EAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Western Slope Truss Mfg. LLC**.

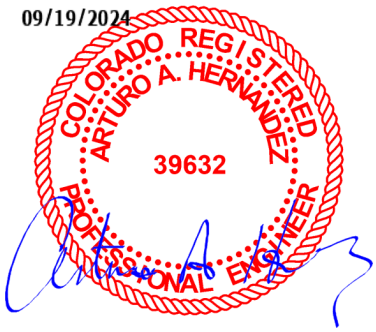
These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **091224 Sebring** - 1219661
G01, G01x, GE01, GE02, GE03, GE04, S01, S02, S03, T01, T02, T03, T04, V01, V02, V03, V04, V05, V06, V07

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (CO, PE-39632)
My license expiration date for the state of CO is 10/31/2025.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

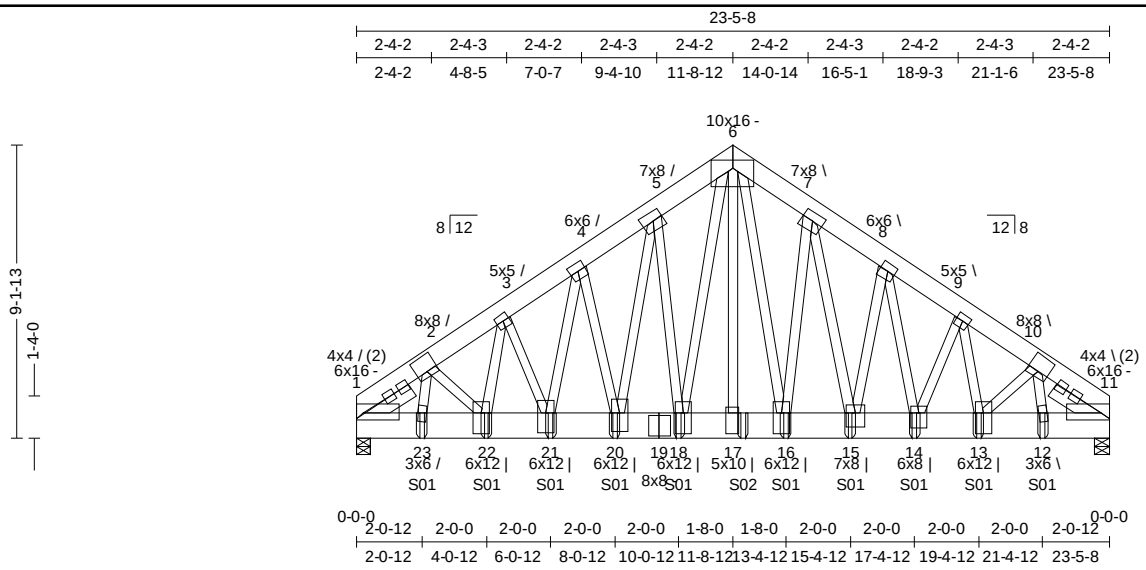
REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: G01
Job: 091224 Sebring
Date: 09/19/24 13:59:42
Page: 1 of 2

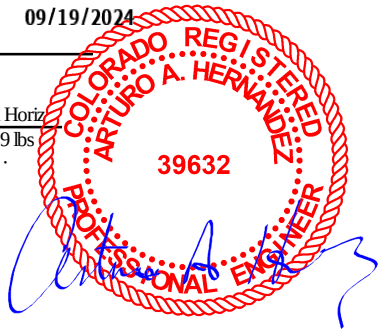
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	3	11.25 in	285 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2021/ TPI 1-2014	TC : 0.59 (10-11)	Vert TL: 0.24 in	L / 999	(14-15)	L / 240
TCLL : 117	Rep Mbr : Yes	BC : 0.85 (11-12)	Vert LL: 0.17 in	L / 999	(14-15)	L / 360
TCDL : 15(rake)	Lumber D.O.L. : 100 %	Web : 0.64 (6-16)	Horz TL: 0.08 in		11	
BCLL : 0						
BCDL : 10						

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	7.01 in	19,710 lbs	·	-554 lbs	·	-554 lbs	39 lbs
11	1	5.5 in	7.21 in	20,267 lbs	·	-557 lbs	·	-557 lbs	·

Bearing enhancers may be required at the following bearings: See Eagle Metal 'Bearing Enhancer' detail for capacity of specific bearing block(s) and connectors: Brg # 1, 11

Material

TC: DFL 1950/1.7 2 x 8
BC: DFL 1950/1.7 2 x 10
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	23-5-8	Down	Proj	18.75 plf	18.75 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	23-5-8	Down	Proj	14.06 plf	14.06 plf	
Bot	0-0-0	23-5-8	Down	Proj	9.38 plf	9.38 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.578	-8,167 lbs		4-5	0.354	-7,599 lbs		7-8	0.364	-7,821 lbs		10-11	0.595	-8,381 lbs			
	2-3	0.394	-8,445 lbs		5-6	0.427	-6,685 lbs		8-9	0.392	-8,415 lbs							
	3-4	0.383	-8,218 lbs		6-7	0.428	-6,782 lbs		9-10	0.404	-8,663 lbs							
BC	11-12	0.854	6,543 lbs	(-173 lbs)	14-15	0.539	6,698 lbs	(-167 lbs)	17-18	0.492	5,400 lbs	(-125 lbs)	21-22	0.536	6,981 lbs	(-184 lbs)		
	12-13	0.489	6,554 lbs	(-174 lbs)	15-16	0.421	6,032 lbs	(-145 lbs)	18-20	0.631	5,947 lbs	(-144 lbs)	22-23	0.475	6,387 lbs	(-173 lbs)		
	13-14	0.560	7,152 lbs	(-185 lbs)	16-17	0.571	5,400 lbs	(-125 lbs)	20-21	0.529	6,558 lbs	(-166 lbs)	23-1	0.838	6,377 lbs	(-173 lbs)		

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Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss:G01
Job: 091224 Sebring
Date: 09/19/24 13:59:43

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	3	11.25 in	285 lbs

Web	2-22	0.234	965 lbs	(-19 lbs)	5-20	0.478	1,975 lbs	(-63 lbs)	7-16	0.230	-1,194 lbs		9-13	0.109	452 lbs	(-13 lbs)
	3-22	0.106	440 lbs	(-13 lbs)	5-18	0.202	-1,049 lbs		7-15	0.522	2,156 lbs	(-66 lbs)	10-13	0.236	975 lbs	(-19 lbs)
	3-21	0.060	-435 lbs		6-18	0.555	2,293 lbs	(-75 lbs)	8-15	0.198	-1,287 lbs					
	4-21	0.360	1,489 lbs	(-47 lbs)	6-17	0.410	1,694 lbs	(-49 lbs)	8-14	0.356	1,470 lbs	(-45 lbs)				
	4-20	0.190	-1,234 lbs		6-16	0.637	2,634 lbs	(-79 lbs)	9-14	0.069	-502 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
S01	BC	2-0-12
S01	BC	4-0-12
S01	BC	6-0-12
S01	BC	8-0-12
S01	BC	10-0-12
S02	BC	12-0-12
S01	BC	13-4-12
S01	BC	15-4-12
S01	BC	17-4-12
S01	BC	19-4-12
S01	BC	21-4-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.50 has been applied for this truss analysis.
5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Three identical trusses shall be built and attached as follows: Head Side - SDW Simpson (3 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 3 staggered rows @ 0-7-4 oc , Webs - 1 row 16d Nails or Gun Nails [min 0.135"x3"] @ 2-0-0 oc.
6) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
7) Lateral bracing shall be attached to each ply.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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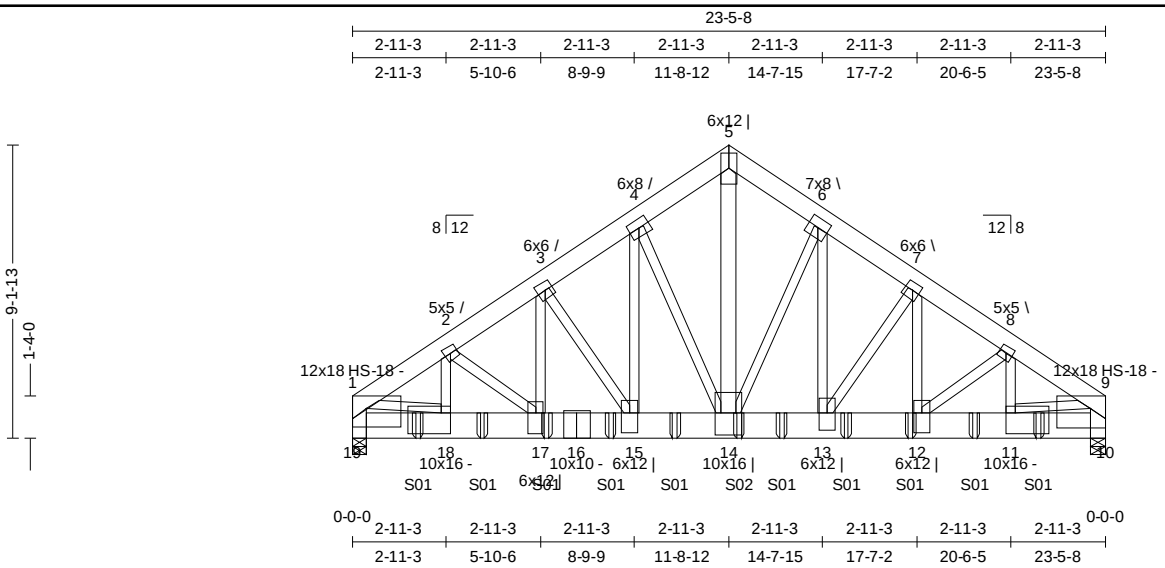
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss:G01x
Job: 091224 Sebring
Date: 09/19/24 13:59:45

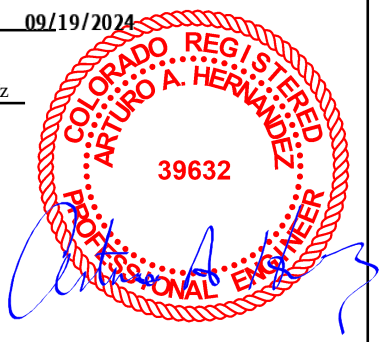
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	3	11.25 in	267 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2021/ TPI 1-2014	TC : 0.38 (7-8)	Vert TL: 0.26 in	L / 999	(12-13)	L / 240
TCLL : 117	Rep Mbr : Yes	BC : 0.99 (13-14)	Vert LL: 0.18 in	L / 999	(12-13)	L / 360
TCDL : 15(rake)	Lumber D.O.L. : 100 %	Web : 0.72 (5-14)	Horz TL: 0.06 in		10	
BCLL : 0						
BCDL : 10						

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
19	1	5.5 in	2.58 in	19,705 lbs	·	-554 lbs	·	-554 lbs	-57 lbs
10	1	5.5 in	2.65 in	20,273 lbs	·	-557 lbs	·	-557 lbs	·

Material

TC: DFL 1950/1.7 2 x 8
BC: DFL 1950/1.7 2 x 10
Web: HF #2 2 x 4 except:
DFL 2400/2.0 2 x 4: 1-18, 9-11
SPF 1650/1.5 2 x 6: 1-19, 5-14, 9-10

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	23-5-8	Down	Proj	18.75 plf	18.75 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	23-5-8	Down	Proj	14.06 plf	14.06 plf	
Bot	0-0-0	23-5-8	Down	Proj	9.38 plf	9.38 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.357	-7,636 lbs		3-4	0.332	-7,112 lbs		5-6	0.354	-6,103 lbs		7-8	0.380	-8,138 lbs	
	2-3	0.370	-7,927 lbs		4-5	0.355	-6,069 lbs		6-7	0.339	-7,272 lbs		8-9	0.367	-7,849 lbs	
BC	11-12	0.539	6,449 lbs	(-172 lbs)	13-14	0.989	5,983 lbs	(-147 lbs)	15-17	0.526	6,616 lbs	(-174 lbs)				
	12-13	0.538	6,773 lbs	(-175 lbs)	14-15	0.835	5,872 lbs	(-146 lbs)	17-18	0.526	6,282 lbs	(-171 lbs)				

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	3	11.25 in	267 lbs

Web	1-19	0.433	-6,064 lbs		3-15	0.211	-1,362 lbs		6-13	0.599	2,478 lbs	(-68 lbs)	9-11	0.681	6,879 lbs	(-184 lbs)
	1-18	0.663	6,699 lbs	(-183 lbs)	4-15	0.551	2,280 lbs	(-65 lbs)	7-13	0.224	-1,447 lbs		9-10	0.446	-6,247 lbs	
	2-18	0.047	-361 lbs		4-14	0.302	-1,513 lbs		7-12	0.354	1,463 lbs	(-38 lbs)				
	2-17	0.107	442 lbs	(-3 lbs)	5-14	0.715	6,021 lbs	(-182 lbs)	8-12	0.104	428 lbs	(-3 lbs)				
	3-17	0.352	1,456 lbs	(-39 lbs)	6-14	0.354	-1,773 lbs		8-11	0.052	-399 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
S01	BC	2-0-12
S01	BC	4-0-12
S01	BC	6-0-12
S01	BC	8-0-12
S01	BC	10-0-12
S02	BC	12-0-12
S01	BC	13-4-12
S01	BC	15-4-12
S01	BC	17-4-12
S01	BC	19-4-12
S01	BC	21-4-12

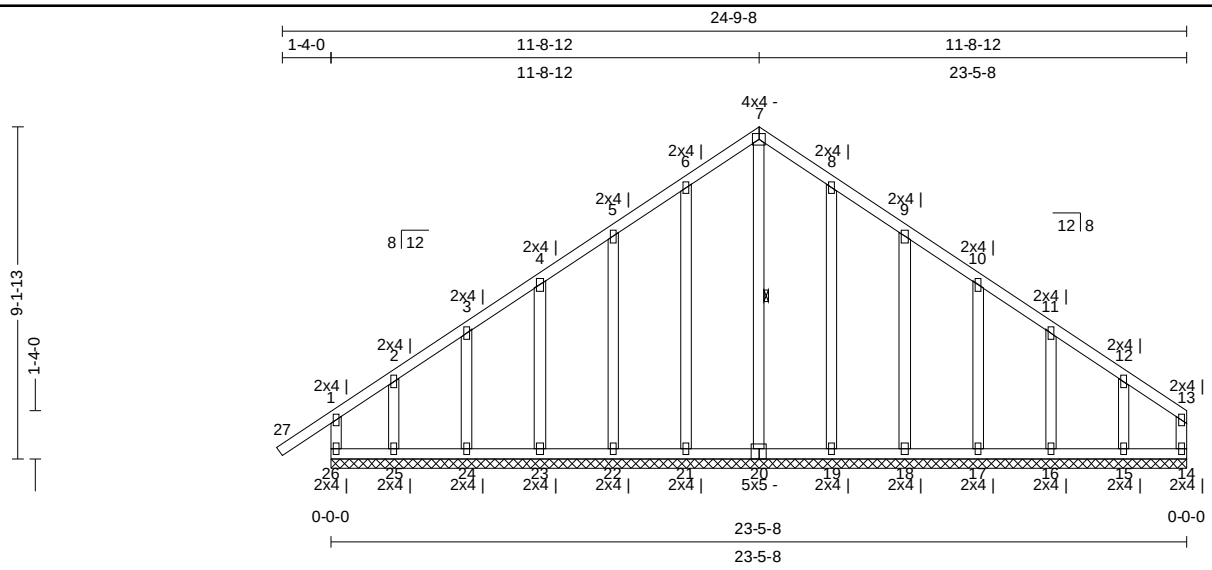
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.50 has been applied for this truss analysis.
5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Three identical trusses shall be built and attached as follows: Head Side - FastenMaster FlatLOK (3 - Ply) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 3 staggered rows @ 0-8-4 oc , Webs - 1 row @ 2-0-0 oc.
6) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
7) Lateral bracing shall be attached to each ply.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: GE01
Job: 091224 Sebring
Date: 09/19/24 13:59:46
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	123 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	81.9	Bldg Code :	IBC 2021/ TPI 1-2014	TC :	0.99 (27-1)	Vert TL:	0 in UP	L / 999	14	L / 240
TCDL :	15(rake)	Rep Mbr :	No	BC :	0.05 (15-16)	Vert LL:	0 in	L / 999	14	L / 360
BCLL :	0	Lumber D.O.L. :	100 %	Web :	0.70 (6-21)	Horz TL:	0 in			
BCDL :	10									

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	697 lbs	310 plf	-140 lbs	-58 lbs	-67 lbs	-140 lbs	85 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 7-20

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC L L = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	6-7	0.597	326 lbs	(-147 lbs)							
	7-8	0.594	326 lbs	(-187 lbs)							
BC											
Web	1-26	0.296	-677 lbs		6-21	0.695	-657 lbs		10-17	0.282	-641 lbs
	3-24	0.131	-534 lbs		7-20	0.174	-439 lbs		11-16	0.125	-509 lbs
	4-23	0.280	-637 lbs		8-19	0.693	-654 lbs		12-15	0.061	-412 lbs
	5-22	0.447	-627 lbs		9-18	0.447	-627 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
7) Gable must be sheathed on one side or lateral bracing applied appropriately.
8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
9) A creep factor of 1.50 has been applied for this truss analysis.
10) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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Eagle Metal Products

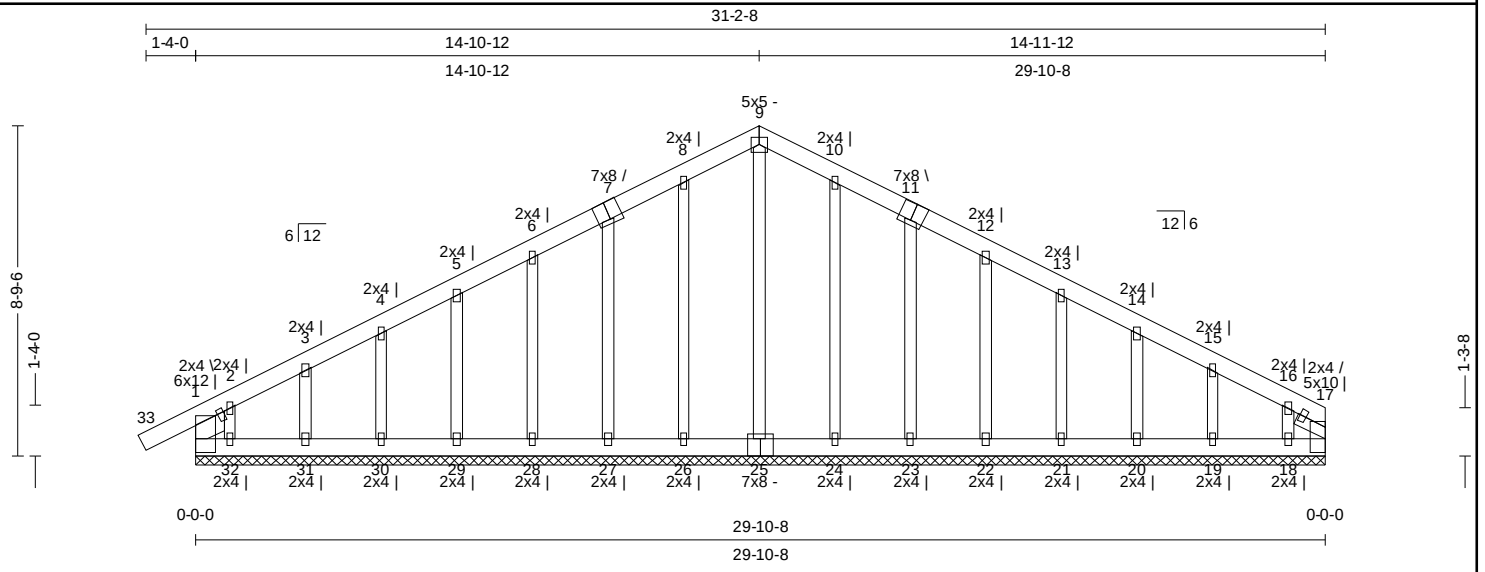
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	123 lbs

11) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 14, 25 may need to be considered.
12) Listed wind uplift reactions based on MWFRS & C&C loading.

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: GE02
Job: 091224 Sebring
Date: 09/19/24 13:59:48
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
29-10-8	6/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	191 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.58 (33-1)	Vert TL: 0 in	L / 999	18	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.14 (32-1)	Vert LL: 0 in	L / 999	18	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.58 (10-24)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	992 lbs	297 plf	-203 lbs	-141 lbs	-147 lbs	-203 lbs	241 lbs

Material

TC: SPF 1650/1.5 2 x 6
BC: SPF 1650/1.5 2 x 6
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.400	-341 lbs								
BC											
Web	3-31	0.059	-398 lbs	7-27	0.425	-603 lbs	11-23	0.425	-602 lbs	15-19	0.060	-405 lbs
	4-30	0.089	-429 lbs	8-26	0.570	-596 lbs	12-22	0.287	-579 lbs			
	5-29	0.189	-577 lbs	9-25	0.384	-307 lbs	13-21	0.189	-579 lbs			
	6-28	0.293	-593 lbs	10-24	0.581	-607 lbs	14-20	0.089	-427 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
7) A creep factor of 1.50 has been applied for this truss analysis.
8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 32, 1 may need to be considered.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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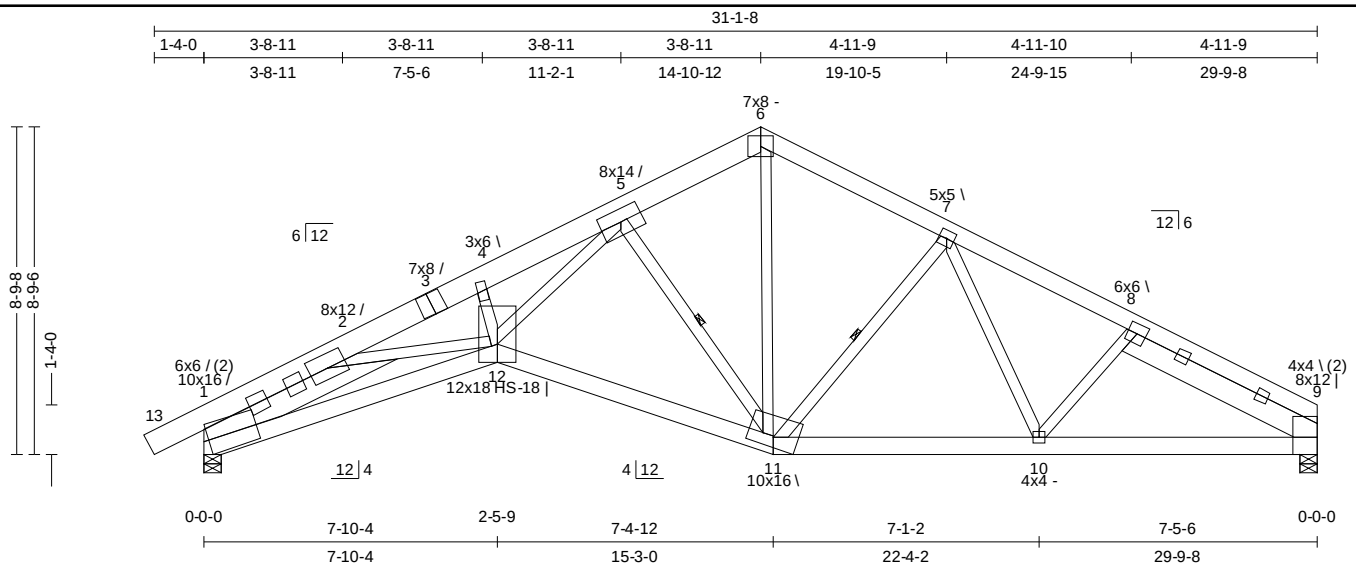
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss:GE03
Job: 091224 Sebring
Date: 09/19/24 13:59:50
Page: 1 of 1

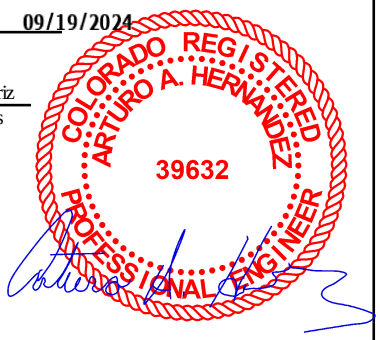
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
29-9-8	6/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	240 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 81.9	Bldg Code : IBC 2021/	TC : 0.94 (7-8)	Vert TL: 0.56 in	L / 624	(11-12)	L / 240
TCDL : 15(rake)	TPI 1-2014	BC : 0.67 (12-1)	Vert LL: 0.37 in	L / 938	(11-12)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.91 (5-11)	Horz TL: 0.33 in		9	
BCDL : 10(rake)	Lumber D.O.L. : 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.48 in	3,750 lbs	·	-118 lbs	-175 lbs	-175 lbs	52 lbs
9	1	5.5 in	3.41 in	3,427 lbs	·	-94 lbs	-108 lbs	-108 lbs	·

Material

TC: DFL 1950/1.7 2 x 8 except:
SPF 1650/1.5 2 x 6: 6-9
BC: DFL 2400/2.0 2 x 6
Web: HF #2 2 x 4 except:
DFL 2400/2.0 2 x 4: 5-12

Bracing

TC: Sheathed or Purlins at 2-9-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 5-11, 7-11

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC L L = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.326	-5,384 lbs		4-5	0.539	-9,665 lbs		6-7	0.907	-3,609 lbs		8-9	0.543	-3,096 lbs				
	2-4	0.558	-9,913 lbs		5-6	0.434	-3,438 lbs		7-8	0.942	-4,848 lbs								
BC	9-10	0.558	4,348 lbs	(-102 lbs)	10-11	0.377	3,933 lbs	(-30 lbs)	11-12	0.521	4,944 lbs	(-20 lbs)	12-1	0.672	7,350 lbs	(-150 lbs)			
Web	2-12	0.409	1,690 lbs		5-12	0.587	5,929 lbs	(-81 lbs)	6-11	0.470	1,944 lbs	(-122 lbs)	7-10	0.100	414 lbs	(-4 lbs)			
	4-12	0.099	-713 lbs		5-11	0.908	-3,498 lbs		7-11	0.465	-1,662 lbs		8-10	0.131	-407 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.50 has been applied for this truss analysis.
6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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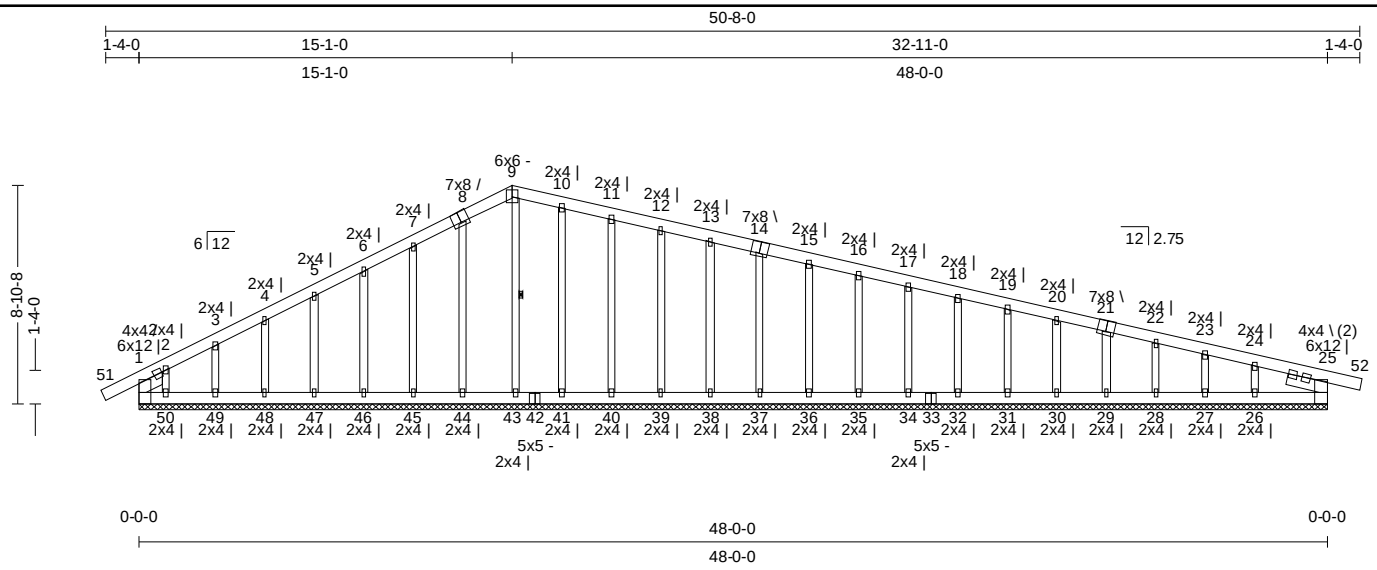
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss:GE04
Job: 091224 Sebring
Date: 09/19/24 13:59:52
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
48-0-0	6/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	302 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 81.9	Bldg Code : IBC 2021/	TC : 0.63 (25-52)	Vert TL: 0 in	L / 999	25	L / 240
TCDL : 15(rake)	TPI 1-2014	BC : 0.19 (25-26)	Vert LL: 0 in	L / 999	25	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.65 (8-44)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,103 lbs	304 plf	-471 lbs	-281 lbs	-	-471 lbs	-251 lbs

Material

TC: SPF 1650/1.5 2 x 6
BC: SPF 1650/1.5 2 x 6
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 9-43

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	24-25	0.401	-427 lbs								
BC											
Web	3-49	0.081	-539 lbs	9-43	0.136	-390 lbs	15-36	0.296	-517 lbs	21-29	0.070	-389 lbs
	4-48	0.145	-669 lbs	10-41	0.597	-519 lbs	16-35	0.206	-429 lbs	22-28	0.060	-387 lbs
	5-47	0.225	-659 lbs	11-40	0.541	-532 lbs	17-34	0.156	-389 lbs	23-27	0.055	-391 lbs
	6-46	0.339	-660 lbs	12-39	0.468	-524 lbs	18-32	0.128	-389 lbs	24-26	0.051	-387 lbs
	7-45	0.463	-637 lbs	13-38	0.409	-527 lbs	19-31	0.104	-389 lbs			
	8-44	0.652	-664 lbs	14-37	0.353	-527 lbs	20-30	0.085	-391 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
7) Gable must be sheathed on one side or lateral bracing applied appropriately.
8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
9) A creep factor of 1.50 has been applied for this truss analysis.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

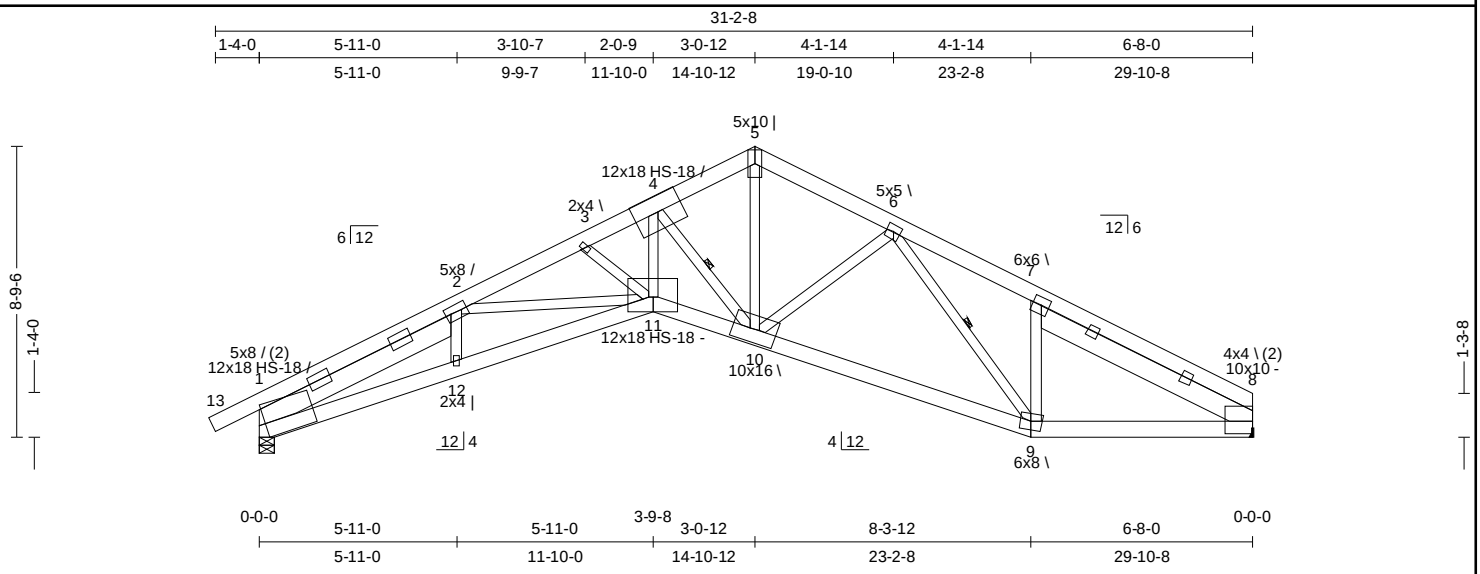
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
48-0-0	6/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	302 lbs

10) ⋱ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
11) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 50, 1, 25 may need to be considered.
12) Listed wind uplift reactions based on MWFRS & C&C loading.

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: S01
Job: 091224 Sebring
Date: 09/19/24 13:59:54
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
29-10-8	6/12	10	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	231 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.82 (4-5)	Vert TL: 0.83 in	L / 425	(11-12)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.75 (11-12)	Vert LL: 0.55 in	L / 636	(11-12)	L / 360
BCLL: 0	Rep Mbr: Yes	Web: 0.94 (4-10)	Horz TL: 0.58 in		8	
BCDL: 10(rake)	Lumber D.O.L.: 100 %					

09/19/2024

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.49 in	3,752 lbs	·	-114 lbs	-171 lbs	-171 lbs	52 lbs
8	1	1.5 in	---	3,439 lbs	·	-92 lbs	-106 lbs	-106 lbs	·

Material

TC: SPF 1650/1.5 2 x 6
BC: DFL 2400/2.0 2 x 6
Web: HF #2 2 x 4 except:
DFL 2400/2.0 2 x 4: 4-11

Bracing

TC: Sheathed or Purlins at 2-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-10, 6-9

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TCLL = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.770	-4,031 lbs		3-4	0.570	-8,768 lbs		5-6	0.820	-5,210 lbs		7-8	0.762	-3,225 lbs
	2-3	0.700	-9,672 lbs		4-5	0.821	-5,077 lbs		6-7	0.795	-5,254 lbs				
BC	8-9	0.511	4,459 lbs	(-62 lbs)	10-11	0.696	8,011 lbs		12-1	0.691	8,553 lbs	(-119 lbs)			
	9-10	0.386	5,128 lbs	(-37 lbs)	11-12	0.749	8,594 lbs	(-123 lbs)							
Web	2-11	0.227	939 lbs		4-10	0.937	-5,644 lbs		6-9	0.346	-1,199 lbs				
	3-11	0.180	-1,016 lbs		5-10	0.832	3,438 lbs	(-106 lbs)	7-9	0.265	-897 lbs				
	4-11	0.572	5,781 lbs	(-29 lbs)	6-10	0.406	-763 lbs								

Notes

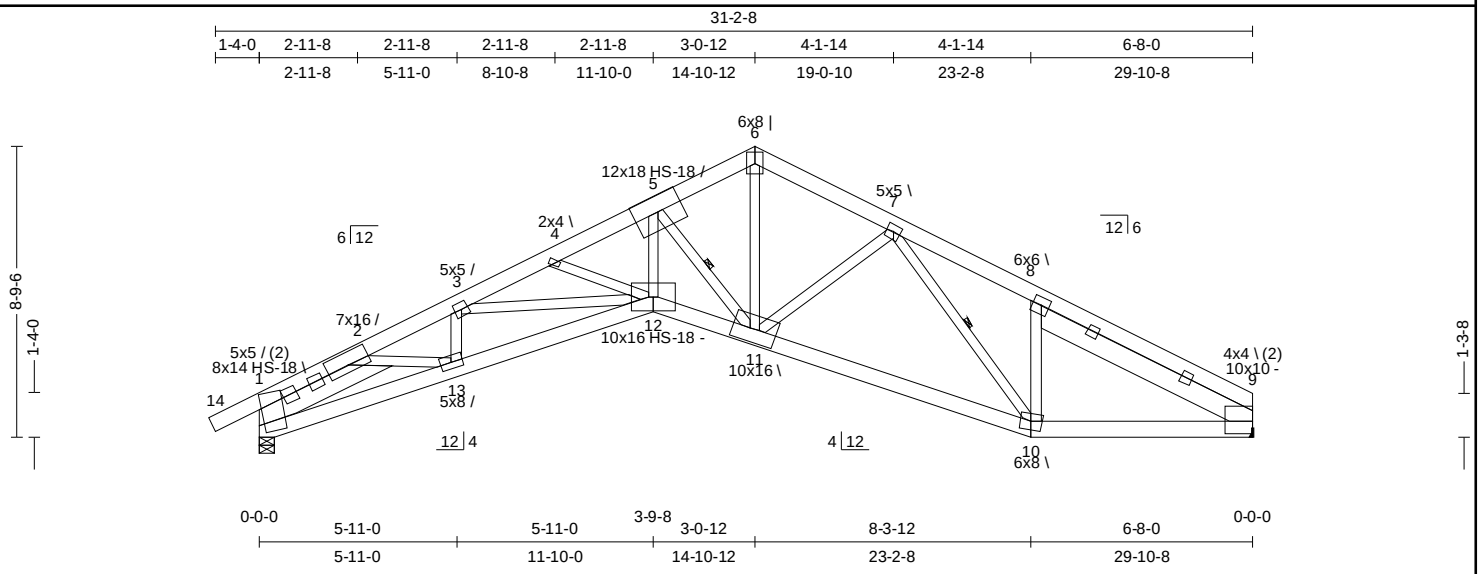
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: S02
Job: 091224 Sebring
Date: 09/19/24 13:59:56
Page: 1 of 1

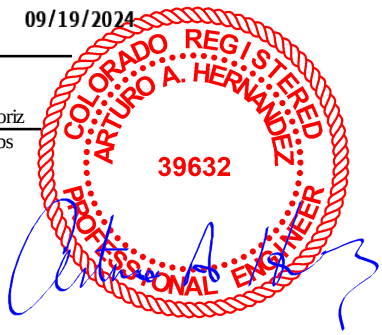
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
29-10-8	6/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	237 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.63 (6-7)	Vert TL: 0.72 in	L / 492	(12-13)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.83 (12-13)	Vert LL: 0.48 in	L / 735	(12-13)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.93 (5-11)	Horz TL: 0.52 in		9	
BCDL: 10(rake)	Lumber D.O.L.: 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.49 in	3,752 lbs	·	-114 lbs	-171 lbs	-171 lbs	52 lbs
9	1	1.5 in	---	3,439 lbs	·	-92 lbs	-106 lbs	-106 lbs	·

Material

TC: DFL 2400/2.0 2 x 6
BC: DFL 2400/2.0 2 x 6
Web: HF #2 2 x 4 except:
DFL 2400/2.0 2 x 4: 5-12

Bracing

TC: Sheathed or Purlins at 2-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 5-11, 7-10

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TCLL = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.357	-4,024 lbs		4-5	0.582	-8,804 lbs		7-8	0.605	-5,262 lbs						
	2-3	0.573	-9,286 lbs		5-6	0.623	-5,082 lbs		8-9	0.570	-3,343 lbs						
	3-4	0.609	-9,912 lbs		6-7	0.635	-5,213 lbs										
BC	9-10	0.404	4,463 lbs	(-61 lbs)	11-12	0.791	8,009 lbs		13-1	0.456	6,167 lbs	(-120 lbs)					
	10-11	0.435	5,121 lbs	(-37 lbs)	12-13	0.831	8,642 lbs	(-108 lbs)									
Web	2-13	0.575	2,376 lbs		4-12	0.259	-1,147 lbs		6-11	0.825	3,409 lbs	(-109 lbs)	8-10	0.267	-905 lbs		
	3-13	0.137	-988 lbs		5-12	0.549	5,548 lbs	(-10 lbs)	7-11	0.394	-741 lbs						
	3-12	0.217	896 lbs		5-11	0.934	-5,629 lbs		7-10	0.340	-1,179 lbs						

Notes

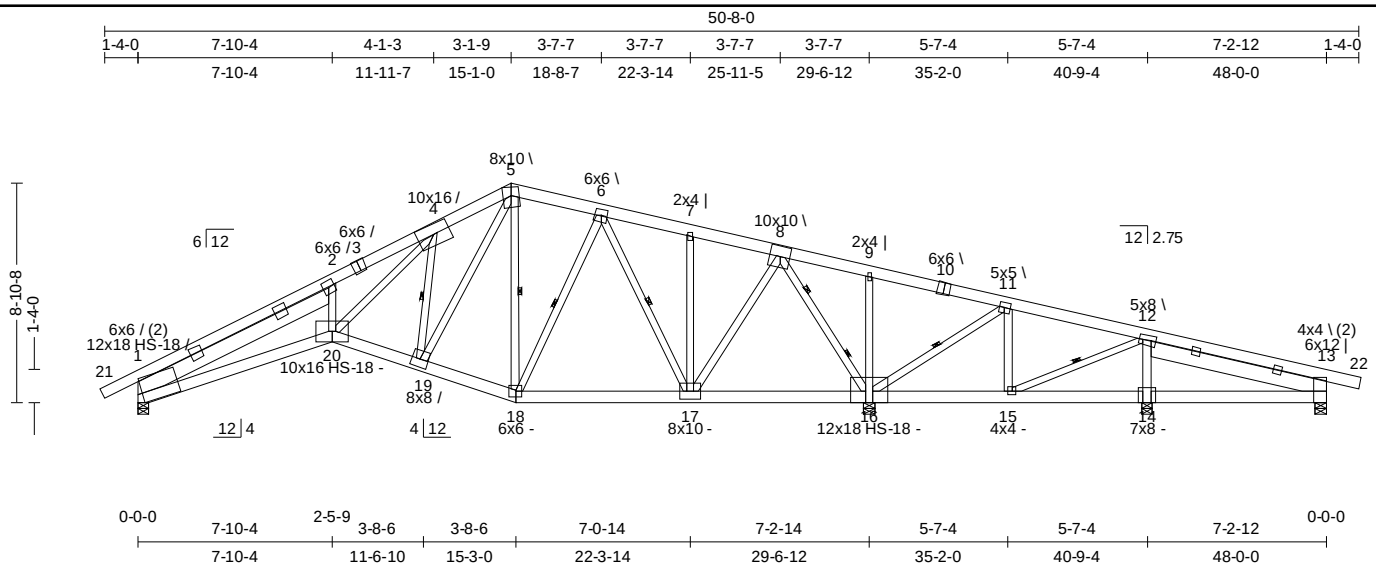
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: S03
Job: 091224 Sebring
Date: 09/19/24 13:59:59
Page: 1 of 2

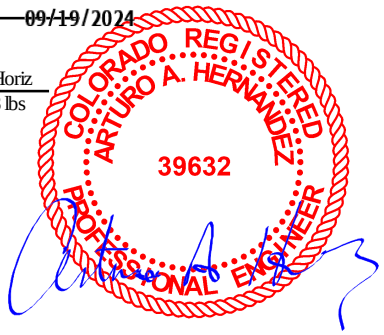
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
48-0-0	6/12	6	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	368 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 81.9	Bldg Code : IBC 2021/	TC : 0.98 (1-2)	Vert TL: 0.61 in	L / 568	(19-20)	L / 240
TCDL : 15(rake)	TPI 1-2014	BC : 0.83 (20-1)	Vert LL: 0.45 in	L / 771	(19-20)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.92 (5-19)	Horz TL: 0.35 in		16	
BCDL : 10(rake)	Lumber D.O.L. : 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.47 in	3,730 lbs	·	-106 lbs	·	-106 lbs	-118 lbs
16	1	5.5 in	3.67 in	5,897 lbs	·	-227 lbs	·	-227 lbs	·
14	1	5.5 in	1.50 in	1,306 lbs	·	-51 lbs	·	-51 lbs	·
13	1	5.5 in	1.50 in	893 lbs	·	-70 lbs	·	-70 lbs	·

Material

TC: DFL 2400/2.0 2 x 6 except:
SPF 1650/1.5 2 x 6: 5-10, 10-22
BC: DFL 2400/2.0 2 x 6
Web: HF #2 2 x 4 except:
DFL 2400/2.0 2 x 4: 4-20

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 7-10-0, Purlin design by Others.
Web: One Midpoint Row: 4-19, 5-18, 6-18, 6-17, 11-16, 12-15
Two Third Point Rows: 8-16

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC L L = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.978	-5,034 lbs		5-6	0.634	-2,143 lbs		8-9	0.731	2,906 lbs		12-13	0.839	-838 lbs				
	2-4	0.513	-9,177 lbs		6-7	0.631	-1,582 lbs		9-11	0.756	2,947 lbs	(-21 lbs)							
	4-5	0.415	-4,470 lbs		7-8	0.639	-1,586 lbs		11-12	0.759	1,857 lbs								
BC	13-14	0.104	-692 lbs		15-16	0.119	-1,764 lbs		17-18	0.178	1,901 lbs		19-20	0.563	3,691 lbs				
	14-15	0.091	-692 lbs		16-17	0.156	-866 lbs		18-19	0.214	2,182 lbs		20-1	0.828	8,310 lbs	(-28 lbs)			
Web	2-20	0.162	-1,085 lbs		5-18	0.638	-1,838 lbs		8-17	0.649	2,681 lbs	(-33 lbs)	11-15	0.132	544 lbs				
	4-20	0.634	6,411 lbs	(-120 lbs)	6-18	0.352	1,456 lbs	(-84 lbs)	8-16	0.871	-4,254 lbs		12-15	0.249	-1,176 lbs				
	4-19	0.681	-3,683 lbs		6-17	0.721	-2,061 lbs		9-16	0.589	-1,072 lbs		12-14	0.205	-1,113 lbs				
	5-19	0.920	3,804 lbs	(-99 lbs)	7-17	0.772	-942 lbs		11-16	0.435	-1,794 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: S03
Job: 091224 Sebring
Date: 09/19/24 13:59:59

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
48-0-0	6/12	6	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	368 lbs

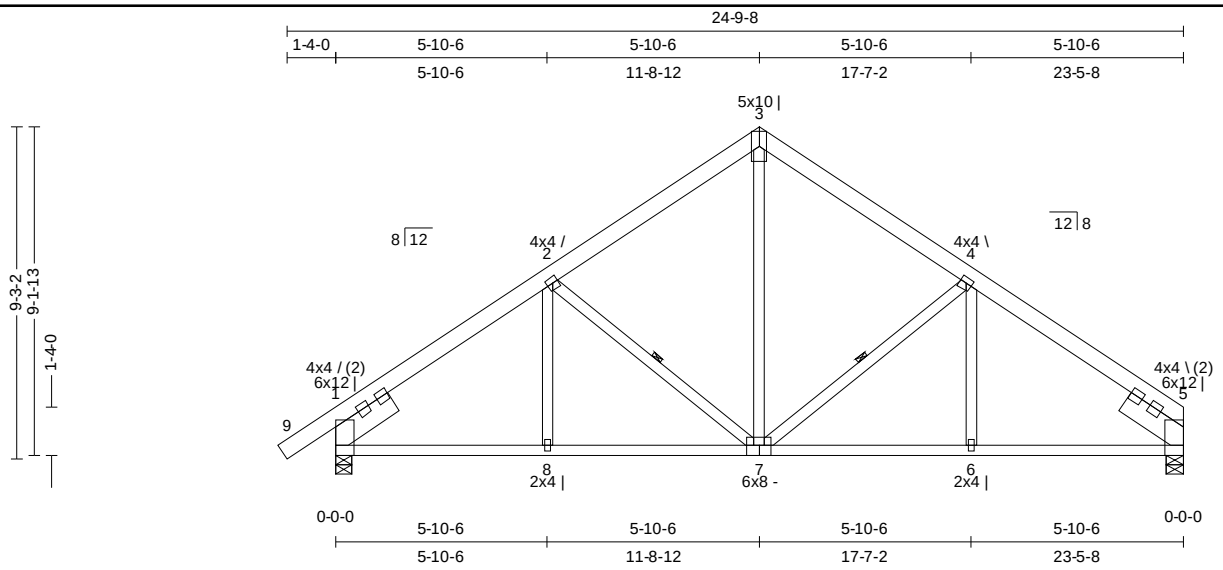
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.50 has been applied for this truss analysis.
6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: T01
Job: 091224 Sebring
Date: 09/19/24 14:00:01
Page: 1 of 1

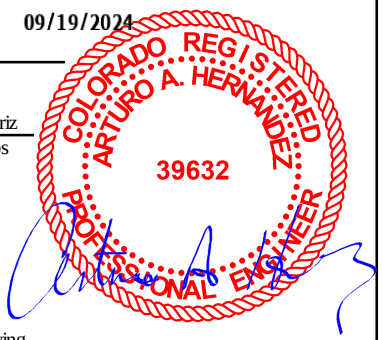
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
23-5-8	8/12	6	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	142 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.86 (2-3)	Vert TL: 0.24 in	L / 999	(7-8)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.67 (8-1)	Vert LL: 0.15 in	L / 999	(7-8)	L / 360
BCLL: 0	Rep Mbr: Yes	Web: 0.34 (4-7)	Horz TL: 0.07 in		5	
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.18 in	3,194 lbs	·	-79 lbs	-59 lbs	-79 lbs	100 lbs
5	1	5.5 in	2.86 in	2,871 lbs	·	-52 lbs	-4 lbs	-52 lbs	·

Material

TC: DFL 2400/2.0 2 x 6
BC: DFL 2400/2.0 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 4-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-7, 4-7

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC LL = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.827	-3,574 lbs		3-4	0.852	-2,617 lbs								
	2-3	0.859	-2,603 lbs		4-5	0.761	-3,608 lbs								
BC	5-6	0.637	2,681 lbs		6-7	0.401	2,681 lbs		7-8	0.400	2,637 lbs		8-1	0.675	2,637 lbs	(-118 lbs)
Web	2-7	0.326	-1,104 lbs		4-7	0.340	-1,151 lbs								
	3-7	0.244	1,011 lbs	(-92 lbs)											

Notes

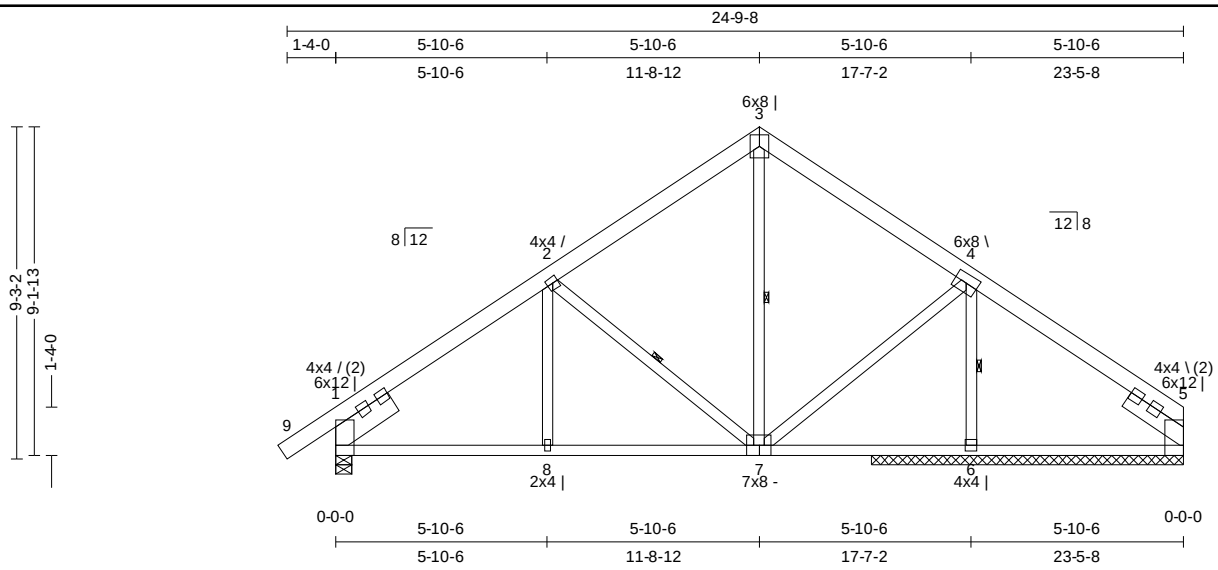
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.50 has been applied for this truss analysis.
6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: T02
Job: 091224 Sebring
Date: 09/19/24 14:00:03
Page: 1 of 1

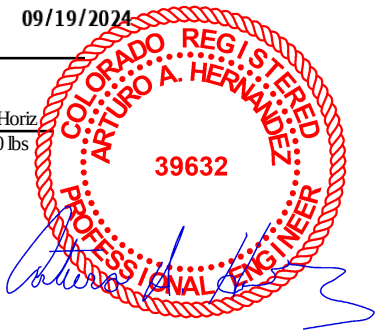
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	137 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.84 (3-4)	Vert TL: 0.17 in	L / 999	(7-8)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.82 (8-1)	Vert LL: 0.11 in	L / 999	(7-8)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.42 (4-6)	Horz TL: 0.04 in		6	
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	4.12 in	2,626 lbs	·	-71 lbs	-61 lbs	-71 lbs	100 lbs
6	1	103.5 in	N/A	2,721 lbs	·	-27 lbs	·	-27 lbs	·
5	1	103.5 in	N/A	821 lbs	·	-38 lbs	-20 lbs	-38 lbs	·

Material

TC: DFL 2400/2.0 2 x 6
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-7, 3-7, 4-6

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC LL = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.627	-2,749 lbs		3-4	0.838	-1,569 lbs								
	2-3	0.541	-1,688 lbs		4-5	0.752	-791 lbs								
BC	5-6	0.306	386 lbs	(-116 lbs)	6-7	0.306	386 lbs	(-116 lbs)	7-8	0.546	1,997 lbs		8-1	0.815	1,997 lbs	(-154 lbs)
Web	2-7	0.383	-1,297 lbs		4-6	0.424	-2,586 lbs								
	4-7	0.348	1,438 lbs												

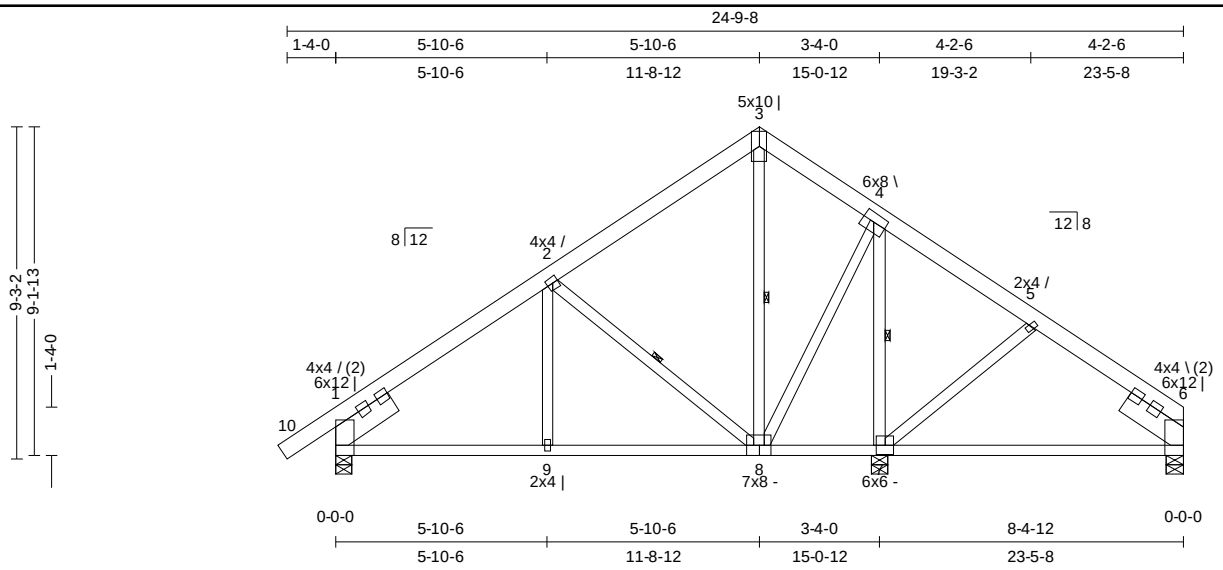
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.50 has been applied for this truss analysis.
6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

	Western Slope Truss Mfg 175 Clay Ave. Craig, CO 81625 Phone: 970-826-0840	Truss: T03 Job: 091224 Sebring Date: 09/19/24 14:00:05 Page: 1 of 1

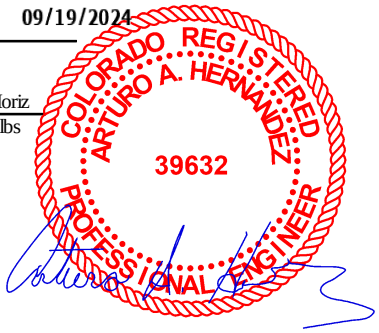
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
23-5-8	8/12	1	1-4-0	0-0-0	0-0-0	0-0-0	1	24 in	138 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.81 (4-5)	Vert TL: 0.14 in	L / 642	(6-7)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.73 (9-1)	Vert LL: 0.07 in	L / 999	(6-7)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.52 (5-7)	Horz TL: 0.03 in		6	
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.66 in	2,334 lbs	·	-59 lbs	-54 lbs	-59 lbs	100 lbs
7	1	5.5 in	4.47 in	2,846 lbs	·	-54 lbs	-14 lbs	-54 lbs	·
6	1	5.5 in	1.64 in	1,044 lbs	·	-18 lbs	·	-18 lbs	·

Material

TC: SPF 1650/1.5 2 x 6
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-8, 3-8, 4-7

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC LL = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.710	-2,315 lbs	3-4	0.665	-764 lbs	5-6	0.427	-994 lbs		
	2-3	0.672	-1,224 lbs	4-5	0.812	-398 lbs					
BC	6-7	0.403	725 lbs	8-9	0.494	1,667 lbs	9-1	0.731	1,667 lbs	(-202 lbs)	
Web	2-8	0.408	-1,380 lbs	4-8	0.392	1,621 lbs	5-7	0.519	-901 lbs		
	3-8	0.185	-486 lbs	4-7	0.496	-2,147 lbs					

Notes

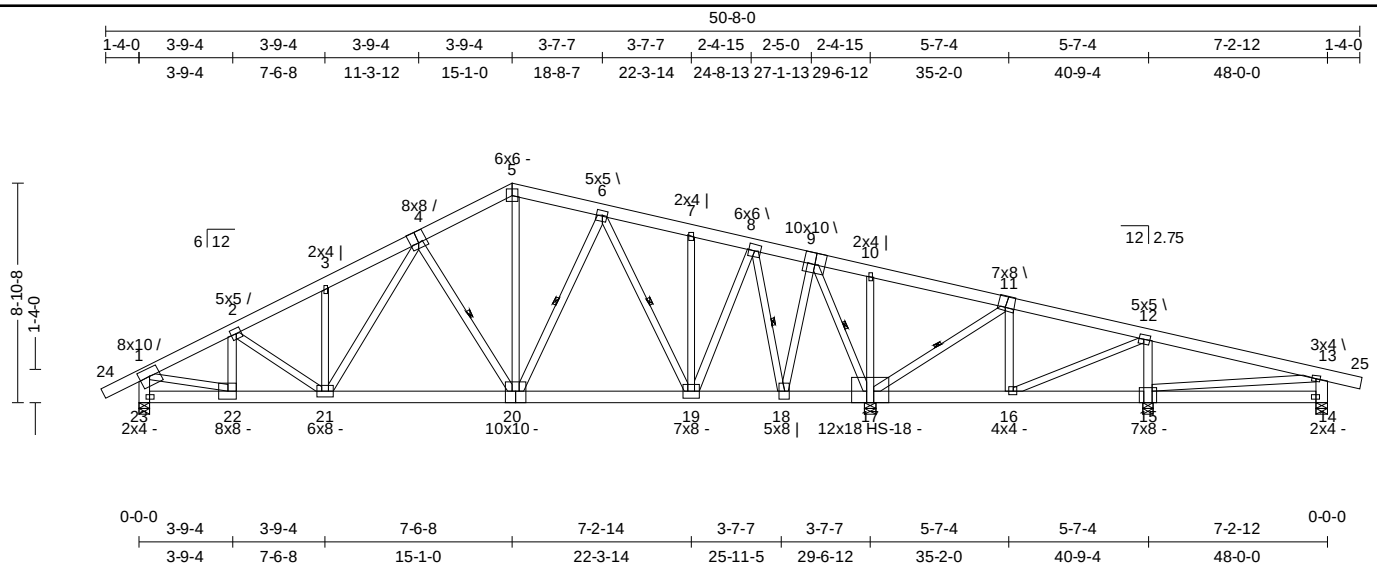
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.50 has been applied for this truss analysis.
6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: T04
Job: 091224 Sebring
Date: 09/19/24 14:00:07
Page: 1 of 2

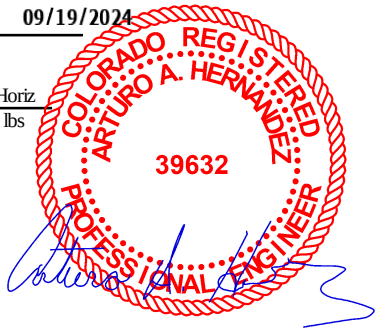
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
48-0-0	6/12	5	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	334 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.76 (12-13)	Vert TL: 0.25 in	L / 999	(20-21)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.55 (21-22)	Vert LL: 0.16 in	L / 999	(20-21)	L / 360
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (1-22)	Horz TL: 0.07 in		17	
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
23	1	5.5 in	1.60 in	4,081 lbs	·	-131 lbs	·	-131 lbs	-69 lbs
17	1	5.5 in	3.45 in	5,082 lbs	·	-211 lbs	·	-211 lbs	·
15	1	5.5 in	1.50 in	1,748 lbs	·	-55 lbs	·	-55 lbs	·
14	1	5.5 in	1.50 in	957 lbs	·	-66 lbs	·	-66 lbs	·

Material

TC: DFL 2400/2.0 2 x 6
BC: SPF 1650/1.5 2 x 6
Web: HF #2 2 x 4 except:
SPF 1650/1.5 2 x 4: 1-22
SPF 1650/1.5 2 x 6: 1-23, 13-14

Bracing

TC: Sheathed or Purlins at 4-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-20, 6-20, 6-19, 8-18, 9-17, 11-17

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC L L = 20 psf.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.356	-4,930 lbs		5-6	0.482	-2,702 lbs		9-10	0.405	910 lbs					
	2-3	0.542	-5,145 lbs		6-7	0.331	-2,256 lbs		10-11	0.555	1,026 lbs					
	3-4	0.515	-5,131 lbs		7-8	0.290	-2,225 lbs		11-12	0.624	-359 lbs					
	4-5	0.582	-3,210 lbs		8-9	0.377	-1,173 lbs									
BC	17-18	0.231	499 lbs		20-21	0.487	3,646 lbs	(-6 lbs)								
	18-19	0.212	1,497 lbs		21-22	0.546	4,252 lbs	(-80 lbs)								
	19-20	0.353	2,416 lbs													
Web	1-23	0.292	-4,018 lbs		4-20	0.571	-1,960 lbs		8-18	0.511	-2,359 lbs		13-14	0.066	-904 lbs	
	1-22	0.823	4,406 lbs	(-67 lbs)	5-20	0.247	1,020 lbs	(-70 lbs)	9-18	0.575	2,377 lbs	(-58 lbs)				
	2-22	0.189	-1,134 lbs		6-20	0.220	909 lbs	(-424 lbs)	9-17	0.711	-3,456 lbs					
	2-21	0.079	325 lbs		6-19	0.462	-1,320 lbs		10-17	0.536	-975 lbs					
	3-21	0.457	-1,212 lbs		7-19	0.649	-792 lbs		11-17	0.300	-1,237 lbs					
	4-21	0.350	1,447 lbs	(-67 lbs)	8-19	0.431	1,782 lbs	(-28 lbs)	12-15	0.289	-1,570 lbs					

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Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
48-0-0	6/12	5	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	334 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.50 has been applied for this truss analysis.
6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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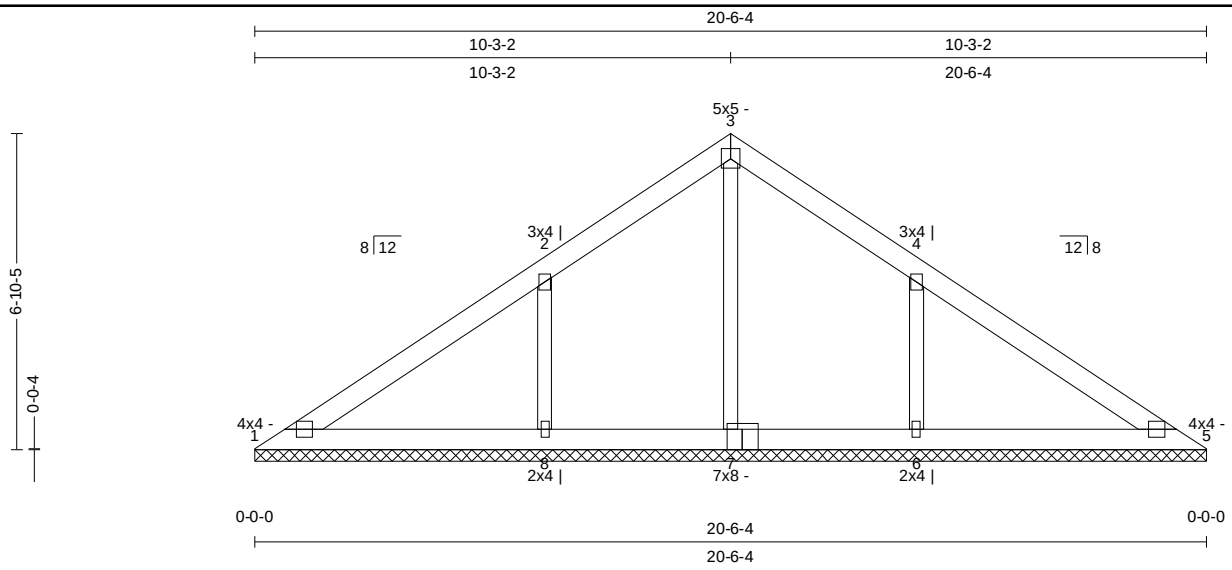
Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: V01
Job: 091224 Sebring
Date: 09/19/24 14:00:09
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	90 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.81 (3-4)	Vert TL:	0.01 in	L / 999	(5-6)	L / 240
TCDL: 15(rake)	TPI 1-2014	BC: 0.53 (5-6)	Vert LL:	0 in	L / 999	(8-1)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.37 (4-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.: 100 %						

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,628 lbs	347 plf	-357 lbs	-112 lbs	-103 lbs	-357 lbs	714 lbs

Material

TC: SPF 1650/1.5 2 x 6
BC: SPF 1650/1.5 2 x 6
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.777	-702 lbs	2-3	0.807	-717 lbs	3-4	0.807	-698 lbs	4-5	0.777	-684 lbs
BC												
Web	2-8	0.371	-1,505 lbs	3-7	0.283	-396 lbs	4-6	0.371	-1,506 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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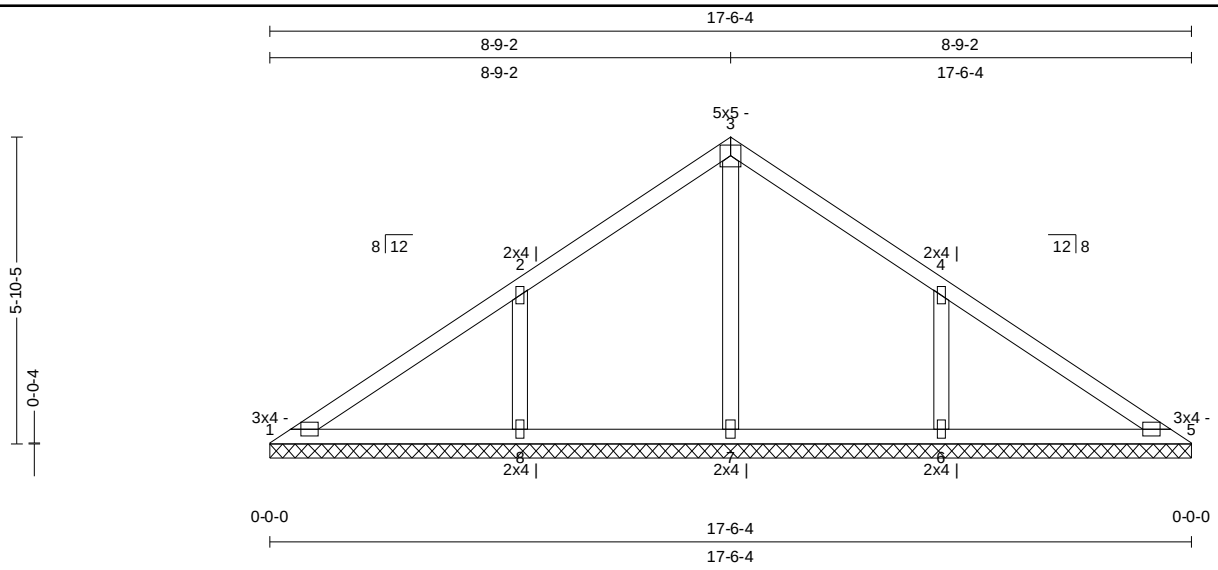
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: V02
Job: 091224 Sebring
Date: 09/19/24 14:00:10
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
17-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	57 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 81.9	Bldg Code : IBC 2021/	TC : 0.90 (3-4)	Vert TL: 0.01 in	L / 999	(5-6)	L / 240
TCDL : 15(rake)	TPI 1-2014	BC : 0.73 (5-6)	Vert LL: 0.01 in	L / 999	(5-6)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.27 (3-7)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,463 lbs	371 plf	-374 lbs	-96 lbs	-106 lbs	-374 lbs	724 lbs

Material

TC: DFL 2400/2.0 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.873	-635 lbs	2-3	0.901	-680 lbs	3-4	0.901	-680 lbs	4-5	0.873	-635 lbs
BC												
Web	2-8	0.251	-1,344 lbs	3-7	0.265	-461 lbs	4-6	0.251	-1,344 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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Western Slope Truss Mfg
175 Clay Ave.
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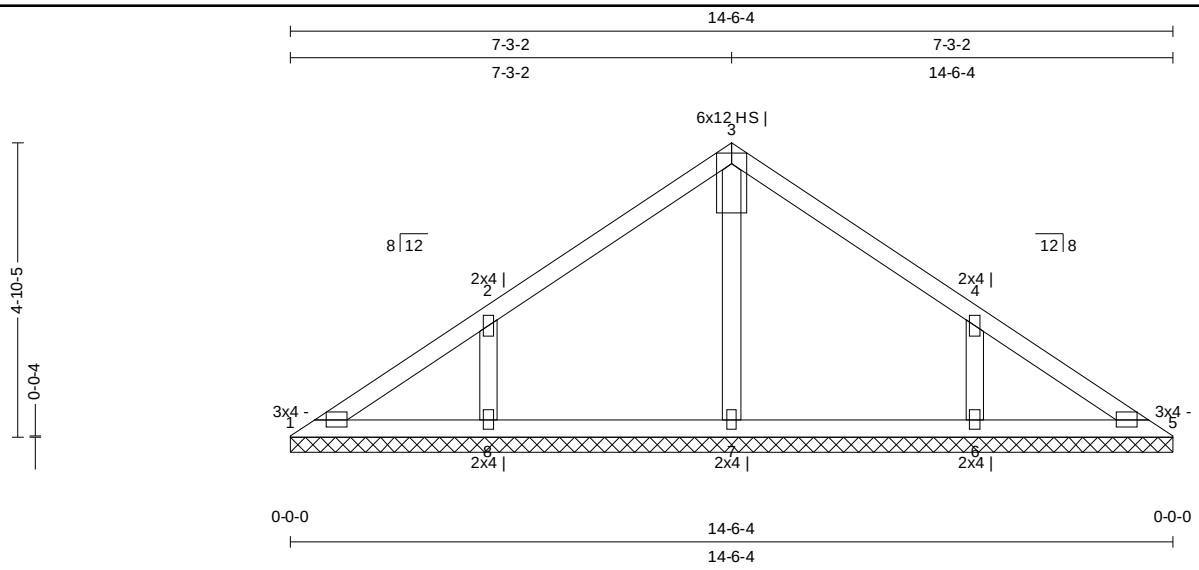
Phone: 970-826-0840

Truss: V03
Job: 091224 Sebring
Date: 09/19/24 14:00:12
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
14-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	47 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.86 (3-4)	Vert TL: 0.01 in	L/999	(6-7)	L/240
TCDL: 15(rake)	TPI 1-2014	BC: 0.10 (6-7)	Vert LL: 0 in	L/999	5	L/360
BCLL: 0	Rep Mbr: No	Web: 0.21 (3-7)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,235 lbs	306 plf	-27 lbs	-81 lbs	-108 lbs	-108 lbs	340 lbs

Material

TC: DFL 2400/2.0 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.679	-451 lbs	2-3	0.857	-649 lbs	3-4	0.857	-649 lbs	4-5	0.679	-451 lbs
BC												
Web	2-8	0.164	-1,163 lbs	3-7	0.206	-528 lbs	4-6	0.164	-1,163 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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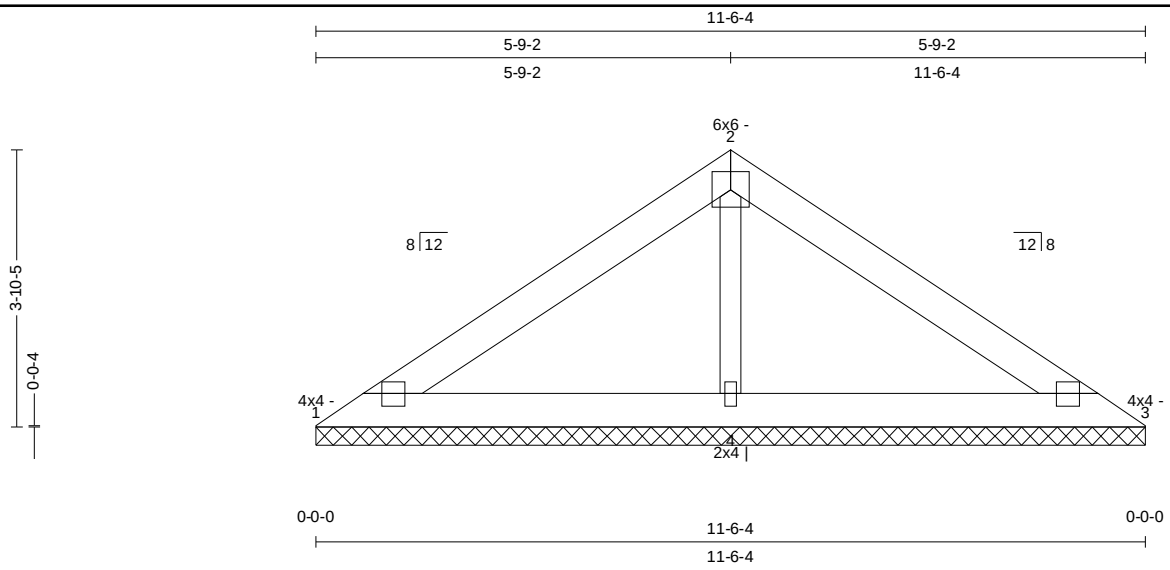
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: V04
Job: 091224 Sebring
Date: 09/19/24 14:00:14
Page: 1 of 1

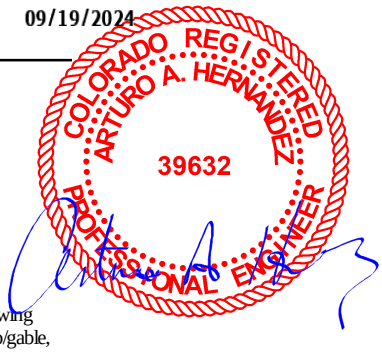
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
11-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	45 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 81.9	Bldg Code: IBC 2021/	TC: 0.88 (2-3)	Vert TL: 0.01 in	L/999	(3-4)	L/240
TCDL: 15(rake)	TPI 1-2014	BC: 0.46 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (2-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

09/19/2024



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,732 lbs	364 plf	-304 lbs	-87 lbs	-150 lbs	-304 lbs	768 lbs

Material

TC: SPF 1650/1.5 2 x 6
BC: SPF 1650/1.5 2 x 6
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.878	-849 lbs	2-3	0.878	-849 lbs					
BC											
Web	2-4	0.104	-491 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 4x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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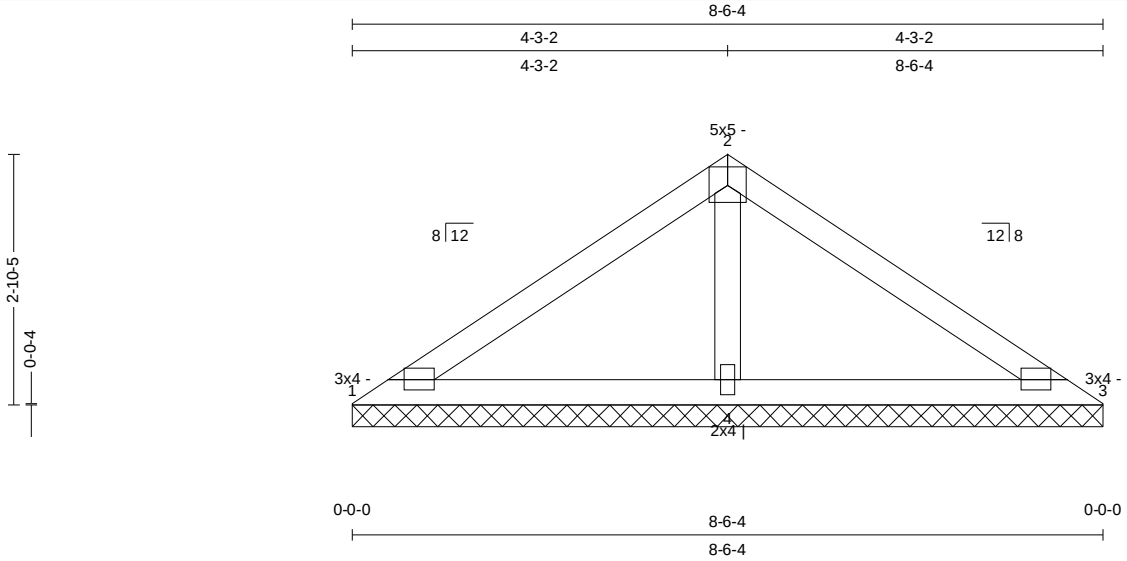
Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: V05
Job: 091224 Sebring
Date: 09/19/24 14:00:15
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
8-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	24 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	81.9	Bldg Code :	IBC 2021/ TPI 1-2014	TC :	0.79 (2-3)	Vert TL:	0.01 in	L / 999	(3-4)	L / 240
TCDL :	15(rake)	Rep Mbr :	No	BC :	0.63 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 360
BCLL :	0	Lumber D.O.L. :	100 %	Web :	0.06 (2-4)	Horz TL:	0 in			
BCDL :	10									

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,414 lbs	403 plf	-321 lbs	-72 lbs	-153 lbs	-321 lbs	694 lbs

Material

TC: DFL 2400/2.0 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.790	-646 lbs	2-3	0.790	-646 lbs					
BC											
Web	2-4	0.063	-383 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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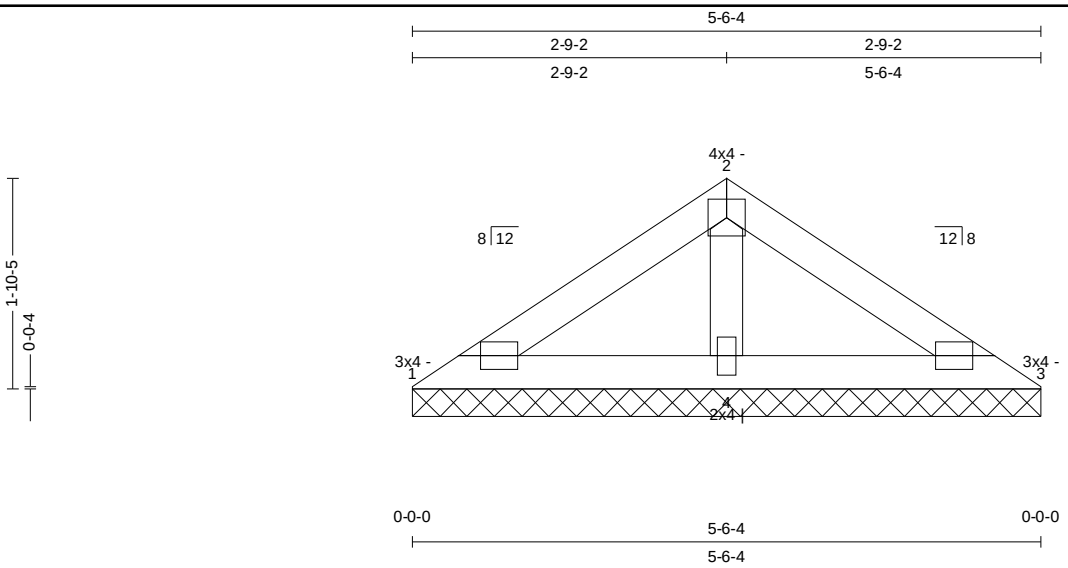
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: V06
Job: 091224 Sebring
Date: 09/19/24 14:00:17
Page: 1 of 1

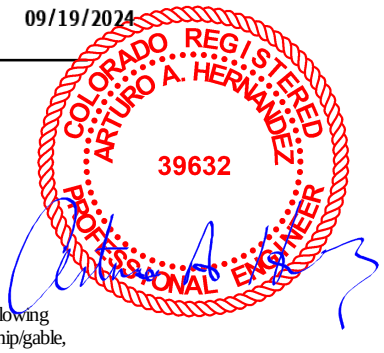
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
5-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 81.9	Bldg Code : IBC 2021/	TC : 0.61 (2-3)	Vert TL: 0 in	L / 999	(3-4)	L / 240
TCDL : 15(rake)	TPI 1-2014	BC : 0.19 (3-4)	Vert LL: 0 in	L / 999	(3-4)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.03 (2-4)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

09/19/2024



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	676 lbs	313 plf	-72 lbs	-33 lbs	-93 lbs	-93 lbs	263 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: HF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 117 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.610	-339 lbs		2-3	0.610	-339 lbs				
BC											
Web											

Notes

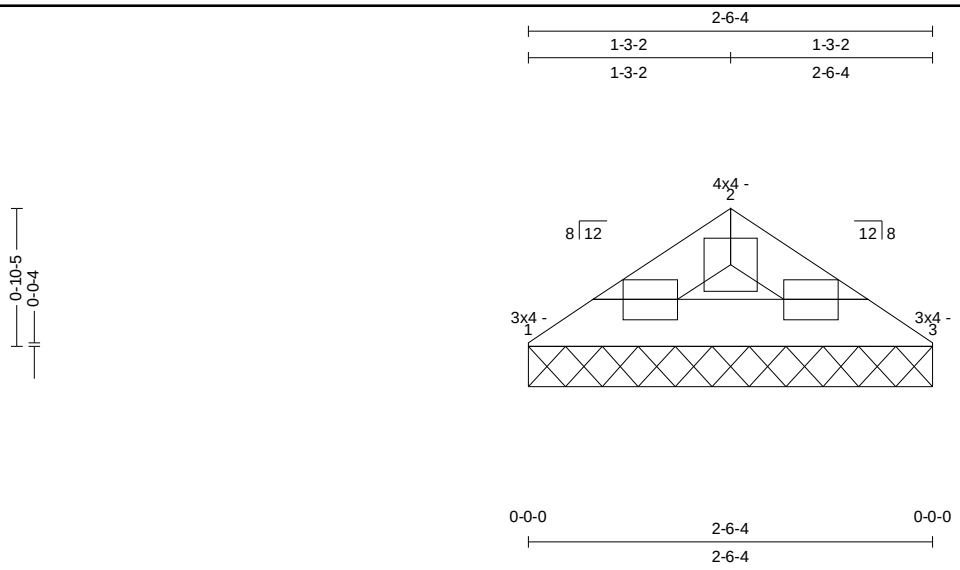
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Western Slope Truss Mfg
175 Clay Ave.
Craig, CO 81625
Phone: 970-826-0840

Truss: V07
Job: 091224 Sebring
Date: 09/19/24 14:00:19
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
2-6-4	8/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	6 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 81.9	Bldg Code : IBC 2021/	TC : 0.13 (2-3)	Vert TL: 0 in	L / 999	3	L / 240
TCDL : 15(rake)	TPI 1-2014	BC : 0.03 (3-1)	Vert LL: 0 in	L / 999	3	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.00 (1)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

09/19/2024

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	201 lbs	218 plf	-	-10 lbs	-34 lbs	-34 lbs	-106 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web:

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (81.9 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 117 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.00. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of TC L L = 20 psf.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.50 has been applied for this truss analysis.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Bearing Block(s) Capacity [lbs]										
Species	Rows	Nail	Blocks	Bearing	MIN Spacing	Nail Spacing				
						Min	3"	4"	5"	6"
8d BOX										
Douglans Fir-Larch	2	0.113	1	3.5*	1.375	1,662	795	650	506	361
Douglans Fir-Larch	3	0.113	1	3.5	1.375	2,818	1,301	1,084	867	723
Douglans Fir-Larch	2	0.113	2	3.5*	2.750	1,662	1,590	1,084	795	650
Douglans Fir-Larch	3	0.113	2	3.5*	2.750	2,818	2,529	2,023	1,662	1,373
Douglans Fir-Larch	3	0.113	1	5.5*	1.375	2,818	1,301	1,084	867	723
0.120 DIAMETER GUN										
Douglans Fir-Larch	2	0.120	1	3.5*	1.500	1,793	896	733	570	407
Douglans Fir-Larch	3	0.120	1	3.5	1.500	2,934	1,467	1,222	978	815
Douglans Fir-Larch	2	0.120	2	3.5*	3.000	1,793	1,793	1,222	896	733
Douglans Fir-Larch	3	0.120	2	3.5*	3.000	2,934	2,852	2,282	1,874	1,548
Douglans Fir-Larch	3	0.120	1	5.5*	1.500	2,934	1,467	1,222	978	815
10d OR 12d BOX										
Douglans Fir-Larch	2	0.128	1	3.5*	1.625	1,762	834	556	556	464
Douglans Fir-Larch	3	0.128	1	3.5	1.625	2,967	1,576	1,205	1,113	834
Douglans Fir-Larch	2	0.128	2	3.5*	3.250	1,576	0	1,113	927	834
Douglans Fir-Larch	3	0.128	2	3.5*	3.250	2,967	0	2,411	2,040	1,669
Douglans Fir-Larch	3	0.128	1	5.5*	1.625	2,967	1,576	1,205	1,113	834
8d COMMON OR 0.131 DIAMETER GUN										
Douglans Fir-Larch	2	0.131	1	3.5*	1.625	1,845	874	583	583	486
Douglans Fir-Larch	3	0.131	1	3.5	1.625	3,108	1,651	1,262	1,165	874
Douglans Fir-Larch	2	0.131	2	3.5*	3.250	1,651	0	1,165	971	874
Douglans Fir-Larch	3	0.131	2	3.5*	3.250	3,108	0	2,525	2,137	1,748
Douglans Fir-Larch	3	0.131	1	5.5*	1.625	3,108	1,651	1,262	1,165	874
16d BOX										
Douglans Fir-Larch	2	0.135	1	3.5*	1.625	1,960	928	619	619	516
Douglans Fir-Larch	3	0.135	1	3.5	1.625	3,281	1,753	1,238	1,134	928
Douglans Fir-Larch	2	0.135	2	3.5*	3.250	1,753	0	1,238	1,031	928
Douglans Fir-Larch	3	0.135	2	3.5*	3.250	3,300	0	2,475	2,063	1,856
Douglans Fir-Larch	3	0.135	1	5.5*	1.625	3,300	1,753	1,238	1,134	928
12d COMMON OR 20d BOX										
Douglans Fir-Larch	2	0.148	1	3.5*	1.875	1,764	941	706	588	470
Douglans Fir-Larch	3	0.148	1	3.5	1.875	2,940	1,882	1,411	1,176	1,058
Douglans Fir-Larch	2	0.148	2	3.5*	3.750	1,411	0	1,411	1,058	823
Douglans Fir-Larch	3	0.148	2	3.5*	3.750	2,822	0	2,705	1,999	1,529
Douglans Fir-Larch	3	0.148	1	5.5*	1.875	2,822	1,646	1,411	1,176	941
16d COMMON										
Douglans Fir-Larch	2	0.162	1	3.5*	2.000	1,973	1,127	845	704	564
Douglans Fir-Larch	3	0.162	1	3.5	2.000	3,241	1,973	1,691	1,409	1,127
Douglans Fir-Larch	2	0.162	2	3.5*	4.000	1,691	0	1,691	1,268	986
Douglans Fir-Larch	3	0.162	2	3.5*	4.000	3,100	0	3,100	2,254	1,832
Douglans Fir-Larch	3	0.162	1	5.5*	2.000	3,241	1,973	1,691	1,409	1,127

* = Wider Bearing Does Not Increase Bearing Block Capacity

For Total Capacity, Add (Truss Plies)*(Bearing Width)*625 to Bearing Block(s) Capacity

1=A "2" implicates one bearing enhancer each side of truss

Typical Truss Capacity [lbs]						
Plies x 1.5 = Truss Width	Bearing Width		Fc Perp		Truss Capacity	
(1.5)	(x) (3.5)		(x) (625)		(=) (3,281)	
1.5	3.5		625		3,281	
1.5	5.5		625		5,156	
3	3.5		625		6,563	
3	5.5		625		10,313	
4.5	3.5		625		9,844	
4.5	5.5		625		15,469	
6	3.5		625		13,125	
6	5.5		625		20,625	

Total Reaction = Truss Capacity + Bearing Block(s) Capacity

Truss Reaction <= Bearing Block Allowable

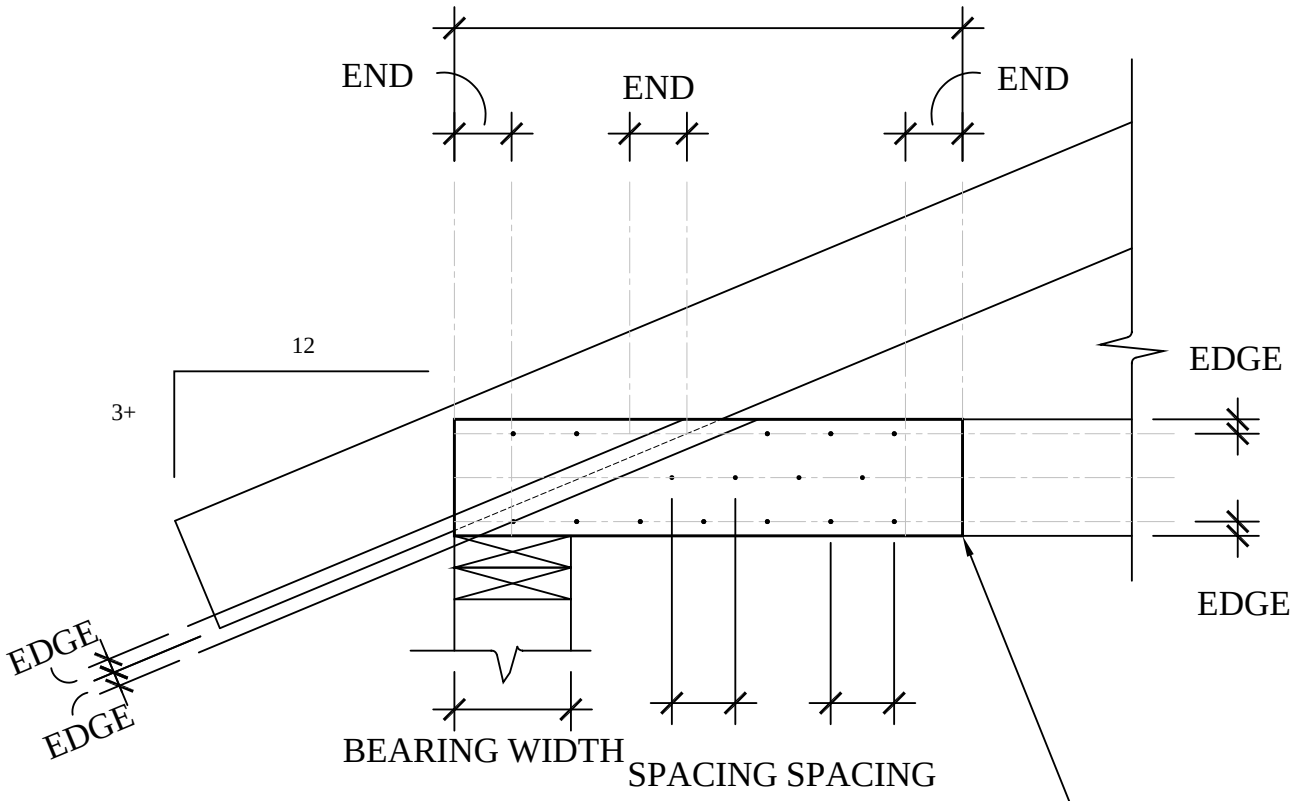
EAGLE METAL

BEARING ENHANCERS
(DFL)

REV:	2.1
ENG:	MDV
CAD:	RC
DATE:	03/16/18

DRAWING NUMBER
DR-61

2'-0" BLOCK UNLESS NOTED OTHERWISE



NAIL TYPE	NAIL PROPERTIES		
	EDGE	MIN SPACING	END
8d BOX (0.113Ø"x2.5")	3/4	1 3/8	1 3/4
10d BOX (0.128Ø"x3.0")	7/8	1 5/8	2
12d BOX (0.128Ø"x3.25")	7/8	1 5/8	2
16d BOX (0.135Ø"x3.5")	7/8	1 5/8	2 1/8
20d BOX (0.148Ø"x4")	1	1 7/8	2 1/4
8d COMMON (0.131Ø"x2.5")	7/8	1 5/8	2
10d COMMON (0.148Ø"x3.0")	1	1 7/8	2 1/4
12d COMMON (0.148Ø"x3.25")	1	1 7/8	2 1/4
16d COMMON (0.162Ø"x3.5")	1	2	2 1/2
0.120"x2.5" GUN	3/4	1 1/2	1 7/8
0.131"x2.5" GUN	7/8	1 5/8	2
0.120"x3.0" GUN	3/4	1 1/2	1 7/8
0.131"x3.0" GUN	7/8	1 5/8	2

ATTACH BLOCK TO TRUSS WITH CONSTRUCTION GRADE WATER PROOF GLUE (SUCH AS "PL400") & (2) STAGGERED ROWS OF NAILS FOR 2x4 OR 2x6, OR (3) STAGGERED ROWS FOR 2x8, 2x10, 2x12. SEE ATTACHED TABLES FOR SPECIFIC NAIL TYPES, CAPACITIES, & REQUIRED NAIL SPACING.

GENERAL NOTES

1. SEE ATTACHED TABLES FOR CAPACITIES FOR 2 AND 3 ROWS OF NAILS. IF MORE THAN 3 ARE REQUIRED, CONSULT OUR ENGINEERING OFFICE FOR ASSISTANCE.
2. MINIMUM EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS IS 6d; NAILS MAY NOT BE WITHIN EDGE LINE.
3. MINIMUM SPACING OF NAILS IN A ROW IS 12d.
4. MINIMUM END DISTANCE IS 15d; IN ADDITION TO NOTE #2, NAILS MAY NOT BE WITHIN END DISTANCES FROM END OF THE BOARD.
5. BLOCK & BOTTOM CHORD SHALL BE THE SAME SIZE, GRADE, AND SPECIES.
6. WHEN TWO BLOCKS ARE USED, BLOCKS SHALL BE INSTALLED ON OPPOSITES SIDES OF TRUSS.

EAGLE METAL

BEARING ENHANCERS
(TYPICAL EXTERIOR BEARING)

REV: 2.1
ENG: MDV
CAD: RC
DATE: 08/03/12

DRAWING NUMBER
DR-30