BEAM/JOIST

FASTENERS -



TOP FLANGE - STEEL BM

-1/8"

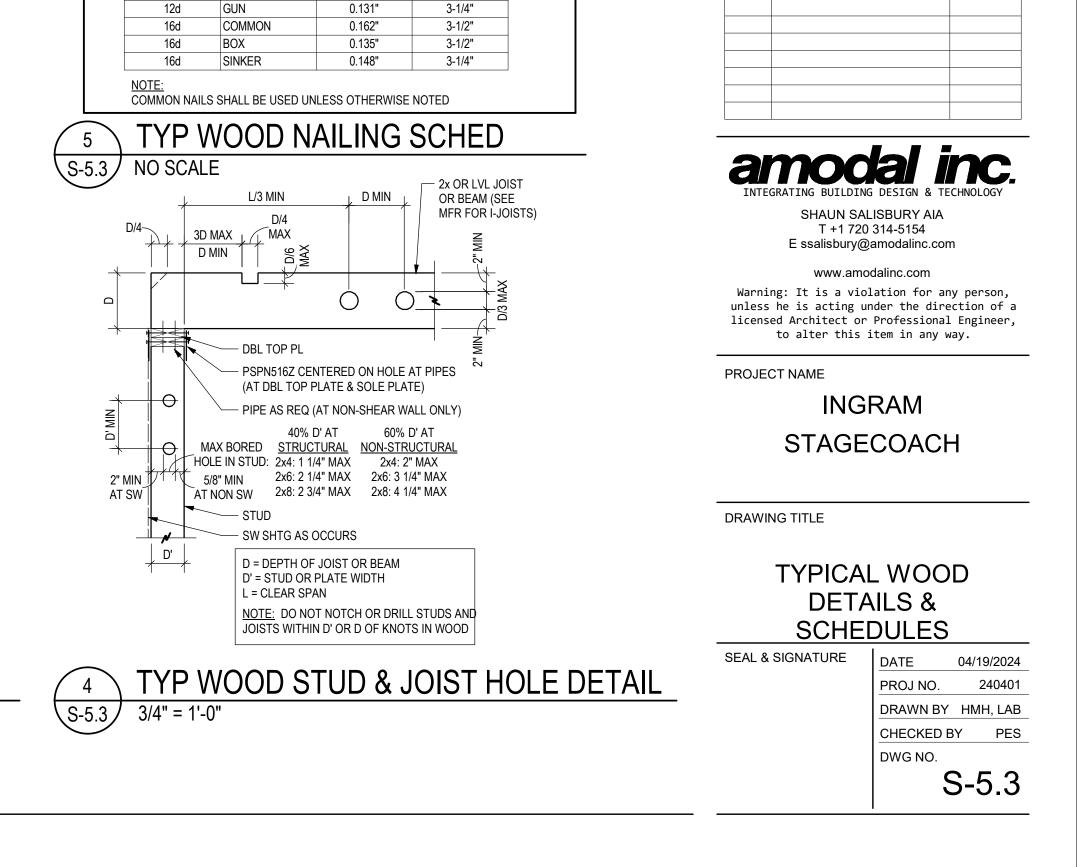
SIDE

HEADER FASTENERS —

BEAM/JOIST

FASTENERS

WEB BLOCKING



1/16" MIN ✓ 1/4" MAX WEB PACK OUT REQ'D AT EGQ, GLTV, HGLTV , (MAX) HANGERS AND WHERE HANGERS REQUIRE FACE NAILS. PROVIDE 1/2"Ø THRU-BOLT THROUGH WEB EACH

PENNYWEIGHT

10d

10d

10d

10d

12d

12d

12d

TYPE

COMMON

BOX

GUN

BOX

GUN

BOX

SINKER

SINKER

COMMON

SINKER

COMMON

DIAMETER

0.131"

0.113"

0.113"

0.113"

0.148"

0.128"

0.120"

0.131"

0.148"

0.128"

0.135"

LENGTH

2-1/2"

2-1/2"

2-3/8"

2-3/8"

2-7/8"

3"

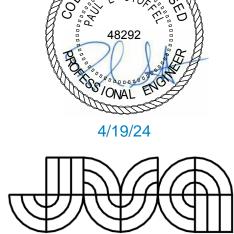
3-1/4"

3-1/4"

3-1/8"

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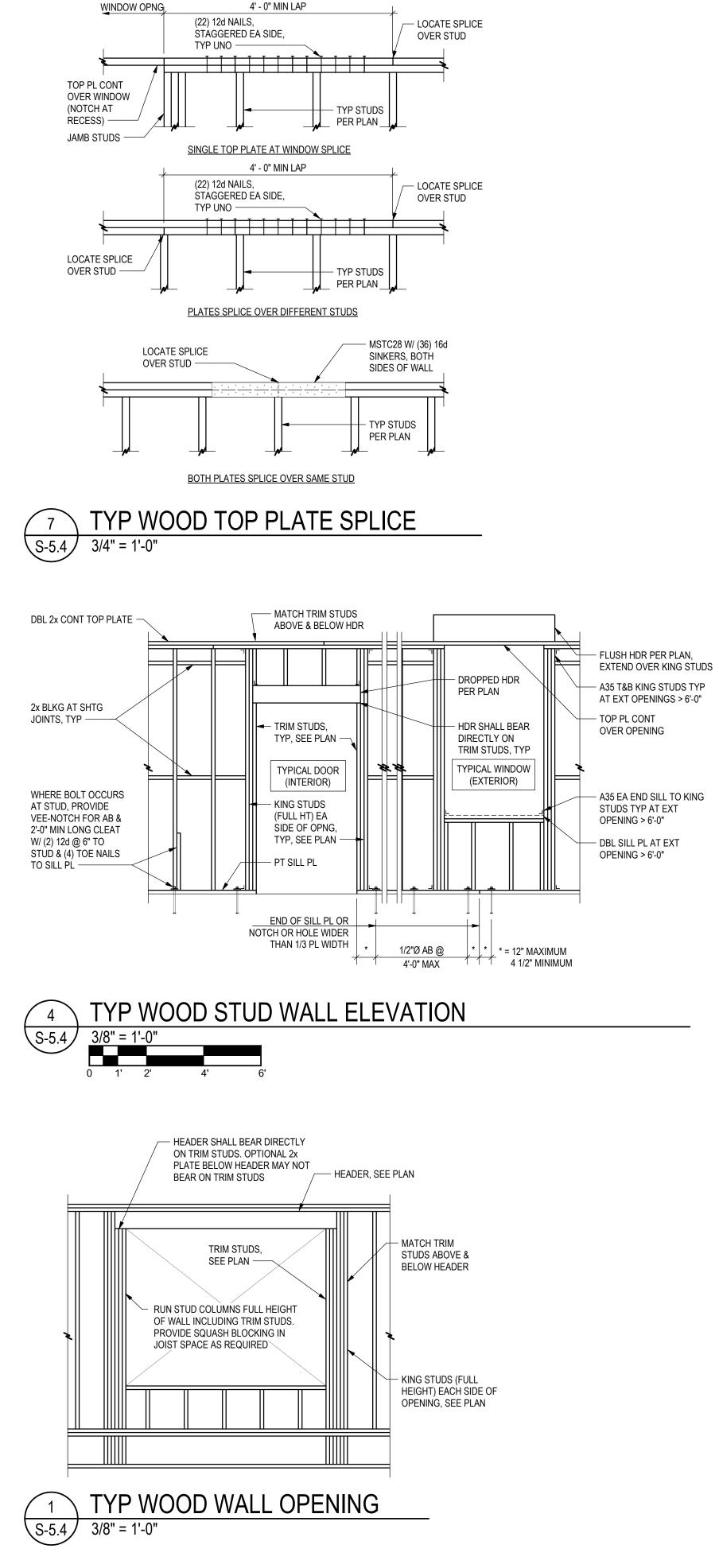


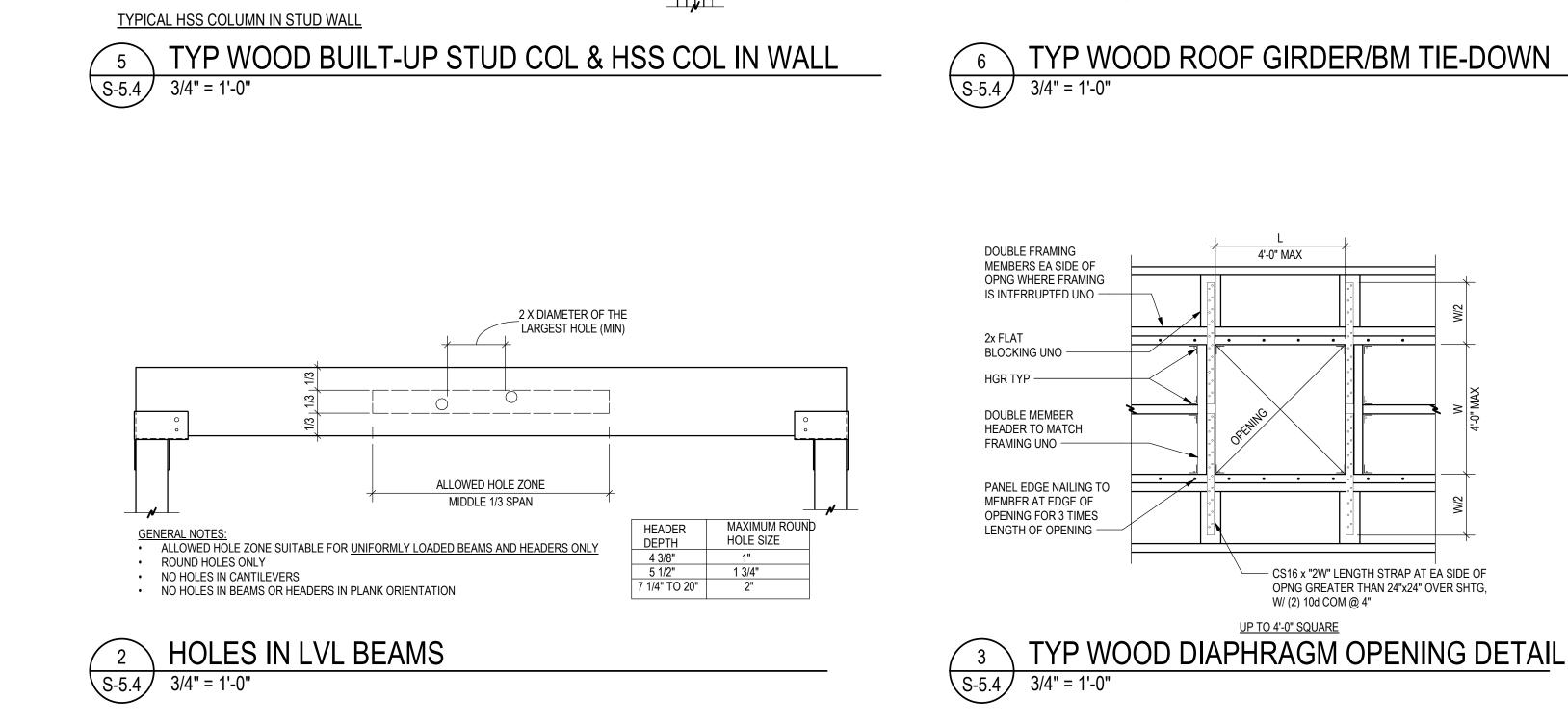
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Date 04/19/2024





— 2x STUD COLUMN

- FASTEN EACH

- (3) 12d @ 12"

ADDITIONAL 2x W/

SDW SCREWS TO

ENGAGE EVERY STUD

(2) 2x TOP PLATE PER SCHEDULE —

~

- HSS COL

2x TRIM STUD EA SIDE OF HSS W/ PANEL EDGE

NAILING AT SHEAR WALL

FASTEN EACH

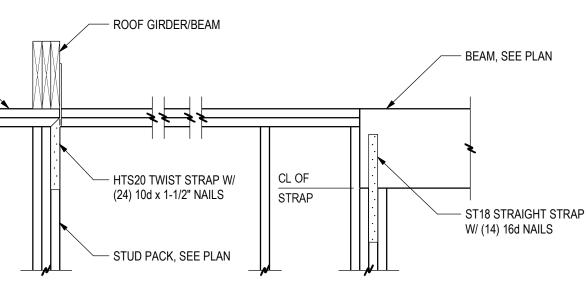
TRIM STUD W/ 1/2"Ø

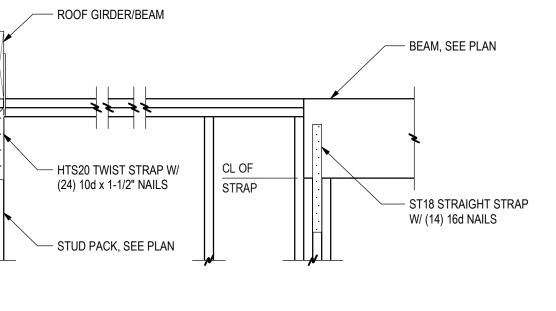
FROM COL ENDS

THRU BOLT @ 24" & 12"

(3) 12d NAILS @ 12", STAGGER FROM NAILS

ON OPPOSITE SIDE -





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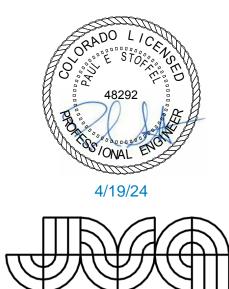
DRAWING IIILE

TYPICAL WOOD

DETAILS SEAL & SIGNATURE DATE 04/19/2024 PROJ NO. 240401

DRAWN BY HMH, LAB CHECKED BY PES DWG NO. S-5.4

Reviewed for Code Compliance 05/09/2024





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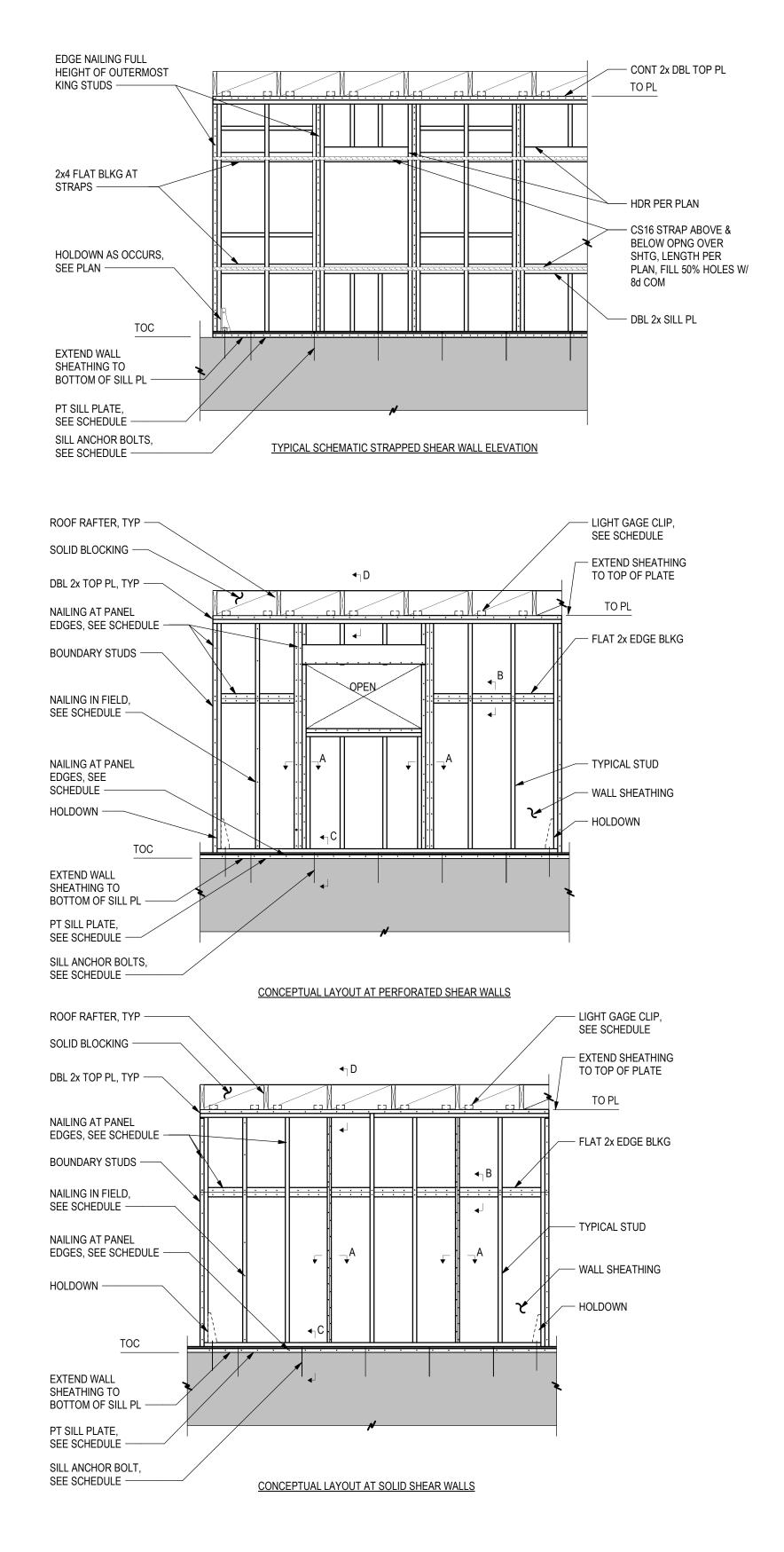
Glenwood Springs

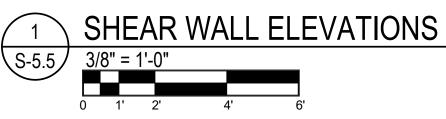
Denver JVA #240401

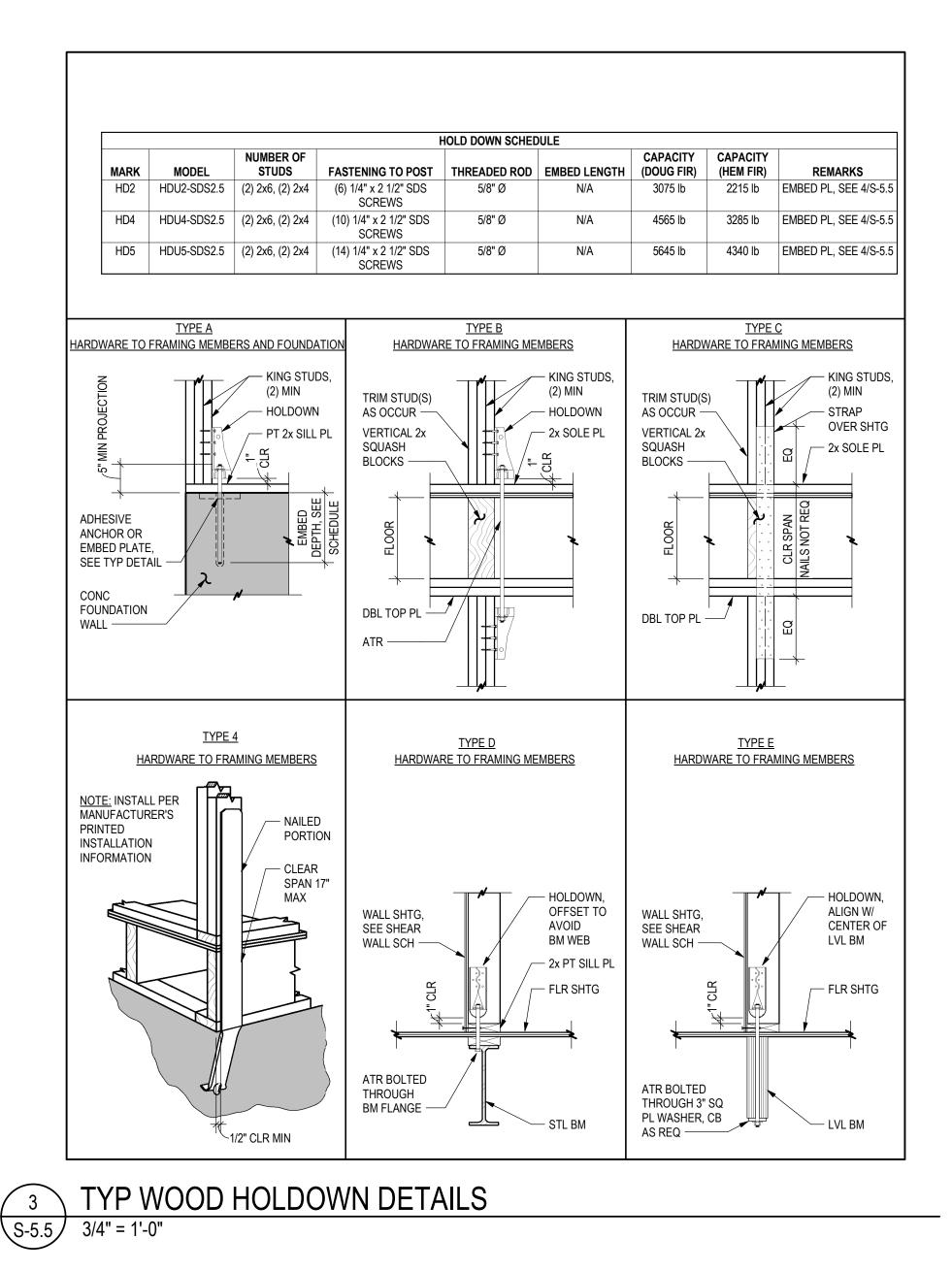
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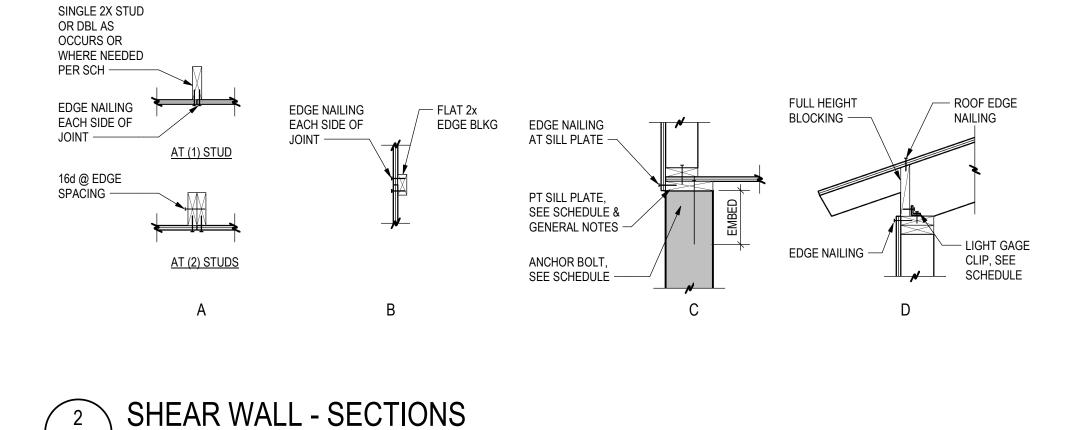
Permit Submission

Date 04/19/2024









∖S-5.5/

3/4" = 1'-0"

0 6" 1'

2'



4

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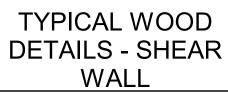
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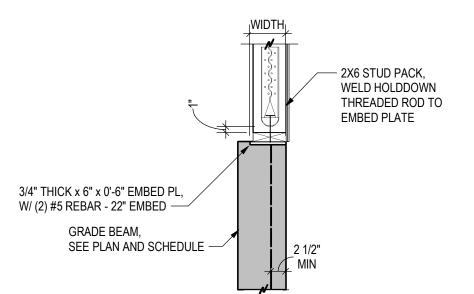
PROJECT NAME

INGRAM STAGECOACH

DRAWING TITLE

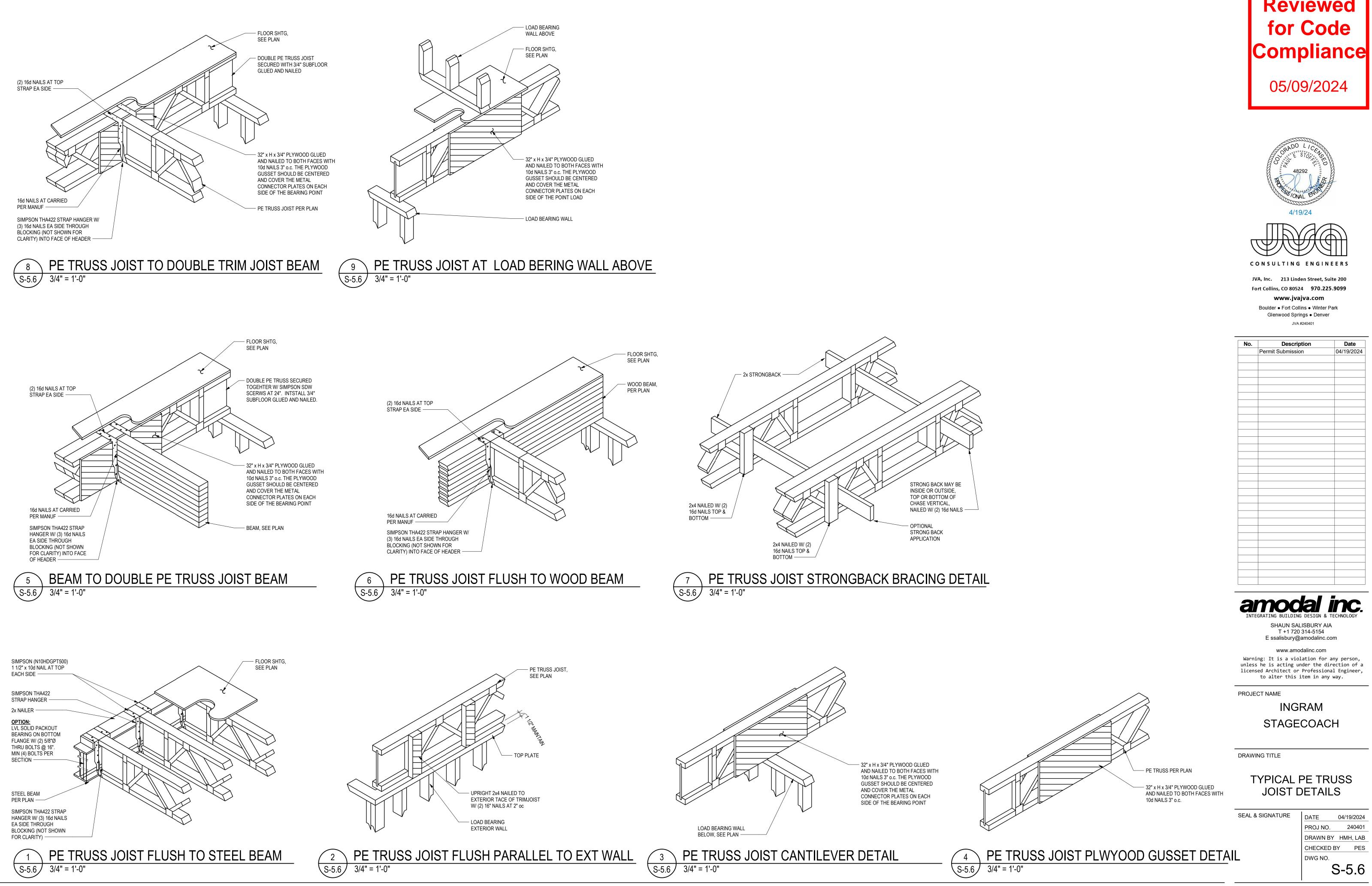


SEAL & SIGNATURE DATE 04/19/2024 PROJ NO. 240401 DRAWN BY HMH, LAB CHECKED BY PES DWG NO. S-5.5



HOLD DOWN EMBED PLATE

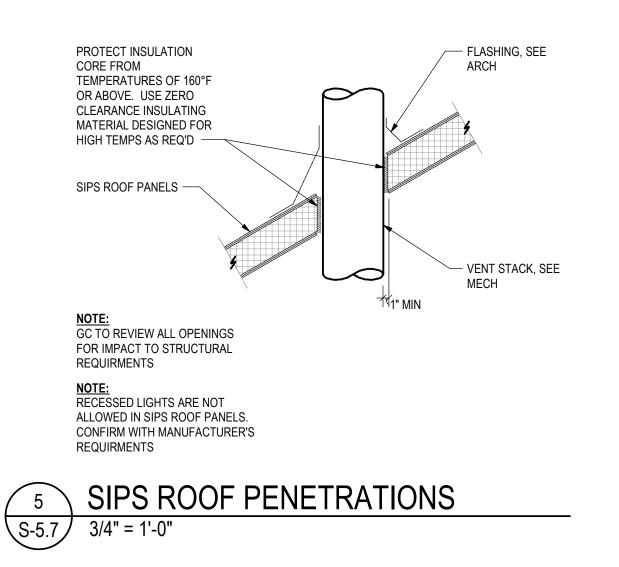
S-5.5 3/4" = 1'-0"

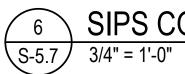


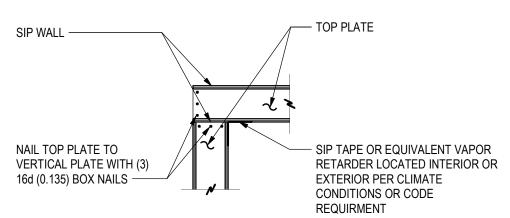
Reviewed

SIP WALL -

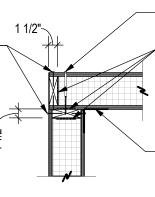
8d BOX (0.113) NAILS @ 6", EACH SIDE ------







PLAN VIEW



- SIP TAPE OR EQUIVALENT VAPOR RETARDER LOCATED INTERIOR OR EXTERIOR PER CLIMATE CONDITIONS OR CODE REQUIRMENT

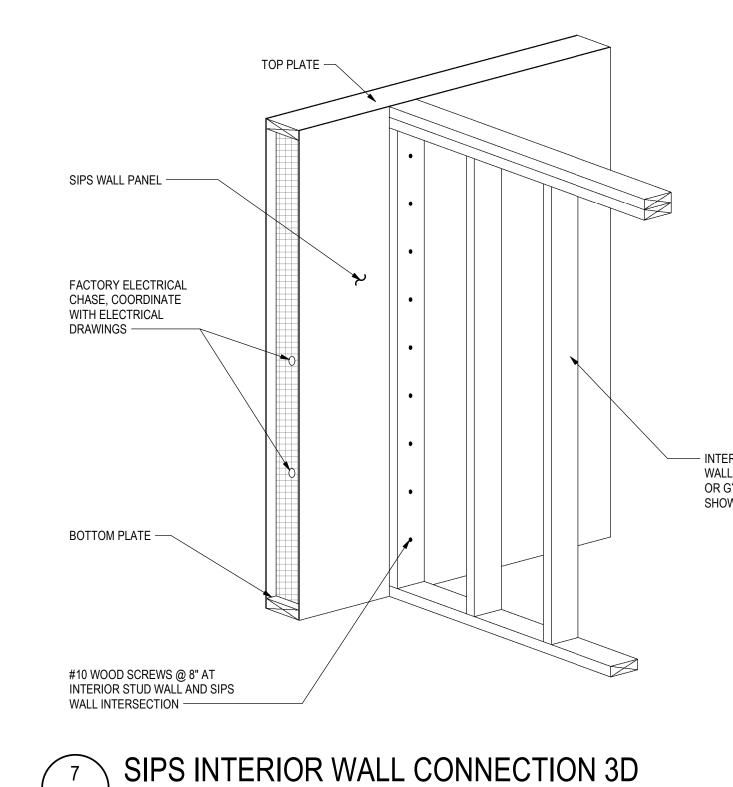
- WOOD SCREW @ 24" MAX,

— 1/2" Ø CONTINUOUS BEAD BY MANUFACTURER

BY MANUFACTURER

SECTION VIEW

SIPS CORNER CONNECTION



S-5.7 1/4" = 1'-0"

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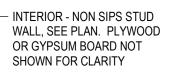
PROJECT NAME

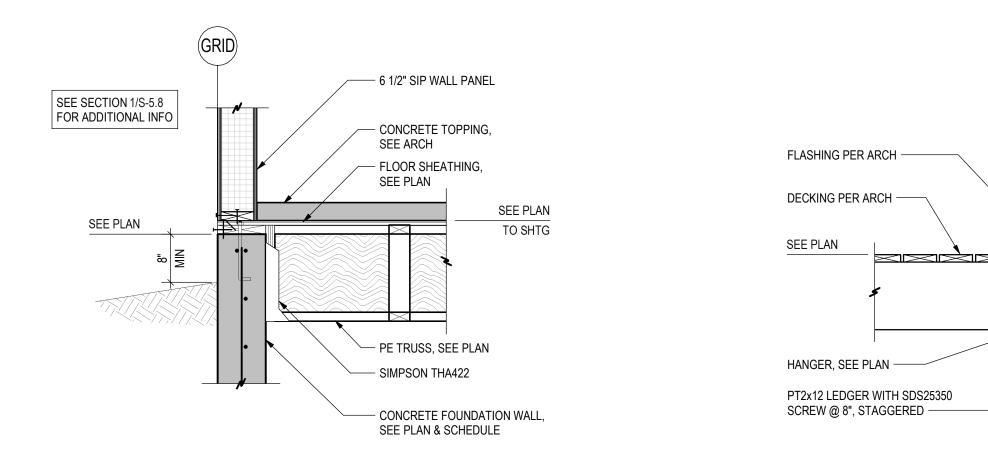
INGRAM STAGECOACH

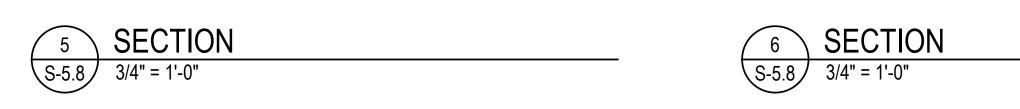
DRAWING TITLE

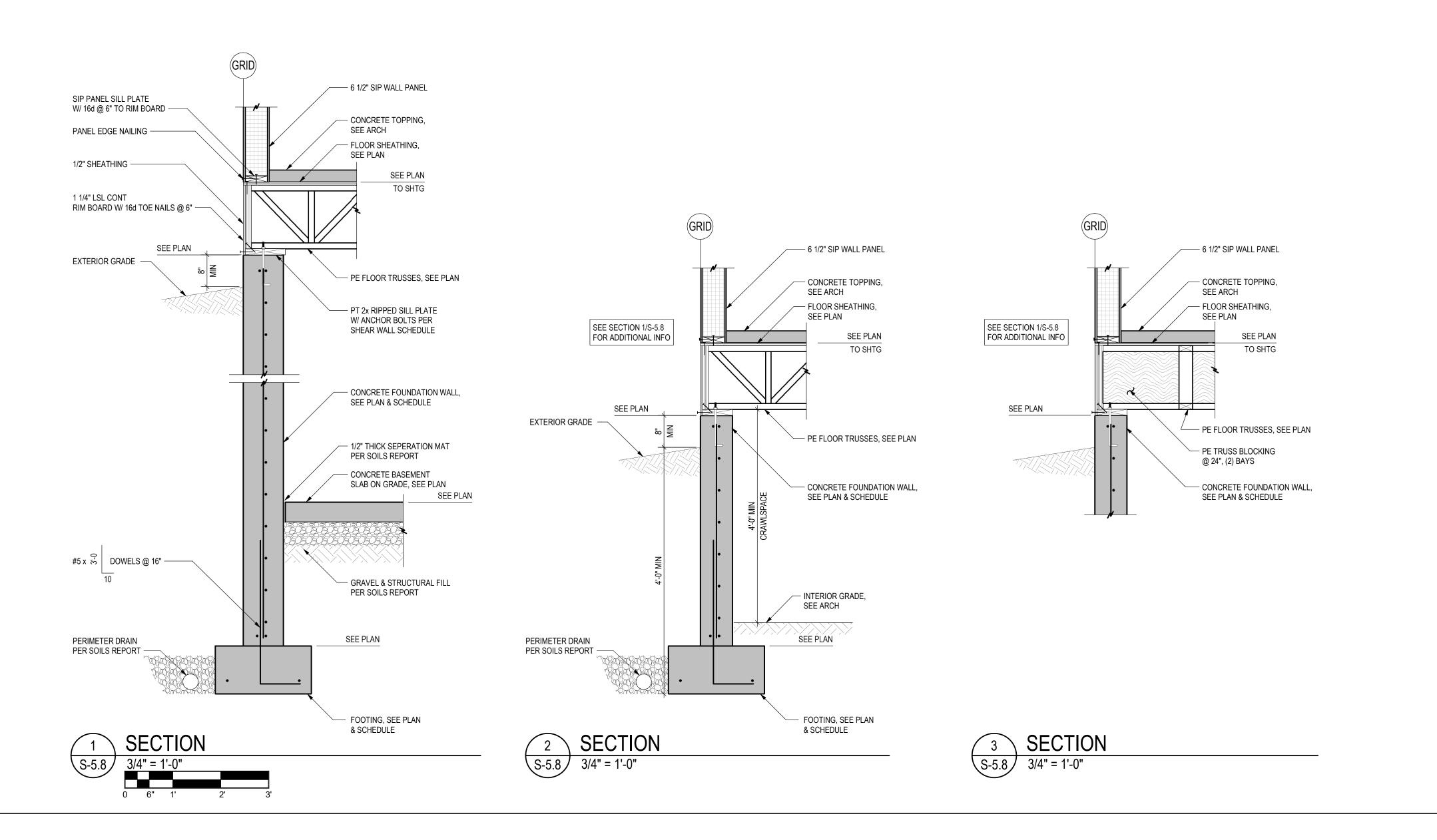
TYPICAL SIP PANEL DETAILS

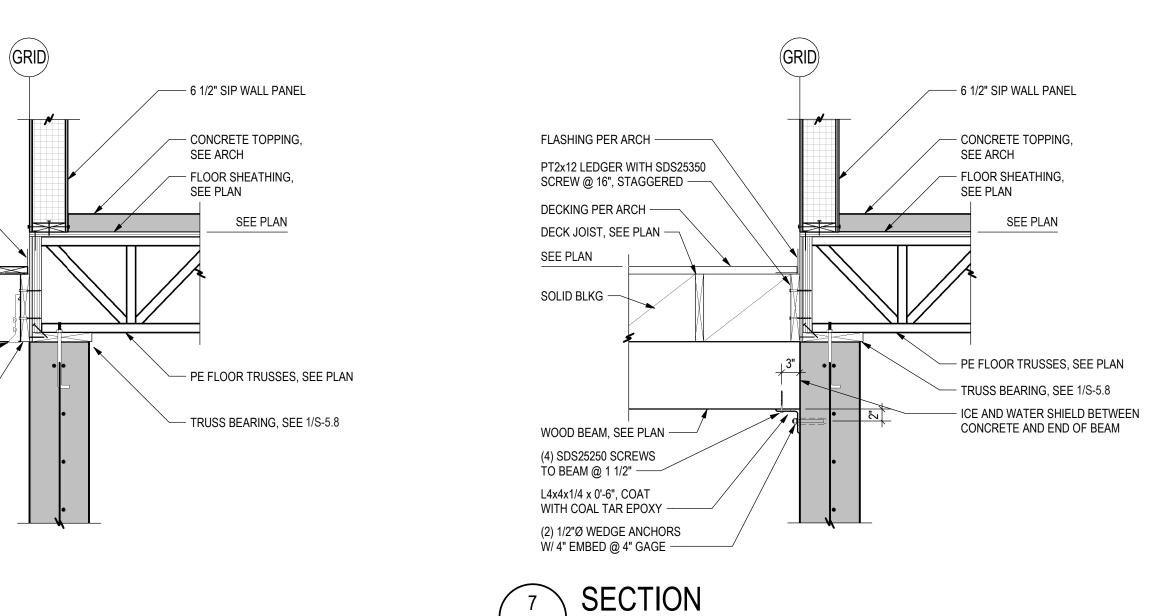
SEAL & SIGNATURE	DATE 04/19/2024
	PROJ NO. 240401
	DRAWN BY HMH, LAB
	CHECKED BY PES
	DWG NO.
	S-5.7



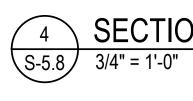








S-5.8 3/4" = 1'-0"



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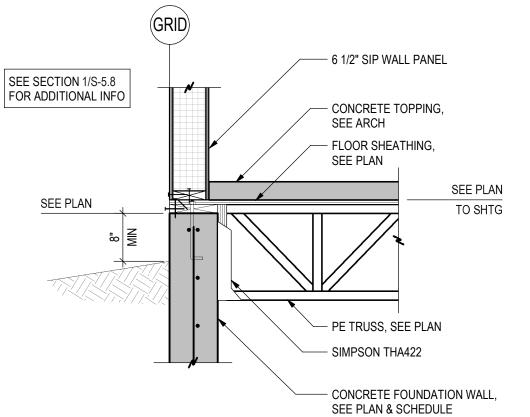
PROJECT NAME

INGRAM STAGECOACH

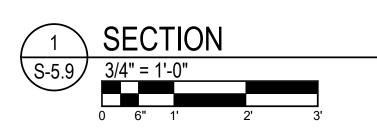
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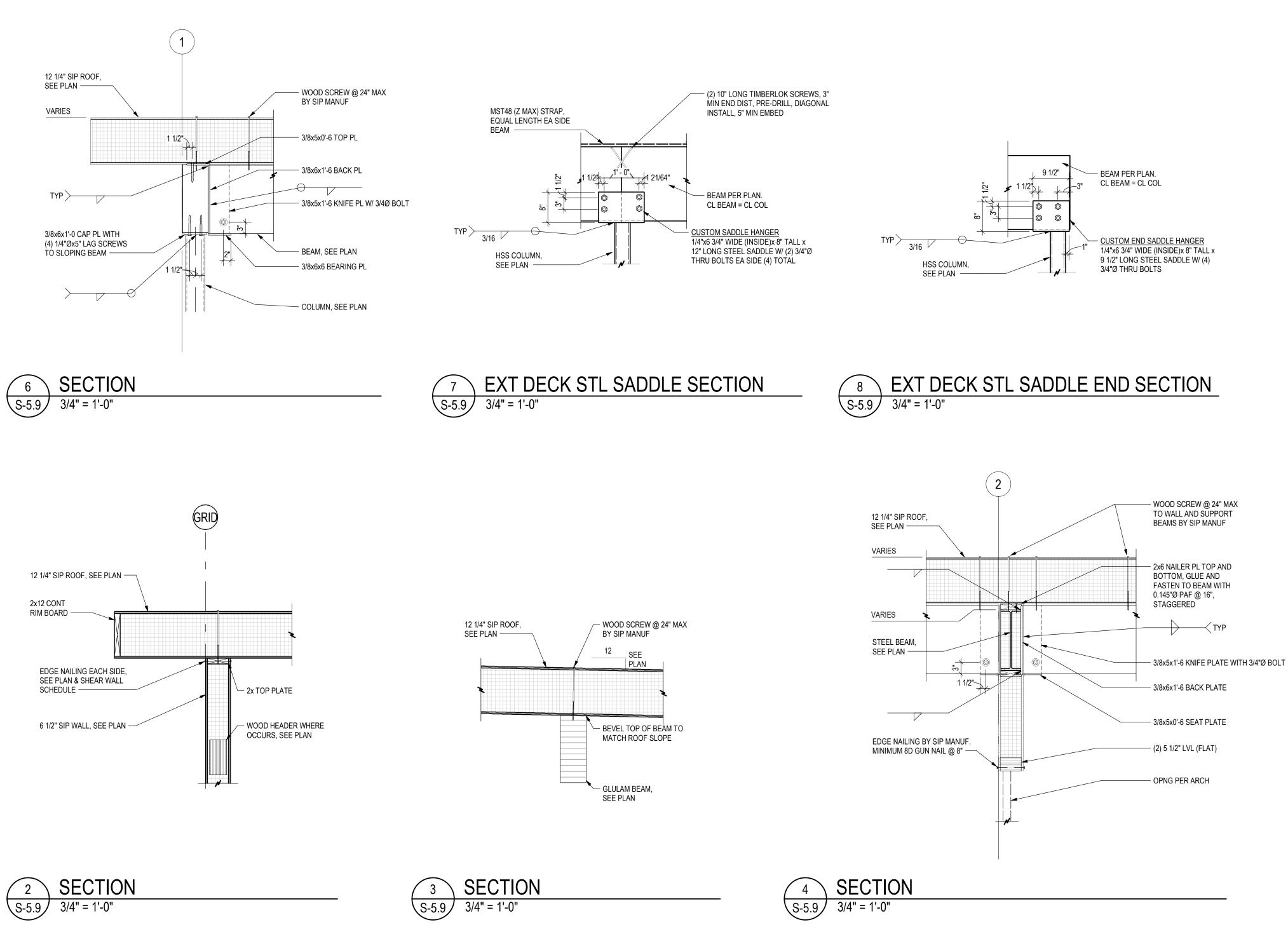
FOUNDATION SECTIONS

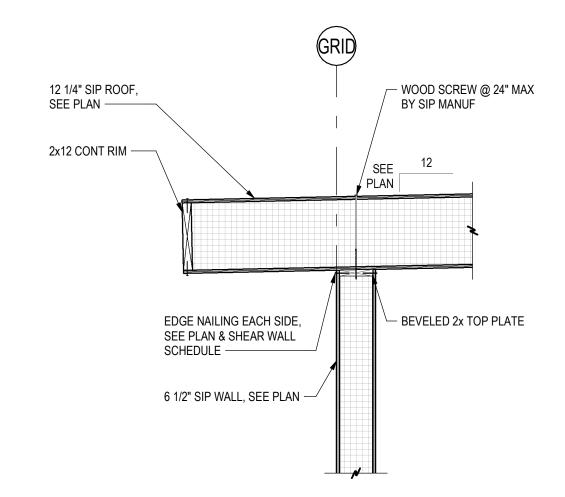
SEAL & SIGNATURE DATE 04/19/2024 240401 PROJ NO. DRAWN BY Author CHECKED BY Checker DWG NO. S-5.8

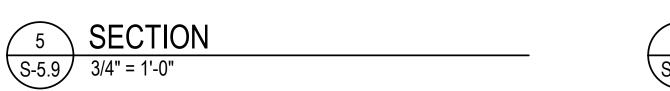


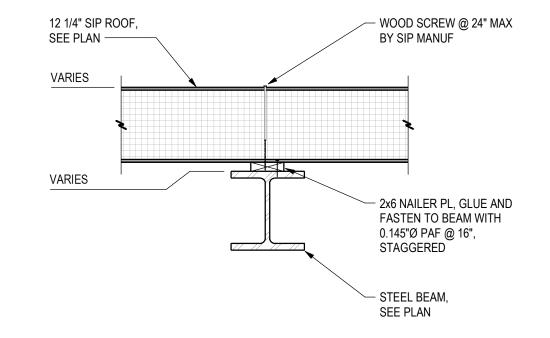
SECTION

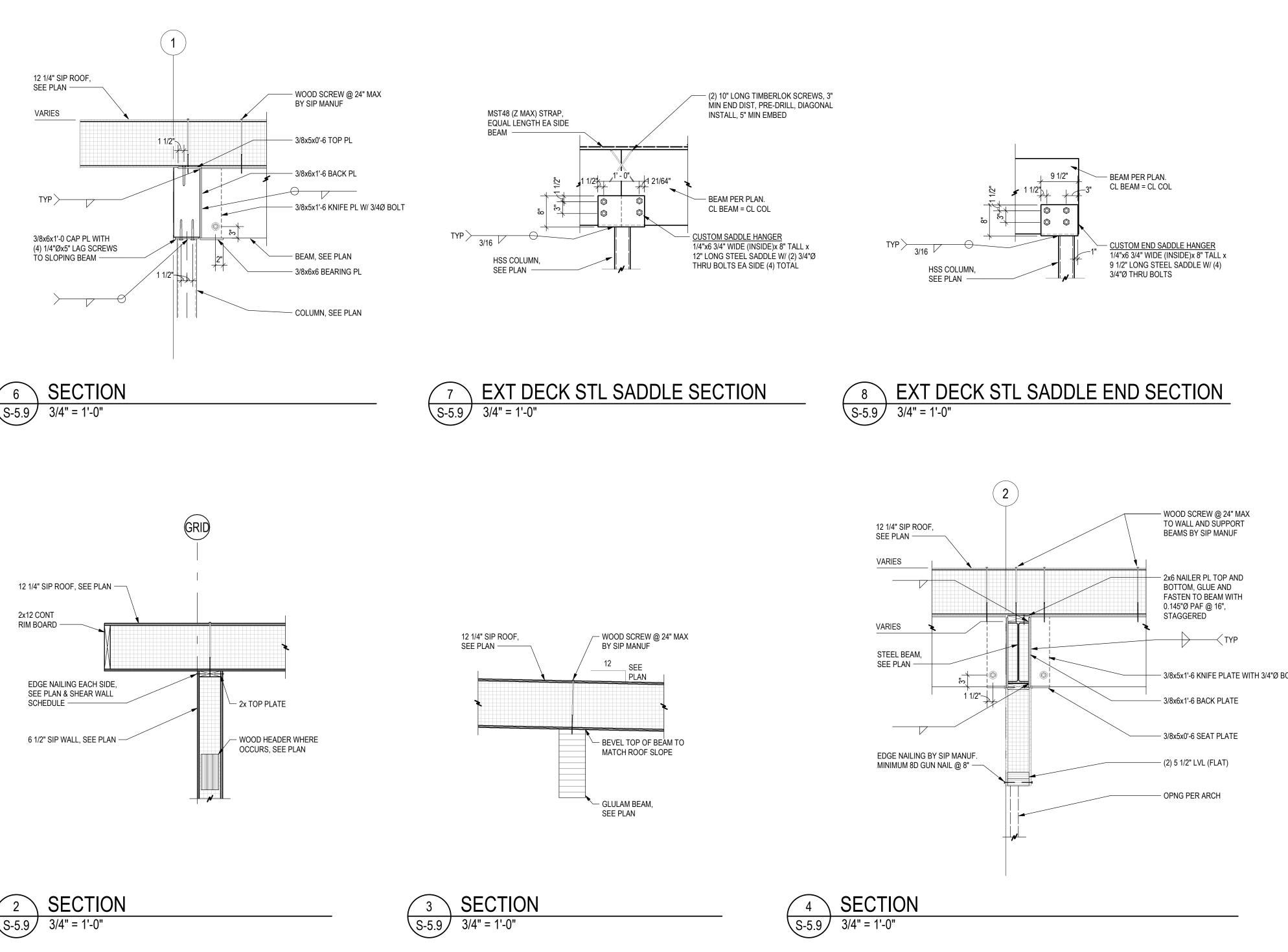




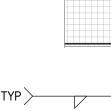


















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PROJECT NAME

INGRAM STAGECOACH

DRAWING TITLE

ROOF SECTIONS

SEAL & SIGNATURE DATE 04/19/2024 240401 PROJ NO. DRAWN BY Author CHECKED BY Checker DWG NO. S-5.9