```
STRUCTURAL GENERAL NOTES - APPLICABLE TO ALL CONSTRUCTION UNLESS OTHERWISE NOTED ON THE PLANS
   A. DESIGN SCOPE BY PSE CONSULTING ENGINEERS. INC. (PSE):
                                                                                                                                                                     G. SHIPPING / CARGO CONTAINER SPECIFICATION:
        1. Design Shown on drawings by PSE is for the following items.
                                                                                                                                                                         1. The shipping / cargo container(s) shall be:
           a. Foundation and framing.
                                                                                                                                                                             a) Undamaged (Free of rust, dents, cracks, et cetera that affect the structural integrity of the container).
       2. Design Shown on PSE drawings does not include: finishes, architectural items, windows, doors, moisture barriers, water proofing, mechanical units,
                                                                                                                                                                              b) Made from steel.
           plumbing, or electrical items.
                                                                                                                                                                              c) Design and tested according to the International Organization for Standardization (ISO) specifications.
    B. GENERAL REQUIREMENT:
                                                                                                                                                                              d) Certified for compliance to the Rules for Certification of Cargo Containers and the International Convention for Safe Containers (CSC) for use
        1. Furnish all labor, materials, and equipment necessary to complete the work shown or inferred by these drawings.
                                                                                                                                                                              e) Container shall have a CSC safety approval placard (CSC Plate) and CITA logo prior to any modification.
       2. Where construction details are not shown or noted for any part of the work, such details shall be the same as for similar work shown on the
                                                                                                                                                                              f) Walls and roof are continuously welded around its entire periphery and is itself made from sheets of corrugated 14ga. Cor-Ten steel also
        3. Notes and details on the drawings take precedence over the general notes and typical details in case of conflict.
       4. Provide manufacturer's approved product evaluation reports (ICBO reports) and a list of all proposed substitutions to the Engineer for review
                                                                                                                                                                           2. Prior to construction/modification, Client / Owner / Contractor shall:
           and written approval before fabrication.
                                                                                                                                                                             a) Provide pictures of high/good quality to PSE showing the following of each container:
        5. Pipes, ducts, sleeves, chases, etc. shall not be placed in slabs, beams, or walls unless specifically shown or noted nor shall any structural
           member be cut for pipe, ducts, etc., unless specifically shown. Obtain prior written approval for installation of any additional holes, ducts, etc.
        6. Locate and protect underground or concealed conduit, plumbing or other utilities where new work is being performed.
        7. The contract drawings and specifications represent the finished structure and do not indicate methods, procedures or sequence of
           construction. The contractor shall take necessary precautions to maintain and insure the integrity of the new and any existing structures
           during construction. The design stresses shall not be exceeded during construction based on the age of each element. Neither the owner nor
           Architect/Engineer will enforce safety measure regulations. Contractor shall design, construct and maintain all safety devices, including shoring
           and bracing for the new and any existing structures and shall be solely responsible for conforming to all local, state and federal safety and
                                                                                                                                                                              b) Visually inspect all existing welds for consistency and undamaged.
           health standards, laws and regulations. Observation visits to the site by the engineer shall not include inspection of the above items.
                                                                                                                                                                              c) Confirm existing plywood floor sheathing is:
        8. Obtain prior written approval for any changes to the drawings.
       9. The contractor shall review and compare the structural drawings with all other Construction Documents, such as Architectural, Mechanical and Intertect.
           Electrical drawings, specifications, etc. Do not scale drawings. The contractor shall verify dimensions, elevations and all information. Report, in
           writing, any inconsistencies, errors, or omissions to the Architect/Engineer of record before proceeding with the work.
        10. All existing constructions shown are schematic only. Contractor is responsible to verify actual conditions and allow for them in his bid. Notify
            the Architect/Engineer, in writing, in case of any discrepancy between actual conditions and what is shown on the structural drawings before
                                                                                                                                                                             d) If possible, provide manufacturer drawings of container to PSE.
             proceeding with the work.
                                                                                                                                                                          3. Dimensions provided may be the nominal dimensions of the container. Contractor/fabricator, owner, and/or architect to verify actual dimension
         11. See Architectural, Mechanical, Electrical and other drawings for embedded items.
                                                                                                                                                     07-03-2024
                                                                                                                                                                             before construction.
        12. Camber shall be provided for all members with 30 feet or more of span. Check beam table and contact the Structural Engineer for the
                                                                                                                                                    FBNR ATI-0021
            amount of camber.
        13. Shop drawings:
                                                                                                                                                                      H. ABBREVIATIONS:
            a) Shop drawings shall be submitted in the form of two copies.
            b) Prior to submittals, the general contractor shall review all submittals for conformance with the Construction Documents and shall stamp
                submittals as being "Reviewed for Conformance".
             c) Any detail on the shop drawing that deviates from the Construction Documents shall be marked with the note "This is a change"
             d) Shop drawing submittals processed by the Structural Engineer are not Change Orders.
                                                                                                                                                                            APA
            e) Shop drawings shall be submitted to the Architect/Engineer prior to fabrication and construction regarding all structural items including:
                 -Concrete and masonry reinforcement, drawings shall conform to ACI 315 and ACI 318.
                 -Structural steel, drawings to conform to AISC.
                 -Glued-Laminated members, drawings to conform to AITC.
                 -Prefabricated wood joists and trusses, drawings to conform to ICBO product evaluation report.
                 -Wood trusses, drawings to conform to UBC.
             f) Shop drawings or calculations submitted for review that require re-submittal for re-review, as determined by the Structural Engineer, shall
               be billed hourly to the general contractor. Re-review will not proceed without written approval from the general contractor for additional
        14. Submit seismic anchorage calculations stamped by a licensed Professional Engineer for all equipment and components weighing more than
                                                                                                                                                                            CONN
        15. Submit structural drawings signed and sealed by a professional Engineer licensed in the State where the project is located for any structural
            member needed for this project that is not designed by P.S.E.
                                                                                                                                                                            DBL
        17. Any substitutions for structural members, hardware or details shall be reviewed by the Architect and Structural Engineer. Such review will be
             billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed.
        18. All communication shall be in writing. No verbal communications, decisions, instructions or approvals shall be valid.
                                                                                                                                                                            DWL
    C. CODE AND LOADS:
          1. All design, material, and construction work for this project shall conform to the Colorado State Building Codes
               based on the 2021 International Building Code (IBC).
         2. Design parameters.
                                                                                                                                                                            EMBED EMBEDMENT
          a. Floor Live Load = 40 psf.
                                                       b. Floor Dead Load = 15 psf.
                                                                                                                                                                            EOR ENGINEER OF RECORD
          c. Roof Live Load = n/a psf.
                                                       d. Roof Dead load = 15 psf.
          e. Ground Snow Load, Pg = 76.8psf.
                                                       f. Flat Roof snow load = 53.8 psf.
          g. Snow Exposure Factor, Ce = 1.0
                                                       h. Snow Load Importance Factor, ls = 1.0
           i. Thermal Factor, Ct = 1.0
                                                       j. Ultimate Wind Speed (3 second gust) = 105 mph
          k. Wind Importance Factor, lw = 1.0

 Wind Exposure = C

          m. Internal Pressure Coefficient = 0.85
                                                       n. Components and Cladding studs = 38 psf
                                                      p. Site Class = D
          o. Seismic Importance Factor, le = 1.0
          q. Ss = 0.582
                                                       r. S1 = 0.102
          s. Sms = 0.777
                                                       t. Sm1 = 0.244
         u. Sds = 0.518
                                                       v. Sd1 = 0.163
                                                       x. Basic Seismic Force Resisting System = Metal Sheathed Shipping Container
          w. Seismic Design Category = D
          y. Design Base Shear = 0.259 * W
                                                       z. Approximate Fundamental Period, T = 0.104
          aa. Response Modification Factor, R = 2.0 bb. Analysis Procedure Used = Equivalent Lateral Force Procedure
    D. INSPECTION:
         The owner shall employ one or more qualified Inspectors to provide inspections during construction in according with section 1701 of the above code. The
         Inspector shall be certified by the building official to perform the type of inspection specified. Inspection shall be provided for:
             b. Reinforcement placement, prior to closing the forms and delivery of concrete.
               . Concrete placement.
              d. Bolts installed in concrete and masonry, prior to and during the placement of concrete around bolts.
             e. Structural Steel.
             f. Field welding.
             a. High-strength bolting.
              h. During preparation and taking of test specimens.
                See other sections of these notes for more required inspections.
         Note: All discrepancies shall be brought to the immediate attention of the contractor for correction; then if not corrected, to the building official and
                to the Engineer in writing. The inspector shall furnish an inspection report to the building official and to the Engineer/Architect of Record.
    E. TESTING:
        The owner/contractor shall retain an independent testing laboratory to test the quality of:
             a. Soil or fill material supporting footings and slab-on-grade.
             c. Mortar shall be tested in accordance with UBC standard 21-16.
             d. Grout shall be tested in accordance with UBC standard 21-18.
             e. All other material used in this project as required by the Engineer.
             f. A copy of test results shall be sent to the Engineer of Record.
    F. STRUCTURAL STEEL:
        1. Unless noted otherwise on plans, all structural steel shall be per Table 2—3 of AISC Manual of Steel Construction, Thirteenth Edition, as shown below:
                                            On-Site Fabrication
                                                                       Steel, Typ
                                                                                           Yeild Strength
                                            W-shape
                                                                     A 992
                                                                     A 36
                                            Angles
                                            Rectangular Tube, HSS A 500, Gr.C
                                            Round Tube, HSS
                                                                     A 500, Gr.C
                                                                      A53, Gr.B
                                                                     A 325
                                                                                             120/105
                                                                     A 570 Gr. 33
                                                                   A 607 Gr. 5
                                            Container/Module Tube Corten/A242
Container/Module Channel Corten/A242
                                            Container/Module Panel Corten/A242
          2. Fabrication and erection shall be in accordance with the American Institute of Steel Construction (AISC).
         3. All beam connections shall be bolted or shop welded as detailed on the drawing or designed by fabricator per AISC Manual of Steel Construction allowable
           Stress Design, "Framed Beam Connections." Bolts shall be 3/4 inch. diameter ASTM A325, load indicator bolts. All bolts shall be tightened to the
            minimum tension specified in the specification for structural joints using A325 or A490 bolts.
         4. All welding shall conform to the current American Welding Society (AWS) Specifications and be performed by certified welders.
         5. Column anchor bolts shall have minimum yield strength of 36 KSI.
         6. Metal deck shall be the type as indicated on the drawings. The deck shall be welded to the supporting members per manufacturer's recommendations or as
             indicated on the drawings whichever is more restricting or stringent.
         7. All openings in metal deck to have 4" X 4" X 1/4" angle frames set between joists.
         8. All structural steel and bar joists shall have one shop coat of rust inhibitor primer paint conforming to specification. Field touch up all unpainted areas
            and weld areas.
         9. Grout for base plates shall be nonmetallic, non-shrinkage cementitious grout having a minimum 3-days compressive strength of 4000 PSI.
         10. Reference specifications for additional requirements.
```

38600 Main street, Milner CO 80487

as shipping containers by the American Bureau of Shipping (ABS) or other approved Certified Inspection and Testing Agency (CITA).

— Not damaged, cracked, deformed, delaminated, or showing any other signs that structural integrity has been compromised

— If existing plywood floor sheathing needs to be replaced, inform PSE prior to replacing with equivalent pressure/preservative treated OSB

LL LIVE LOAD

MATL MATERIAL

MAX MAXIMUM

MACHINE BOLT

MFR MANUFACTURER

MINIMUM

METAL

NUMBER

NEAR SIDE

ON CENTER

NOT TO SCALE

OUTSIDE DIAMETER

OH OPPOSITE HAND
OSB ORIENTED STRAND BOARD

SPECIALTY CODE

OTOB OUT TO OUT OF BEARING

PRESSURE TREATED

PLATE WASHER

REFERENCE

REN ROOF EDGE NAILING

REINF REINFORCEMENT

ON SITE VERIFY

PLATE

PSE, INC.

OFDSC OREGON ONE & TWO FAMILY

OREGON STRUCTURAL

DWELLING SPECIALTY CODE

POUND PER LINEAR FOOT

RFT RAFTERS

SIM SIMILAR

STD STANDARD STGR STAGGER STANDARD

STIFF STIFFENERS

TOFNAII

TOW TOP OF WALL

TYP TYPICAL

VERT VERTICAL

WD WOOD

W/ WITH

W/O WITHOUT

UBC

NOTES

SHEAR NAIL

SPEC SPECIFICATION

SNOW LOAD

TOP & BOTTOM

TYPICAL DETAILS

TONGUE & GROOVE

UNIFORM BUILDING CODE

UON UNLESS OTHERWISE NOTED

VSH VERTICAL SLOTTED HOLES

WEN WALL EDGE NAILING

WWF WELDED WIRE FABRIC

THICKNESS/THICK

TOP OF BEAM

TOP OF FOOTING

SEP SEPARATION

STRUCTURAL GENERAL

- Free of hazardous materials, liquids, and/or stains, or shall be encapsulated.

or $\frac{1}{4}$ " steel plate with equivalent or better fastening to container floor joists.

EACH SIDE

NAILING

FAR SIDE

RFPORT

GALV GALVANIZED

GRADE

HORIZ HORIZONTAL

HDR HEADER

HGR HANGER

INT INTERIOR

LDGR LEDGER

JOINT

FRAMING ANCHOR

FROST DEPTH

FEN FLOOR EDGE NAILING

FINISHED FLOOR

FIELD/INTERMEDIATE

GLB GLUED LAMINATED BEAM

BUILDING OFFICIALS

COLD-FORMED STEEL

INSIDE DIAMETER

LGST LIGHT GAUGE STEEL,

GENERAL CONTRACTOR

GEOTECHNICAL INVESTIGATION

HSH HORIZONTALLY SLOTTED HOLES PERP PERPENDICULAR

INTERNATIONAL CONFERENCE OF PL

EQ EQUAL

FW FACH WAY

- Fully fastened to container floor joists per original construction.

All four sides

Under framing

- Bottom Rails

Roof

Interior

- CSC Plate

ANCHOR BOLT

ASSOCIATION

ARCHITECTURAL

BOUNDARY NAIL

BOTTOM OF FOOTING

CONSTRUCTION JOINT

OR CONTROL JOINT

CENTER LINE

CONNECTION

CONTINUOUS

DOUBLE

DIMENSION

DRAWING

EXISTING

EACH FACE

ELEVATION

EDGE NAIL

DOWEL

EACH

DEAD LOAD

DITTO (REPEAT)

CALIFORNIA BUILDING CODE FTG FOOTING

AMERICAN PLYWOOD

ADDITIONAL

AI TERNATE

BOTTOM

BLOCKING

continuously welded together. This steel, also used for the side and end walls has a minimum yeild strength of 50ksi and tensile of 70ksi.

State of Colorado Division of Housing Jul/23/2024 APPROVED PLAN Subject to field inspection

	2 m-J
SHEET IND	EX:
Sl	GENERAL STRUCTURAL NOTES
S2	CONTAINER FLOOR PLANS
S2.1	CONTAINER DETAILS

Structural details for this project are for illustration only. They are not drawn to scale unless noted otherwise. Contractor must verify all dimensions before fabrication or construction. Do not scale drawing

PSE Consulting Engineers, Inc.

www.structure1.com Klamath Falls Office 250 Main Klamath Falls, Oregon Phone: (541) 850-6300 Fax: (541) 850-6233 info@structure1.com

Medford Office 836 Mason Way Medford OR. 97501 Phone: (541) 858-8500 Fax: (541) 776-4663 infomd@structure1.com

lcensed in 48 States!

onstruction Types: ht Gauge Steel, Straw Bal nboo, Log, Timber/Wood, ructural Insulated Panels/SI asonry, Steel, Concrete, Modular Homes/Factory Built ousing (FBH), ICF, Shippin ntainers, and many more! mmercial or Residential. Green/Sustainable!

Project:

AquaWorks DBO, Inc. Shipping Container

38600 Main St. Milner CO 80487

Owner / Client: |AquaWorks DBO Inc.

5-22-202

Expires 10/31/2025

DRAWN BY: M.R.

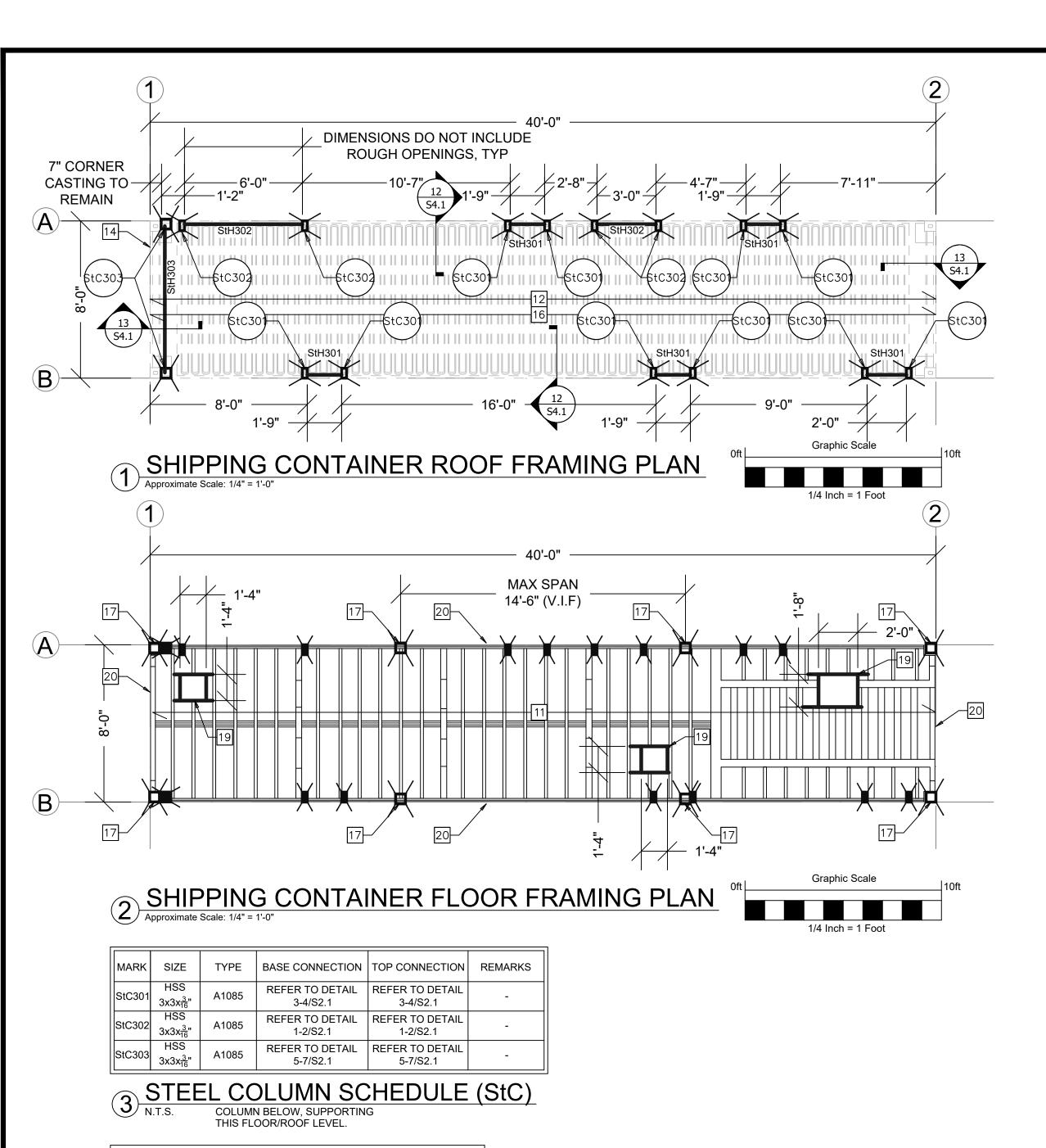
DS. BY: M.R.D CHK BY: N.T. DATE: 2-02-2024

GENERAL NOTES

PAGE NO:

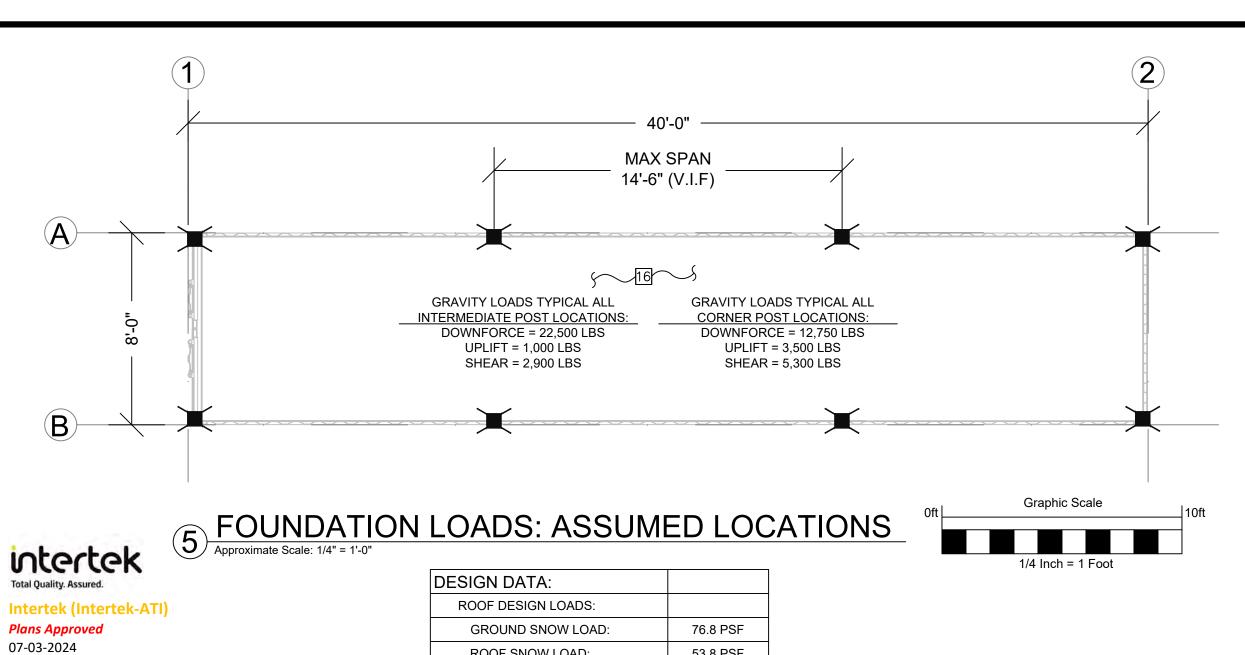
PROJECT #:

AQUAWORKS DBC INC., 224-2002



MARK	MAX OPENING	COMBINATION	FRAME SIZE	REMARKS
StH301	UP TO 2'-0"	A1085	HSS 3x3x 3 "	REFER TO DETAIL 3-4/S2.1
StH302	UP TO 6'-0"	A1085	HSS 3x3x ³ / ₁₆ "	REFER TO DETAIL 1-2/S2.1
StH303	UP TO 8'-0"	A1085	HSS 3x3x ³ / ₁₆ "	REFER TO DETAIL 5-7/S2.1
	StH301 StH302	OPENING StH301	MARK OPENING COMBINATION StH301 UP TO 2'-0" A1085 StH302 UP TO 6'-0" A1085 StH303 UP TO 41085	MARK OPENING COMBINATION FRAME SIZE StH301 UP TO 2'-0" A1085 HSS 3x3x ³ / ₁₆ " StH302 UP TO 6'-0" A1085 HSS 3x3x ³ / ₁₆ " StH303 UP TO A1085 HSS 3x3x ³ / ₁₆ "

4 STEEL HEADER SCHEDULE (StH)



ROOF SNOW LOAD: 53.8 PSF ROOF LIVE LOAD: n/a 10 PSF ROOF DEAD LOAD: 5 PSF CEILING DEAD LOAD: FLOOR DESIGN LOADS: 15 PSF FLOOR DEAD LOAD: 40 PSF FLOOR LIVE LOAD: MEP ROOM LIVE LOAD OCCUPANCY: WIND DESIGN LOADS: **ULTIMATE (3-SEC GUST)** 105 MPH EXPOSURE CATEGORY: С SEISMIC DESIGN LOADS: 0.582 0.102 SEISMIC DESIGN CATEGORY D 1

FBNR ATI-0021

LEGEND:

DISCONTINUOUS 2x6 DF-L #2 @ 16" O.C. SHEAR WALL AND/OR LOAD BEARING WALL SUPPORTING/BELOW THIS FLOOR/ROOF.

DISCONTINUOUS 2x6 DF-L #2 @ 16" O.C. SHEAR WALL AND/OR LOAD BEARING WALL ABOVE THIS FLOOR/ROOF.

DISCONTINUOUS COLUMN SUPPORTING THIS FLOOR/ROOF.

STUB, SHORT,POST.

INDICATES SHEET NOTES.

INDICATES COLUMN MARK, REFER TO COLUMN SCHEDULE.

NUMERICAL VALUE, 1, 2, 3 ETC.

SHEET NOTES:

REFER TO S1 FOR STRUCTURAL GENERAL NOTES AND TO ROOF DETAIL SHEETS FOR CONSTRUCTION DETAILS.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

PROVIDE MULTIPLE STUDS UNDER BEAMS OR TRUSSES TO MATCH WIDTH OF SUPPORTED MEMBER, TYP. STUDS SHALL BE CONTINUED

ROOF DRAINAGE SHALL BE DIRECTED AWAY FROM FOUNDATION.

IN LOWER FLOORS AND/OR CRAWL SPACE TO FOOTING, TYP.

PROVIDE SOLID BLOCKING UNDER POSTS AND MULTIPLE STUDS TO TRANSFER LOADS TO POSTS/STUDS BELOW.

LAY FLOOR AND ROOF STRUCTURAL PANELS WITH THE LONG DIMENSION AT RIGHT ANGLE TO SUPPORTS AND CONTINUOUS OVER TWO OR MORE SPANS.

LIMIT LIVE LOAD DEFLECTION TO SPAN OVER 480 FOR RAFTERS, JOISTS, BEAMS.

FLOOR SHEATHING SHALL BE 3/4" OR MORE THICK APA T&G PLYWOOD OR OSB WITH 48"/24" SPAN RATING. USE 8d @ 4" O.C. (BN) AT EXTERIOR WALLS AND INTERIOR SHEAR WALLS. 8d @ 6" O.C. (FEN) ALONG PANEL EDGES AND 8d @ 10" O.C. AT INTERMEDIATE SUPPORTS, UON. MINIMUM PENETRATION IS 1 5/8" INTO FRAMING. USE GLUE.

FIELD GLUE FLOORS TO ALL SUPPORTS AND T&G EDGES PER APA, AFG-01. FRAMING SHALL BE FREE OF SURFACE MOISTURE AND DEBRIS PRIOR TO GLUING.

0 IF HEAVY EQUIPMENT (WEIGHING OVER 500LBS) IS PLACED OVER FINISHED FLOOR CONTACT EOR FOR REVIEW PRIOR TO INSTALLATION.

1 EXISTING CONTAINER FLOOR BEAMS/RAILS, JOISTS & FLOORING TO REMAIN UNMODIFIED U.N.O

EXISTING CONTAINER ROOF TO REMAIN UNMODIFIED U.N.O

CENTER FOOTING UNDER POSTS AND WALLS UNLESS OTHERWISE NOTED ON PLANS AND/OR DETAILS.

15 LINE OF SHIPPING CONTAINER ABOVE

MAXIMUM 14'-6" O.C. ALONG BOTTOM RAIL.

6 CONTRACTOR'S CHOICE: 2x4 DF-L #1 CEILING JOIST @ 12" O.C. WITH 2x BLOCKING AT HALFWAY POINT AND WOOD SHIM ABOVE @ 1/3RD POINTS. REFER TO DETAIL 12/S2.1 FOR CONNECTION TO TOP RAIL OR

POINT. REFER TO DETAIL 13/S2.1 FOR CONNECTION TO TOP RAIL 7 CONTRACTOR TO ATTACH SHIPPING CONTAINER TO FOUNDATION BY OTHERS WITH BASE PLATE AS PER 8-9/S2.1 AT CORNERS AND

2x4 DF-L #1 RAFTER @ 12" O.C. WITH 2x BLOCKING AT HALFWAY

REINFORCE BOTTOM RAIL ALONG THIS LINE AS PER DETAIL 11/S2.1

GI CONTRACTOR SHALL VERIFY OPENING IN FLOOR. FLOOR OPENING SHALL BE REINFORCED WITH (2) L4x4x4 A36 ANGLE BETWEEN EXISTING FRAMING MEMBERS AND (2) 4"x4" A36 FLAT PLATE BETWEEN NEW 'L'-ANGLE

FOUNDATION NOT BY PSE. OWNER/CONTRACTOR TO HIRE LOCAL LICENSED ENGINEER OR PSE TO DESIGN FOUNDATION TO SUPPORT SHIPPING CONTAINER. CONTACT PSE FOR ADDITIONAL INFORMATION.

> State of Colorado Division of Housing Jul/23/2024



DRAWN BY: M.R.I DS. BY: M.R.D CHK BY: N.T. DATE: 2-02-2024

CONTAINER

FLOOR PLANS PAGE NO:

PSE Consulting

Engineers, Inc.

www.structure1.com

Klamath Falls Office 250 Main Klamath Falls, Oregon

Phone: (541) 850-6300 Fax: (541) 850-6233

info@structure1.com

Medford Office

836 Mason Way

Medford OR. 97501

Phone: (541) 858-8500

Licensed in

48 States!

t Gauge Steel, Sťraw Bale

ooo, Log, Timber/Wood,

ctural Insulated Panels/SI

sonry, Steel, Concrete,

dular Homes/Factory Built

ısına (FBH), ICF, Shippin

itainers, and many more!

imercial or Residential.

AquaWorks

DBO, Inc.

Shipping

Container

38600 Main St.,

Milner CO 80487

Owner / Client:

AquaWorks DBO

Inc.

5-22-202

Expires 10/31/2025

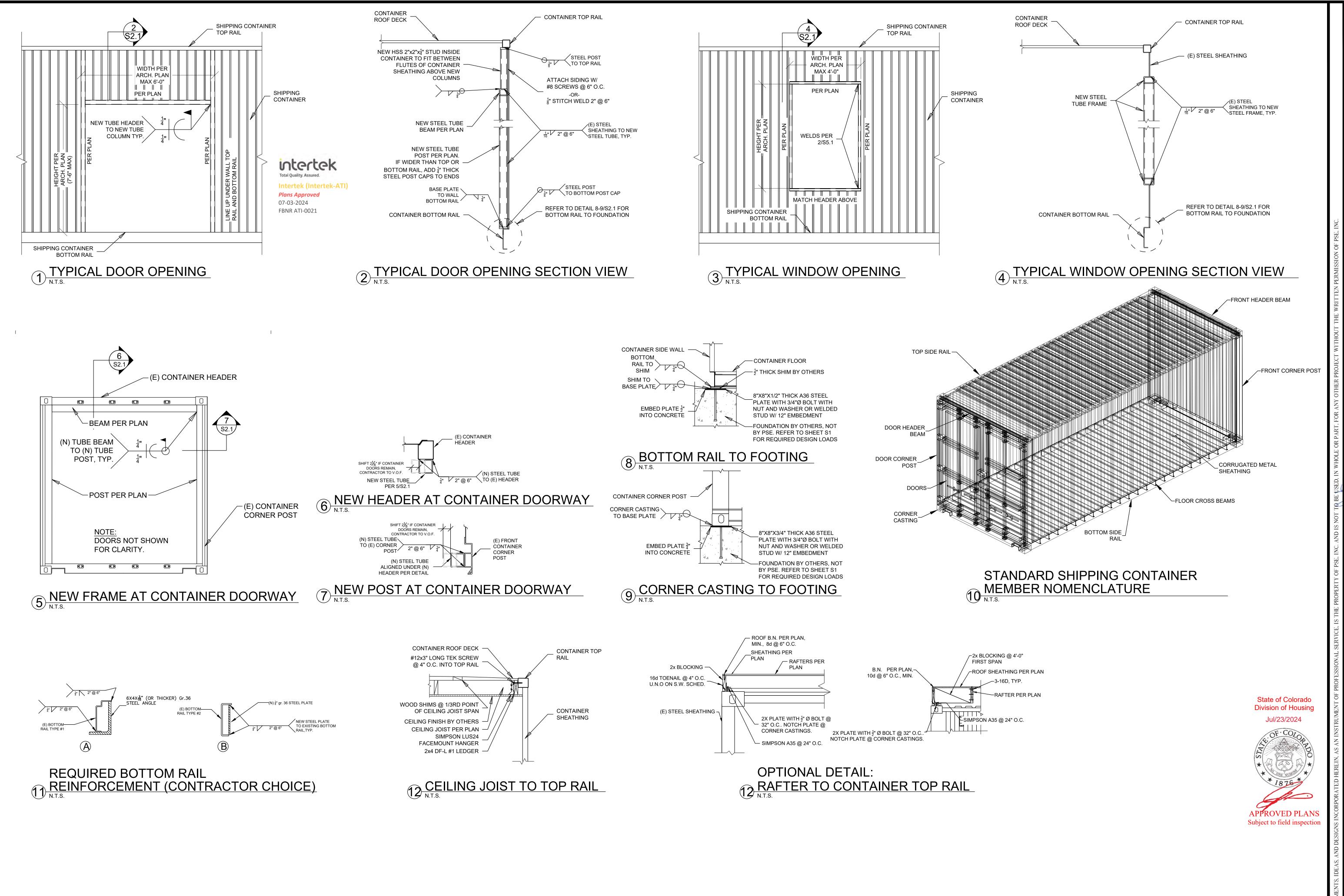
Green/Sustainable!

Project:

Fax: (541) 776-4663 infomd@structure1.com

PROJECT #: AquaWorks DBC Inc., 224-2002

Structural details for this project are for illustration only. They are not drawn to scale unless noted otherwise. Contractor must verify all dimensions before fabrication or construction. Do not scale drawings



PSE Consulting Engineers, Inc.

www.structure1.com Klamath Falls Office 250 Main Klamath Falls, Oregon 97601 Phone: (541) 850-6300 Fax: (541) 850-6233 info@structure1.com

Medford Office 836 Mason Way Medford OR. 97501 Phone: (541) 858-8500 Fax: (541) 776-4663 infomd@structure1.com

Licensed in 48 States!

: Gauge Steel, Straw Bal boo, Log, Timber/Wood. ctural Insulated Panels/SI onry, Steel, Concrete dular Homes/Factory Built sıng (FBH), ICF, Shippir tainers, and many more! nmercial or Residential.

Green/Sustainable!

Project:

AquaWorks DBO, Inc. Shipping Container

38600 Main St. Milner CO 80487

Owner / Client: AquaWorks DBO Inc.

5-22-202

Expires 10/31/2025

DRAWN BY: M.R.

DS. BY: M.R.D CHK BY: N.T.

DATE: 2-02-2024

CONTAINER DETAILS

PAGE NO:

S2.1

PROJECT #: AQUAWORKS DB0 INC., 224-2002

health standards, laws and regulations. Observation visits to the site by the engineer shall not include inspection of the above items. 8. Obtain prior written approval for any changes to the drawings. 9. The contractor shall review and compare the structural drawings with all other Construction Documents, such as Architectural, Mechanical and Electrical drawings, specifications, etc. Do not scale drawings. The contractor shall verify dimensions, elevations and all information. Report, in

writing, any inconsistencies, errors, or omissions to the Architect/Engineer of record before proceeding with the work. 10. All existing constructions shown are schematic only. Contractor is responsible to verify actual conditions and allow for them in his bid. Notify the Architect/Engineer, in writing, in case of any discrepancy between actual conditions and what is shown on the structural drawings before

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C. CODE AND LOADS:

1. All design, material, and construction work for this project shall conform to the 2022 Colorado State Building Code based on the 2021 International Building Code (IBC).

 \tilde{I} . Wind Exposure = C

p. Site Class = D

r. S1 = 0.102

t. Sm1 = 0.244

v. Sd1 = 0.163

2. Design parameters.

a. Floor Live Load = 40 psf. b. Floor Dead Load = 15 psf. d. Roof Dead load = 20 psf. c. Roof Live Load = n/a psf. e. Ground Snow Load, Pg = 76.8psf. f. Flat Roof snow load = 53.8 psf. g. Snow Exposure Factor, Ce = 1.0h. Snow Load Importance Factor, Is = 1.0j. Ultimate Wind Speed (3 second gust) = 105 mph

submittals as being "Reviewed for Conformance".

Thermal Factor, Ct = 1.0k. Wind Importance Factor, lw = 1.0m. Internal Pressure Coefficient = 0.85 o. Seismic Importance Factor, le = 1.0 q. Ss = 0.582

s. Sms = 0.777u. Sds = 0.518w. Seismic Design Category = D y. Design Base Shear = 0.259 * W

x. Basic Seismic Force Resisting System = LIGHT FRAME WALLS WITH SHEAR PANELS z. Approximate Fundamental Period, T = 0.104aa. Response Modification Factor, R = 2.0 bb. Analysis Procedure Used = Equivalent Lateral Force Procedure

D. INSPECTION:

The owner shall employ one or more qualified Inspectors to provide inspections during construction in according with section 1701 of the above code. The Inspector shall be certified by the building official to perform the type of inspection specified. Inspection shall be provided for: a. Foundation excavation

n. Components and Cladding studs = $21.71 \, \text{psf}$

b. Reinforcement placement, prior to closing the forms and delivery of concrete. . Concrete placement.

d. Bolts installed in concrete and masonry, prior to and during the placement of concrete around bolts. e. Structural Steel.

a. High-strength bolting.

n. During preparation and taking of test specimens.

See other sections of these notes for more required inspections.

Note: All discrepancies shall be brought to the immediate attention of the contractor for correction; then if not corrected, to the building official and to the Engineer in writing. The inspector shall furnish an inspection report to the building official and to the Engineer/Architect of Record.

E. TESTING: The owner/contractor shall retain an independent testing laboratory to test the quality of:

a. Soil or fill material supporting footings and slab—on—grade. c. Mortar shall be tested in accordance with UBC standard 21-16. d. Grout shall be tested in accordance with UBC standard 21-18.

e. All other material used in this project as required by the Engineer. f. A copy of test results shall be sent to the Engineer of Record.

1. Unless noted otherwise on plans, all structural steel shall be per Table 2—3 of AISC Manual of Steel Construction, Thirteenth Edition, as shown below:

Yeild Strength A 36 Angles Rectangular Tube, HSS A 500. Gr.C Round Tube, HSS A 500, Gr.C A 325 A 570 Gr. 33 A 607 Gr. 5 Container/Module Tube | Corten/A242 Container/Module Channell Corten/A242 Container/Module Panel Corten/A242

. Fabrication and erection shall be in accordance with the American Institute of Steel Construction (AISC). 3. All beam connections shall be bolted or shop welded as detailed on the drawing or designed by fabricator per AISC Manual of Steel Construction allowable Stress Design, "Framed Beam Connections." Bolts shall be 3/4 inch. diameter ASTM A325, load indicator bolts. All bolts shall be tightened to the minimum tension specified in the specification for structural joints using A325 or A490 bolts.

4. All welding shall conform to the current American Welding Society (AWS) Specifications and be performed by certified welders.

5. Column anchor bolts shall have minimum yield strength of 36 KSI. 6. Metal deck shall be the type as indicated on the drawings. The deck shall be welded to the supporting members per manufacturer's recommendations or as

indicated on the drawings whichever is more restricting or stringent. 7. All openings in metal deck to have 4" X 4" X 1/4" angle frames set between joists.

8. All structural steel and bar joists shall have one shop coat of rust inhibitor primer paint conforming to specification. Field touch up all unpainted areas

9. Grout for base plates shall be nonmetallic, non—shrinkage cementitious grout having a minimum 3—days compressive strength of 4000 PSI.

10. Reference specifications for additional requirements.

b. Standard Specification for Structural Glued-Laminated Timber of Softwood Species, AITC 117; Manufacturing.

c. Design and Standard Specifications for Hardwood Glued-Laminated Timber, AITC 119. In case of conflict, the most stringent requirement shall apply.

Submit certificate by one of the above agencies to the Engineer and the Building Inspector prior to installation. . Glued—Laminated timber shall have wet—use adhesive, ASTM D2559. Lamination shall be 2 inches nominal. Appearance shall be Industrial, AITC 110.

4. Colorless end sealer shall be applied immediately to the ends of all members after fabrication and field trimming. Members shall be individually 5. Pressure treatment shall be provided for all members exposed to weather and not protected by a roof or eave overhang.

6. All cuts, holes, etc. shall be re—coated as recommended by the manufacturer. 7. Glued-Laminated timber shall have the following minimum combination and strength:

a. Beams with simple spans shall have combination 24F-V4 or better. b. Continuous beams shall have combination as shown on plans.

1. Provide a copy of the manufacturer's approved ICC product evaluation reports.

2. Wood joists shall be installed according to the manufacturer recommendations and as shown on drawings. Blocking, web stiffeners and bridging etc. shall be as required by the manufacturer's approved ICC product evaluation reports.

3. All joists, ceiling joists and rafters shall have a minimum of 1-1/2 inches bearing at each end on wood or metal, and not less than 3 inches on masonry or concrete. Use approved joist hanger if bearing is not provided.

4. Install full depth solid blocking or cross bracing at intervals not exceeding 8 feet for all joists and rafters 2x12 inches and deeper.

1. Double full height studs shall be used at both ends of all walls shown on the structural drawings, UON.

2. Studs shall have full bearing on plates and sills. 3. Provide blocking at all ceiling levels.

4. Provide multiple studs under beams or trusses to match width of supported member, typical. TOP PLATES AND OR CHORDS:

1. Top plates or chords shall be continuous over headers UON.

2. Top plates shall be two pieces, same size as studs. Stagger splices 4'-0" minimum. Center splices over studs UON.

1. All wood structural panels shall be stamped with the appropriate grade trademark of the American Plywood Association (APA). 2. Block structural panel with 2X4 inch flat blocking where noted on roof or floor framing plans. Use ply clips at mid—span of unsupported panel edges.

3. Maintain 1/8" air space between structural panels in walls, floors and roofs at ends and at edges or as specified by the manufacturer. 4. Wood structural panels shall be manufactured using exterior glue and shall be not less than 4X8 feet except at boundaries.

H. WOOD CONNECTIONS:

1. It is required that the contractor keep the Simpson catalog and/or Simpson Installation Manual on site at all times to be used during the installation of all typical Simpson connections.

2. All exposed steel timber hardware, fasteners and connectors shall be galvanized.

3. All fasteners installed in contact with preservative—treated wood shall be of hot—dipped zinc—coated galvanized steel, stainless steel, silicon bronze or copper. The coating weights for zinc-coated fasteners shall be in accordance with ASTM A153. 4. Connector Hardware model numbers are those for the Simpson—Strong Tie Company. Size and number of nails, screws or bolts to be the maximum specified by the manufacturer UON.

Nails shall be common wire unless otherwise noted. 6. Machine nailing: The use of machine nailing is subject to continued satisfactory performance. Panel nails shall be driven so that the heads are flush with the surface of the panel and the minimum panel edge distance is 1/2 inch. 7. Bolts: maintain a distance not less then 7 bolt diameters from the end and 4 diameters from the edge of the member. Bore holes $\frac{1}{32}$ to $\frac{1}{16}$ inch

larger than the bolt diameter. All nuts shall be tightened when installed and re—tightened at completion of work or before closing in. Thread projection shall be ½ inch minimum beyond the nut. Use 5/16 inch thick X 3" X 3" washers, typ. 8. Lag screw clearance and lead/pilot holes shall be bored in two stages as follows: The clearance hole for the shank shall have the same diameter as

the shank, and the same depth of penetration as the length of unthreaded shank. The lead hole for the threaded portion shall have diameter equal to 70% of the shank diameter and a length equal to at least the length of the threaded portion. 9. Nailed/screwed or bolted hold-down anchors shall be installed per manufacturer's approved [ICC or ICC] product evaluation report. Install hold-downs

3/4 inch minimum above the plate to allow for tightening anchor bolt. The hold-down shall be installed tight to the hold-down post without fillers or dapping. Do not bend hold-down anchors.

10. Connections shall be as detailed on the drawings. If not shown, minimum connections shall be as follows:

a. Joist or rafter to sill or girder, toe nail....

b. Bridaina to joist, toenail each end.... c. Sill plate to joist or blocking, typical, face nail [SN].... ..16d at 6" o.c. d. Double top plates:

 Lower plate to studs • Top plate to lower plate, face nail.. ..16d @ 12" O.C. • Top plate to lower plate at lap Splice [4'-0" minimum].. ...20—16d minimum UON on drawings. • Top plate to lower plate at intersection.

..4-8d toenails or 2-16d endnail. e. Stud to sill plate.... f. Double studs, face nail. ..16d at 12" o.c. g. Blocking between joists or rafters to top plate, toenail... ..3–8d .16d @ 16" o.c. along each edge. n. Continuous header, two pieces... i. Ceiling joists to plate, toenail... . Continuous header to stud, toenail.. 4-8d k. Ceiling joists, laps over partitions, face nail.. .3-16d I. Ceiling joists to parallel rafters, face nail... ..3–16d

..16d @ 12" o.c. m.Built-up corner studs...8d @ 4" O.C. @ 3/8" from all panel n. 5/8" gyp. Sheathing to studs, sill plates & top plates.... edges and 8" O.C. @ intermediate supports.

o. For floor/roof stick framing construction, structural sheathing could be fastened to structural members using 16 gauge wire staples two inches long. Staples shall have a minimum of $\frac{7}{6}$ diameter crown width. For roof and floor, staple spacing shall be per plan. For shear wall, spacing should be

p. Staples for structural insulated panels, sips shall be per sips notes. q. NOTES: REF: To the above Building Code.

I ARRREVIATIONS.

I. ADDREVI	A HONS:						
AB ADDL ALT APA ARCH B BLKG BN BOF IBC CJ CL CLR CONN CONT DBL DIM DL DO DWG DWL E EA EF EL EMBED	ANCHOR BOLT ADDITIONAL ALTERNATE AMERICAN PLYWOOD ASSOCIATION ARCHITECTURAL BOTTOM BLOCKING BOUNDARY NAIL BOTTOM OF FOOTING INTERNATIONAL BUILDING CODE CONSTRUCTION JOINT OR CONTROL JOINT CENTER LINE CLEAR CONNECTION CONTINUOUS DOUBLE DIMENSION DEAD LOAD	FD FEN FF FN FS FTG GAL V GGR GLB GRR HGR HOSH OLD HSB	EQUAL EACH SIDE EACH WAY FRAMING ANCHOR FROST DEPTH FLOOR EDGE NAILING FINISHED FLOOR FIELD/INTERMEDIATE NAILING FAR SIDE FOOTING GALVANIZED GENERAL CONTRACTOR GEOTECHNICAL INVESTIGATION REPORT GLUED LAMINATED BEAM GRADE HEADER HANGER HORIZONTAL HORIZONTALL HORIZONTALLY SLOTTED HOLES INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS INSIDE DIAMETER INTERIOR JOINT LEDGER LIGHT GAUGE STEEL, COLD—FORMED STEEL	MAX MB MFR MIN. MTL NO. NS NTS OC OD OFDSC OH OSB OSSC OSV OTOB PERP	MAXIMUM MACHINE BOLT MANUFACTURER MINIMUM METAL NUMBER NEAR SIDE NOT TO SCALE ON CENTER OUTSIDE DIAMETER OREGON ONE & TWO FAMILY DWELLING SPECIALTY CODE OPPOSITE HAND ORIENTED STRAND BOARD OREGON STRUCTURAL SPECIALTY CODE ON SITE VERIFY OUT TO OUT OF BEARING PERPENDICULAR	SEP SIM SNLC STD STGR ST TB TG KTOF TOYP UORT TYPC UORT VSD WEN	SPECIFICATION STANDARD STAGGER STIFFENERS TOP TOP & BOTTOM TYPICAL DETAILS TONGUE & GROOVE THICKNESS/THICK TOENAIL TOP OF BEAM TOP OF FOOTING TOP OF WALL TYPICAL UNIFORM BUILDING CODE UNLESS OTHERWISE NOTED VERTICAL

Division of Housing Jul/23/2024

APPROVED PLAN

Subject to field inspection

State of Colorado

SHEET INDEX	/.
S1	GENERAL STRUCTURAL NOTES
S2	TYPICAL FRAMING DETAILS
S3	FOUNDATION REACTIONS & FRAMING PLAN

PSE Consulting Engineers, Inc.

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onstruction Types: ht Gauge Steel, Straw Bal nboo, Log, Timber/Wood, uctural Insulated Panels/SI asonry, Steel, Concrete. Modular Homes/Factory Built ousing (FBH), ICF, Shippina ntainers, and many more! mmercial or Residential. Green/Sustainable!

Project:

AquaWorks DBO, Inc. Screen Building

38600 Main St. Milner, Colorado 80487.

Owner / Client: |AquaWorks DBO Inc.

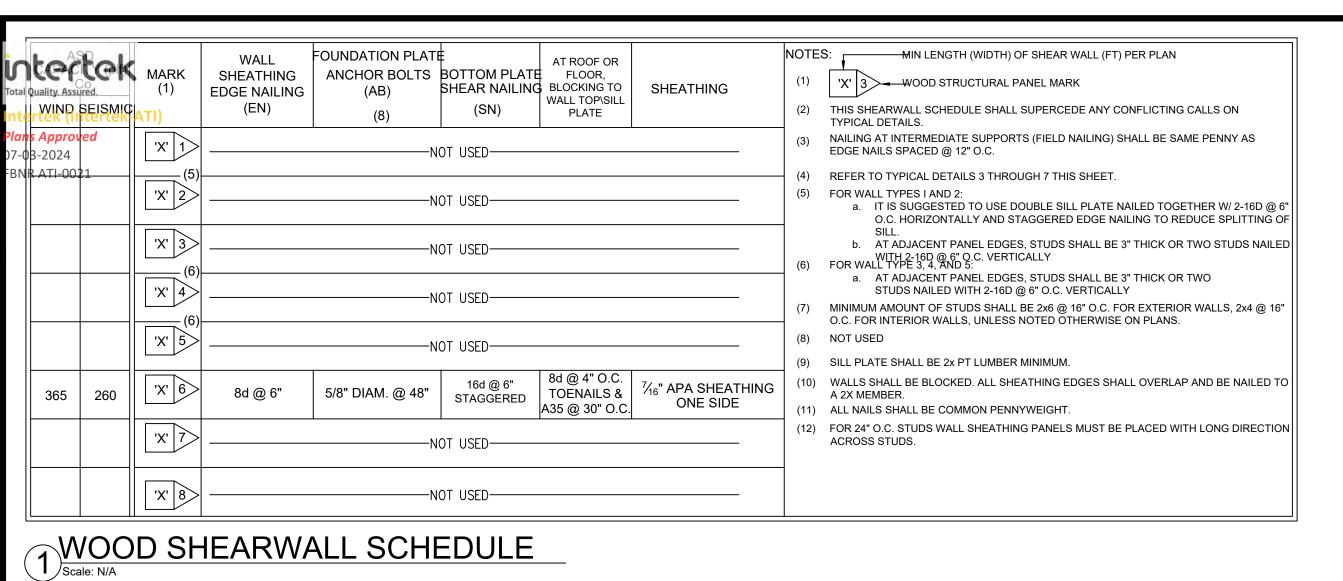
5-22-202 **Expires 10/31/2025**

DRAWN BY: AYPN DS. BY: M.R.D CHK BY: N.T.

DATE: 04-08-202 GENERAL

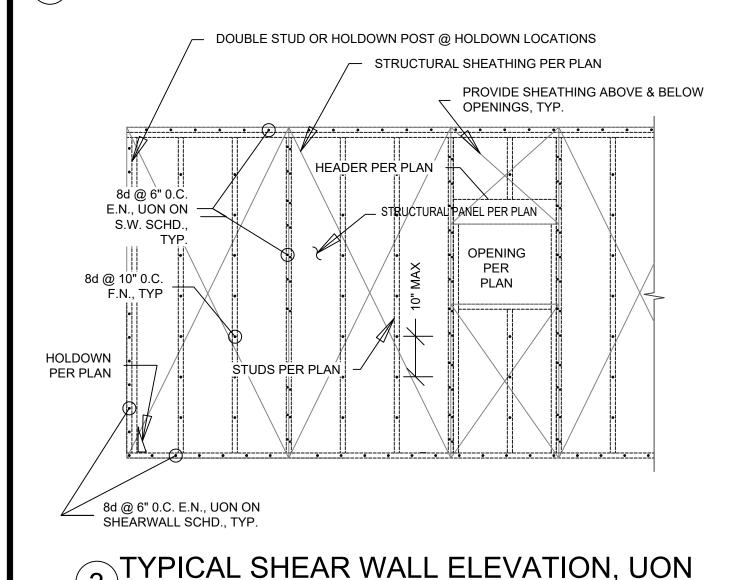
STRUCTURAL NOTES PAGE NO:

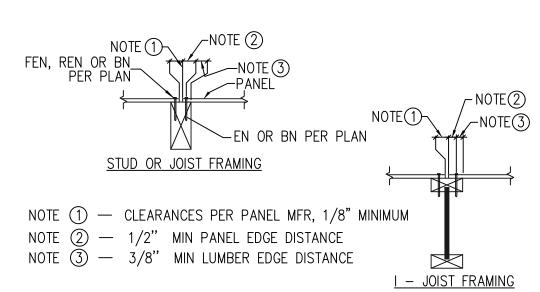
PROJECT #: AQUAWORKS DB0 INC., 224-2002



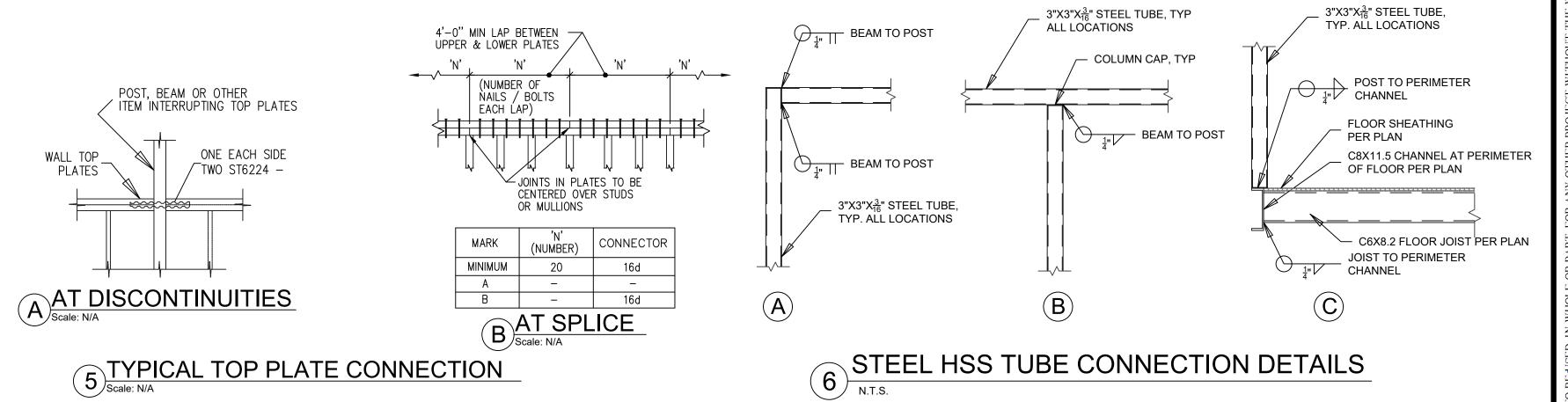
CAPACITY ANCHOR BOLT | EMBED MIN. STEMWALL HOLDOWN HOLDOWN NOTES LENGTH (lbs.) ASD MARK **BRACKET** WIDTH (f'c=2500 psi min) CORNER END EDGE 2 - 2x STUDS 41/4" 1¾" 4,565 HDU4 |HDU4-SDS2.5| SB%x24 OR 1 - 4x POST 1 - 2x STUDS USE MAX. 1,705 N/A N/A CS16 OR N/A N/A N/A NAILING PER CS16 1 - 4x POST MNFR.

NOTE: IT MAY BE NECESSARY TO THICKEN THE FOOTING BELOW SOME ANCHORS TO PROVIDE REQ. EMBED LENGTH AND CLEAR SPACING. 2 HOLDOWN SCHEDULE
Scale: N/A

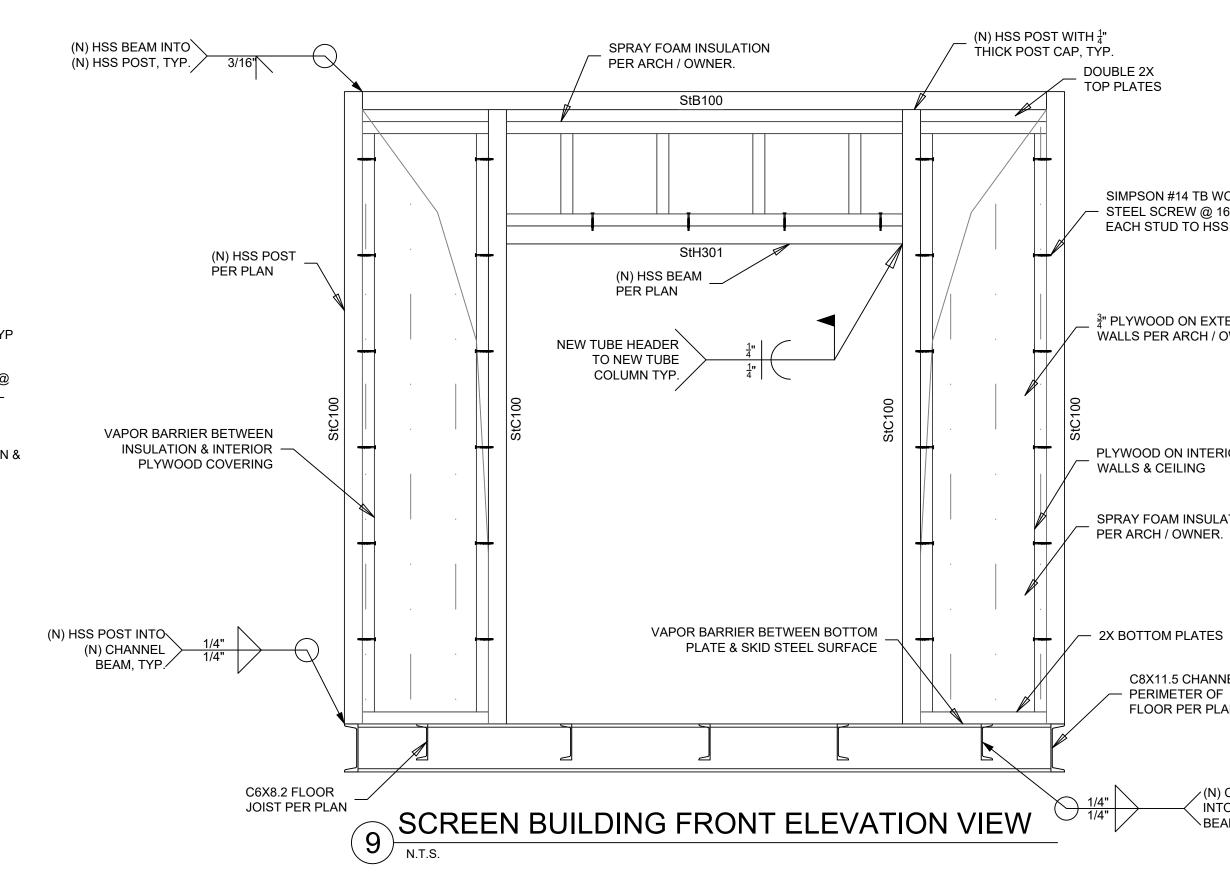


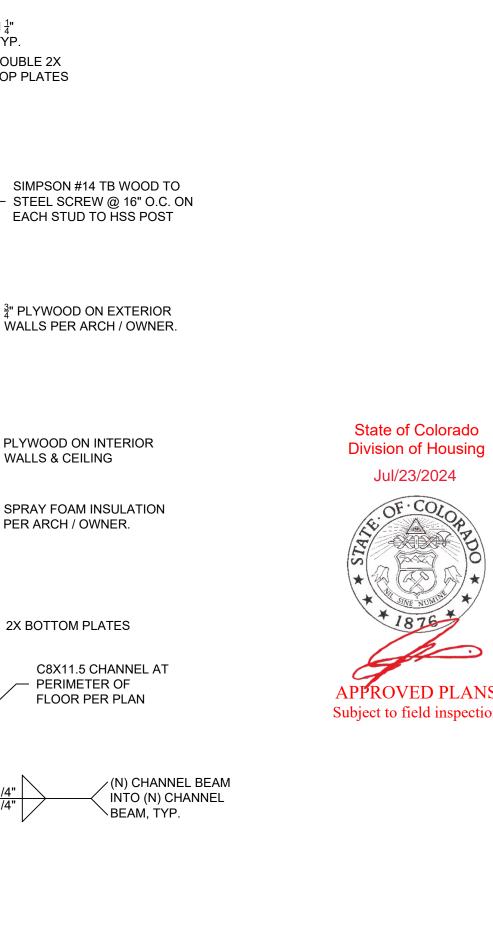


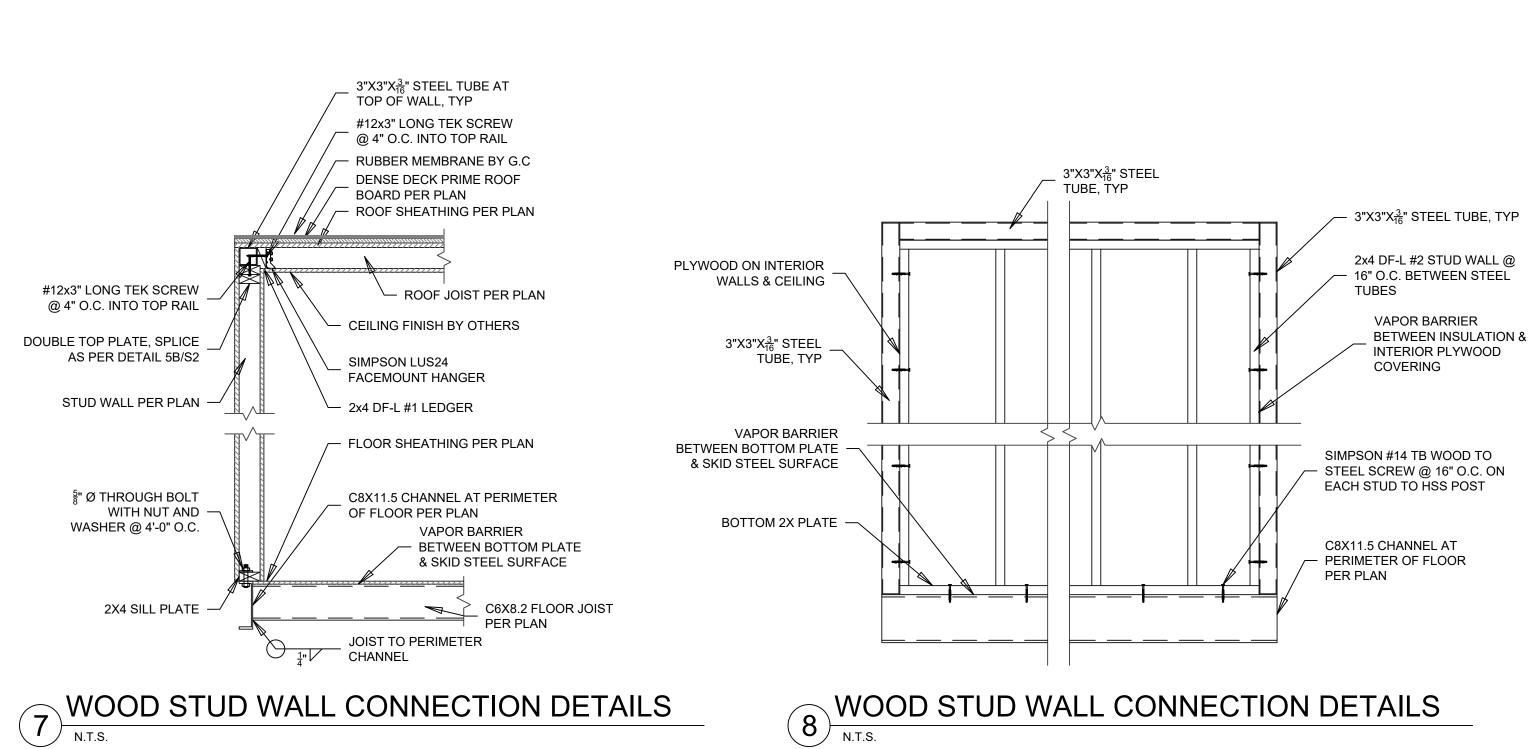
TYPICAL WOOD STRUCTURE PANEL NAILING FOR WALL, FLOOR AND ROOF, UON
Scale: N.T.S.



6 N.T.S.







Structural details for this project are for illustration only. They are not drawn to scale unless noted otherwise. Contractor must verify all dimensions before fabrication or construction. Do not scale drawing

PSE Consulting Engineers, Inc.

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ıt Gauge Steel, Straw Bal boo, Log, Timber/Wood, uctural Insulated Panels/SI sonry, Steel, Concrete, odular Homes/Factory Built using (FBH), ICF, Shippin ntainers, and many more! nmercial or Residential. Green/Sustainable!

Project:

AquaWorks DBO, Inc. Screen Building

38600 Main St., Milner, Colorado 80487.

Owner / Client: AquaWorks DBO Inc.

5-22-202 **Expires 10/31/2025**

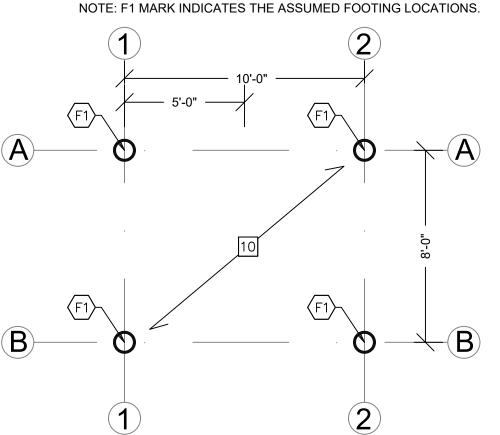
DRAWN BY: AYPN DS. BY: M.R.D

CHK BY: N.T. DATE: 04-08-202

TYPICAL DETAILS PAGE NO:

PROJECT #: AQUAWORKS DB0 Inc., 224-2002

Scale: N.T.S.



NOTE: F1 MARK INDICATES THE ASSUMED FOOTING LOCATIONS. FOR INCLUDING LATERAL LOADS TYPICAL ALL CORNER POST LOCATIONS: DOWNFORCE = 4510 LBS UPLIFT = 996 LBS SHEAR = 1348 LBS

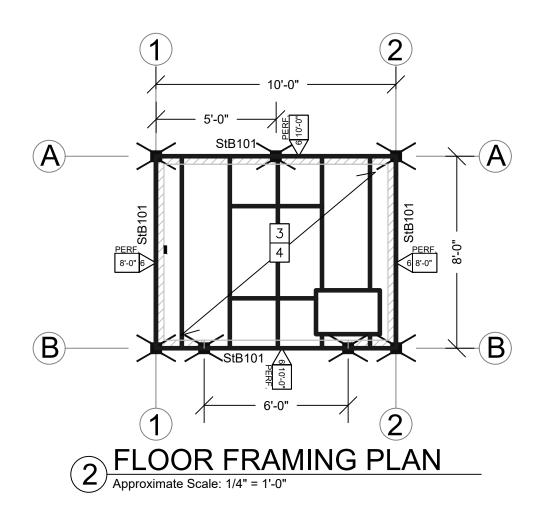
SUMMARY OF REACTIONS AT FOOTING POINTS

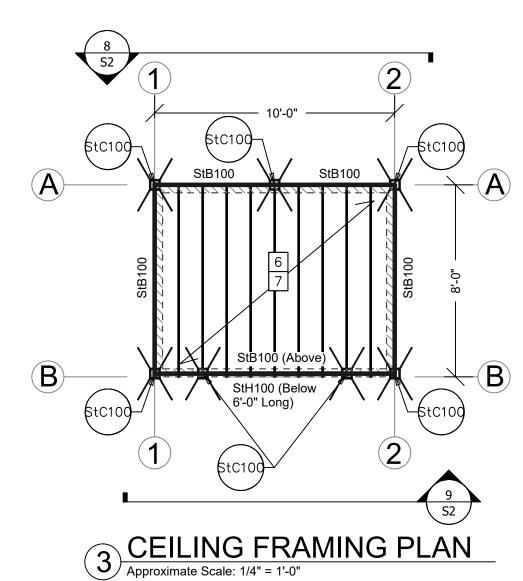
FOR LRFD LOAD COMBINATION

DESIGN DATA:	
ROOF DESIGN LOADS:	
GROUND SNOW LOAD:	76.82 PSF
ROOF SNOW LOAD:	80 PSF
ROOF LIVE LOAD:	20 PSF
ROOF DEAD LOAD:	15 PSF
CEILING DEAD LOAD:	5 PSF
FLOOR DESIGN LOADS:	
FLOOR DEAD LOAD:	15 PSF
FLOOR LIVE LOAD:	40 PSF
LIVE LOAD OCCUPANCY:	RESIDENTIAL
WIND DESIGN LOADS:	
ULTIMATE (3-SEC GUST)	105 MPH
EXPOSURE CATEGORY:	O
SEISMIC DESIGN LOADS:	
Ss	0.582
S1	0.102
SEISMIC DESIGN CATEGORY	С

le 1.0

FOUNDATION LOADS: ASSUMED LOCATIONS





MARK	SIZE	MATERIAL	CAMBER (INCH)	REMARKS
StB100	HSS 3X3X ³ / ₁₆ "	A1085	_	MAIN STRUCTURE BOX FRAME
StB101	C8X11.5	A36	_	BOTTOM DECK FRAME
StB102	C6X8.2	A36	-	BOTTOM DECK FRAME

4 STEEL BEAM SCHEDULE (StB)

MARK	SIZE	TYPE	BASE CONNECTION	TOP CONNECTION	REMARKS
StC100	HSS 3x3x 3 "	A1085	REFER TO DETAIL 6/S2	REFER TO DETAIL 2/S2	-

5 STEEL COLUMN SCHEDULE (StC)

MARK	MAX OPENING	COMBINATION	FRAME SIZE	REMARKS
StH100	UP TO 6'-0"	A1085	HSS 3x3x 3 "	REFER TO DETAIL 9/S2

6 STEEL HEADER SCHEDULE (StH)

LEGEND:

DISCONTINUOUS 2X4 DF-L #2 @ 16" O.C. SHEAR WALL AND/OR LOAD BEARING WALL SUPPORTING/BELOW THIS FLOOR/ROOF.

DISCONTINUOUS 2X4 DF-L #2 @ 16" O.C. SHEAR WALL AND/OR LOAD BEARING WALL SUPPORTING ABOVE THIS FLOOR/ROOF.

COLUMN BELOW AND COLUMN ABOVE THIS FLOOR.

COLUMN SUPPORTING NEXT FLOOR/ROOF UP.

DISCONTINUOUS COLUMN SUPPORTING THIS FLOOR/ROOF.

VERTICAL WINDOW FRAMING STUB POST, NOT FULL INDICATES HOLD-DOWN MARK, REFER TO HOLD

-DOWN SCHEDULE. INDICATES SHEET NOTES.

INDICATES COLUMN MARK, REFER TO COLUMN SCHEDULE.

NUMERICAL VALUE, 1, 2, 3 ETC.

SHEET NOTES:

REFER TO S1 FOR STRUCTURAL GENERAL NOTES AND TO FLOOR DETAIL SHEETS FOR CONSTRUCTION DETAILS. TYPICAL DETAILS ARE GENERALLY NOT CUT ON PLANS BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS. WHERE TYPICAL DETAILS ARE CUT ON PLAN, THE INTENT IS TO ILLUSTRATE THE TYPE OF CONDITION AT WHICH THAT DETAIL IS INTENDED TO APPLY RATHER THAN EVERY OCCURRENCE OF THAT DETAIL.

VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

CHECKERED STEEL FLOOR PLATE OR EQUIVALENT AS PER ARCH DRAWINGS / OWNER. (OR) IF THE FLOOR SHEATHING IS DONE WITH PLYWOOD SHEATHING MEANS, THE FLOOR SHEATHING SHALL BE 3/4" OR MORE THICK APA T&G PLYWOOD OR OSB WITH 48"/24" SPAN RATING. USE 8d @ 4" O.C. (BN) AT EXTERIOR WALLS AND INTERIOR SHEAR WALLS. 8d @ 6" O.C. (FEN) ALONG PANEL EDGES AND 8d @ 10" O.C. AT INTERMEDIATE SUPPORTS, UON. MINIMUM PENETRATION IS 1 5/8" INTO FRAMING. USE GLUE.

ALL THE INTERIOR / INNER SIDE BEAMS ARE StB102 - C6X8.2. & FOR OUTER PERIMETER MAIN BEAM WILL BE StB101 - C8X11.5, AS PER ARCH DRAWING & DIMENSIONS, SEE 1-3/S3 FOR MORE DETAILS.

ROOF DRAINAGE SHALL BE DIRECTED AWAY FROM FOUNDATION.

2x4 DF-L #1 CEILING JOIST @ 12" O.C. WITH 2x BLOCKING AT HALFWAY POINT AND WOOD SHIM ABOVE @ 1/3RD POINTS. REFER TO DETAIL 7/S2.

RUBBER MOISTURE BARRIER ABOVE $\frac{1}{2}$ " DENSEDECK ROOF BOARD WITH (20) FASTENERS PER 4'X8' BOARD INTO ROOF SHEATHING BELOW. SEE DENSDECK TECHNICAL GUIDE FOR ADDITIONAL INFORMATION. ROOF SHEATHING SHALL BE 5/8" THICK APA PLYWOOD WITH 24"/16" SPAN RATING. USE 8d @ 4" O.C. (BN) AT EXTERIOR WALLS AND INTERIOR SHEAR WALLS. 8d @ 6" O.C. (REN) AT PANEL EDGES AND 8d @ 10" O.C. AT INTERMEDIATE SUPPORTS, UON. MINIMUM PENETRATION IS 1 5/8" INTO FRAMING.

8 ALL EXTERIOR WALLS SHALL BE TYPE 6 PER SHEAR WALL PER SHEAR WALL SCHEDULE UNLESS OTHERWISE NOTED ON PLANS.

9 IF HEAVY EQUIPMENT (WEIGHING OVER 500LBS) IS PLACED OVER FINISHED FLOOR CONTACT EOR FOR REVIEW PRIOR TO INSTALLATION.

FOUNDATION, BASE PLATE, ANCHOR BOLT DESIGN BY OTHERS. FOR THE FOUNDATION BASE NODE REACTIONS REFER SHEET 1/S3. CONTACT PSE FOR ADDITIONAL INFORMATION.

PSE Consulting Engineers, Inc.

> www.structure1.com Klamath Falls Office 250 Main Klamath Falls, Oregon 97601 Phone: (541) 850-6300 Fax: (541) 850-6233 info@structure1.com

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Project:

AquaWorks DBO, Inc. Screen Building

38600 Main St., Milner, Colorado -80487.

Owner / Client: AquaWorks DBO, Inc.



Expires 10/31/2025

DRAWN BY: AYPN

DS. BY: M.R.D CHK BY: N.T. DATE: 04-08-2024

FRAMING PLANS & **DETAILS** PAGE NO:

PROJECT #: AquaWorks DBC INC., 224-2002

Structural details for this project are for illustration only. They are not drawn to scale unless noted otherwise. Contractor must verify all dimensions before fabrication or construction. Do not scale drawings

State of Colorado

Division of Housing

Jul/23/2024

Subject to field inspection