STRUCTURAL GENERAL NOTES		
1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND JOB SITE CONDITIONS BEFORE COMMENCING WORK AND SHALL REPORT ANY DISCREPANCIES TO ECLIPSE ENGINEERING, HENCEFORTH REFERRED TO AS THE ENGINEER VII.	NOOD FRAMING 1. MEMBERS	7. GENERAL:
 USE WRITTEN DIMENSIONS. DO NOT USE SCALED DIMENSIONS. WHERE NO DIMENSION IS PROVIDED, CONSULT THE ARCHITECT OR ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH THE WORK. DETAILS IN THE PRANNESS PREFACED WITH THE THE EXPLICATION MAX NOT NECESSARILY. RE REFERENCED ON THE REANS. RUT. 	A. SAWN LUMBER: NO. 2 DOUGLAS FIR/LARCH, WWPA GRADING RULES 1. ALL LUMBER SHALL BE KILN DRIED WITH A MOISTURE CONTENT LESS THAN 19%.	A. FOR CONNECTIONS FOR WOOL B. ALL EXTERIOR WOOD SHALL E MANUFACTURERS RECOMMENT
3. DETAILS IN THE DRAWINGS PREFACED WITH THE TITLE TYPICAL MAY NOT NECESSARILY BE REFERENCED ON THE PLANS, BUT SHALL STILL APPLY AS SHOWN OR DESCRIBED IN THE DETAILS. WHERE NO DETAIL IS REFERENCED, IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO CHOOSE THE RELEVANT TYPICAL DETAIL FROM THOSE PROVIDED.	2. SILLS AND PLATES IN CONTACT WITH MASONRY OR CONCRETE, AND WITHIN 6" OF GRADE, SHALL BE PRESSURE-TREATED DOUGLAS FIR-LARCH. MUD SILL SHALL BE 2X MINIMUM THICKNESS OF THE SAME OR GREATER WIDTH AS THE STUDS ABOVE.	C. ALL NON-BEARING WALLS BEI DEFLECTION.
4. THE DESIGN, ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, SHORING OF EXISTING BUILDING ELEMENTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF	 3. WALL FRAMING SHALL BE 2X6 STUDS @ 16" O.C. UNLESS OTHERWISE NOTED. PROVIDE DOUBLE 2X6 TOP PLATE WITH MINIMUM 48" LAP SPLICE WITH (3) 10d COMMON NAILS MINIMUM, STAGGERED, UNLESS OTHERWISE NOTED. 4. JOISTS AND RAFTERS SHALL HAVE A 1 1/2" MINIMUM BEARING OR SHALL BE SEATED IN METAL HANGERS. 	VIII. <u>TIMBER FRAMING</u> 1. QUALITY ASSURANCE
THE STRUCTURE PRIOR TO WHEN THE ERECTION OF THE FRAMING AND OF THE LATERAL-LOAD-RESISTING SYSTEM IS COMPLETE. 5. THE ENGINEER HOLDS NO LIABILITY FOR UNAUTHORIZED CHANGES TO THE CONSTRUCTION DOCUMENTS MADE BY THE OWNER,	 JOISTS AND RAFTERS SHALL HAVE A T 1/2 MINIMUM BEARING OR SHALL BE SEATED IN METAL HANGERS. BLOCKING SHALL BE SOLID 2X MATERIAL WITH THE SAME DEPTH AS THE JOIST OR RAFTER AND SHALL BE TIGHTLY FITTED BETWEEN JOISTS OR RAFTERS. 	A. WORK SHALL BE IN CONFORM B. ALL TIMBERS SHALL BE GRAD
CONTRACTOR, BUILDING OFFICIAL, OR OTHER INVOLVED PARTY. 6. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROVIDING A SAFE PLACE TO WORK AND FOR MEETING THE REQUIREMENTS OF ALL APPLICABLE JURISDICTIONS, INCLUDING OSHA. THE CONTRACTOR SHALL EXECUTE THEIR WORK TO ENSURE	6. FASTEN BEAMS, COLUMNS, TRIMMER STUDS, AND KING STUDS COMPOSED OF MULTIPLE 2X MEMBERS WITH TWO ROMS OF 10d NAILS @ 12 ON CENTER THROUGH LENGTH OR HEIGHT, STAGGERED TO PREVENT SPLITTING, BETWEEN EACH PLY.	A TIMBER GRADING TRAINING 2. DELIVERY, STORAGE, AND HANDLING A. KEEP TIMBERS DRY DURING D
THE SAFETY OF PERSONS AND ADJACENT PROPERTY AGAINST DAMAGE BY FALLING DEBRIS AND OTHER HAZARDS ASSOCIATED	 B. TIMBERS: NO 1 DOUGLAS FIR/LARCH, WWPA GRADING RULES. C. GLUED LAMINATED TIMBER: 	B. CUT AND STACK TIMBERS SO / BETWEEN BUNDLES TO ALLOW
	 GLUED LAMINATED TIMBER SHALL BE MANUFACTURED IN ACCORDANCE WITH AMERICAN INSTITUTE OF TIMBER CONSTRUCTION AITC 117 AND AITC A190.1. GLUED LAMINATED TIMBER SHALL BE OF THE FOLLOWING GRADES, UNLESS NOTED OTHERWISE: 	C. PROVIDE REASONABLE PROTI ACCUMULATION OF EXCESS MC
1. BUILDING CODE: 2021 IBC 2. BUILDING HEIGHT: 29'	 a. SINGLE SPAN MEMBERS: COMBINATION 24F-V4 b. MULTI-SPAN & CANTILEVERED MEMBERS: COMBINATION 24F-V3 	3. TIMBER AND EXPOSED SAWN LUMBER A. SPECIES: DOUGLAS FIR B. GRADE: NO. 1
 3. MINIMUM EMBEDMENT OR FROST DEPTH: 48" 4. SOIL BEARING CAPACITY: 2000 PSF 5. MINIMUM FOOTING DEAD LOAD PRESSURE: 600 PSF 	 ALL LAMINATED MEMBERS SHALL BE INDUSTRIAL APPEARANCE GRADE, UNLESS NOTED OTHERWISE. D. ENGINEERED LUMBER: 	C. MOISTURE CONTENT: TIMBERS D. TIMBERS: 8X12 AND SMALLER
	 LAMINATED VENEER LUMBER (LVL): a. MINIMUM DESIGN PROPERTIES FOR 1 3/4"-WIDE MEMBERS: Fb = 2,800 PSI, E = 2,000,000 PSI, Fv = 285 PSI b. MINIMUM DESIGN PROPERTIES FOR 3 1/2" AND WIDER MEMBERS: Fb = 3100 PSI, E = 2,000,000 PSI, Fv = 310 PSI 	WITH THE ENGINEER. E. DRESSING: TIMBER AND EXPO
1. GROUND SNOW LOAD: 78 PSF 2. ROOF SNOW LOAD: 60 PSF	 MINIMUM DESIGN FIGHTESTICK STIZE AND MIDER MEMDERS. TO STICE STICE STICE STICE STICE STICE STICE LVL MEMBERS SHALL NOT BE USED IN EXTERIOR APPLICATIONS OR AGAINST CONCRETE. FASTEN MULTI-PLY LVL BEAMS OR JOISTS TOGETHER WITH TWO ROWS OF 10d NAILS @ 12" ON CENTER THROUGH LENGTH, STAGGERED 	4. TIMBER CONNECTORS A. WOOD PEGS: STRAIGHT GRAIN THEIR LENGTH AND AT LEAST
3. FLOOR LIVE LOAD: 40 PSF 4. DECK LIVE LOAD: 60 PSF	TO PREVENT SPLITTING, BETWEEN EACH PLY. PROVIDE (8) ADDITIONAL 10d NAILS BETWEEN EACH PLY DISTRIBUTED CLOSELY TO THE VICINITY OF CONCENTRATED LOADS ON MEMBERS FROM FLUSH-SUPPORTED BEAMS OR JOISTS.	B. SCREWS: THE FOLLOWING SCR
WIND DESIGN DATA	 2. PARALLEL STRAND LUMBER (PSL): a. MINIMUM DESIGN PROPERTIES: Fb = 2900 PSI, E = 2,000,000 PSI, Fv = 750 PSI b. PSL MEMBERS USED IN EXTERIOR APPLICATIONS. OR AGAINST CONCRETE. SHALL BE APPROVED BY THE MANUFACTURER FOR USE IN 	1. SDWH OR SDWS SCRI TO NARROW FACE OF
1. BASIC WIND SPEED: Vult = 115 MPH. VASD = 89 MPH 2. RISK CATEGORY: II	 D. FSL MEMDERS USED IN EXTERIOR APPLICATIONS, OR AGAINST CONCRETE, SHALL BE APPROVED BY THE MANUFACTURER FOR USE IN THE EXPOSURE CONDITION TO WHICH THEY ARE SUBJECT. 3. LAMINATED STRAND LUMBER (LSL): 	C. THRU-BOLTS AND LAG BOLTS: 1. THRU-BOLTS HOLES
 RISK CALEGORY : II WIND EXPOSURE CATEGORY: C INTERNAL PRESSURE COEFFICIENT, GCDI: +/- 0.18 	a. MINIMUM DESIGN PROPERTIES: Fb = 2325 PSI, E = 1,550,000 PSI, Fv = 310 PSI	BEARING ON WOOD. 2. INSTALL LAG BOLTS
SEISMIC DESIGN DATA (PER ASCE 7-22 PER COUNTY CODE) 1. RISK CATEGORY: II	2. PREFABRICATED WOOD TRUSSES:	STANDARD CUT WAS 5. FABRICATION A. TIMBERS SHALL BE FABRICATI
 SEISMIC IMPORTANCE FACTOR, Ic: 1.0 MAPPED SPECTRAL ACCELERATION, 55: 0.3 g 	 A. PREFABRICATED WOOD TRUSSES SHALL BE CONSIDERED A DEFERRED SUBMITTAL UNLESS OTHERWISE INSTRUCTED BY THE BUILDING OFFICIAL. B. MAXIMUM TRUSS SPACING: 24" O.C. C. FLOOR TRUSS LOADING UNLESS NOTED OTHERWISE ON DRAWINGS: 	DRAWINGS SHALL BE AT THE C B. CONNECTION JOINERY: TIMBER
 MAPPED SPECTRAL ACCELERATION, 51: 0.055 g SITE CLASS: C DESIGN SPECTRAL ACCELERATION. 5ds: 0.2 g 	 TOP CHORD LIVE LOAD = 40 PSF TOP CHORD DEAD LOAD = 35 PSF 	VARIATIONS WHERE THE FACES DETAILING OF JOINERY TO ALL
 DESIGN SPECTRAL ACCELERATION, 5d1: 0.048 g SEISMIC DESIGN CATEGORY: B 	 BOTTOM CHORD LIVE LOAD = 0 PSF BOTTOM CHORD DEAD LOAD = 2 PSF 	
9. BASIC SEISMIC FORCE RESISTING SYSTEM: LIGHT-FRAME WOOD WALLS SHEATHED WITH STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE.	5. REVIEW THE PLANS AND DETAILS FOR SPECIAL LOADS INCLUDING, BUT NOT LIMITED TO, REACTIONS FROM PARAPET WALLS, MECHANICAL UNITS, AND AXIAL LOADS FROM SEISMIC CROSS-TIES AND DRAG STRUTS. D TRUSSES TO BE FABRICATED BY A CERTIFIED MEMBER OF THE TRUSS PLATE INSTITUTE. DESIGN, FABRICATION, AND ERECTION TO CONFORM TO ANSI/TPL 1	
 DESIGN BASE SHEAR: 4.0 KIPS, EACH ORTHOGONAL DIRECTION SEISMIC RESPONSE COEFFICIENT, CS: 0.034, EACH ORTHOGONAL DIRECTION RESPONSE MODIFICATION FACTOR, R: 6.50, EACH ORTHOGONAL DIRECTION 	 D. TRUSSES TO BE FABRICATED BY A CERTIFIED MEMBER OF THE TRUSS PLATE INSTITUTE. DESIGN, FABRICATION, AND ERECTION TO CONFORM TO ANSI/TPI 1. E. TRUSS SUBMITTAL PACKAGE: THE TRUSS SUBMITTAL PACKAGE PROVIDED BY THE TRUSS MANUFACTURER SHALL CONSIST OF EACH INDIVIDUAL TRUSS DESIGN DRAWING, THE TRUSS PLACEMENT DIAGRAM, THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING METHOD, AND DETAILS AND ANY OTHER 	
 RESPONSE MODIFICATION FACTOR, R: 6.50, EACH ORTHOGONAL DIRECTION ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE 	STRUCTURAL DETAILS GERMANE TO THE TRUSSES. 1. TRUSS DESIGN DRAWINGS: DRAWINGS SHALL BE PREPARED AND STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE	
II. <u>SHALLOW FOUNDATIONS</u> 1. THE FOUNDATIONS FOR THE PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT BY NW COLORADO	PROJECT IS LOCATED. DRAWINGS SHALL INCLUDE THE WRITTEN, GRAPHIC, AND PICTORIAL DEPICTION OF EACH INDIVIDUAL TRUSS SHALL BE PROVIDED TO THE BUILDING OFFICIAL FOR APPROVAL PRIOR TO INSTALLATION. REFERENCE THE DEFERRED SUBMITTAL SECTION OF THESE NOTES FOR MORE INFORMATION, TRUSS, DESIGN, DRAWINGS, SHALL, ALGO DE PROVIDED, WITH THE GUIRATING, OF TRUSSES, DELIVERED TO THE	
CONSULTANTS, DATED MARCH 27, 2024. THE CONTRACTOR SHALL REFERENCE THE GEOTECHNICAL REPORT FOR ALL EARHTWORK REQUIREMENTS TO PREPARE THE SITE TO SUPPORT THE BUILDING.	INFORMATION. TRUSS DESIGN DRAWINGS SHALL ALSO BE PROVIDED WITH THE SHIPMENT OF TRUSSES DELIVERED TO THE JOB SITE. TRUSS DESIGN DRAWINGS SHALL INCLUDE, AT A MINIMUM, THE FOLLOWING: a. SLOPE OR DEPTH. SPAN. AND SPACING	
III. COLD WEATHER CONSTRUCTION 1. CONCRETE:	 a. SLOPE OR DEPTH, SPAN, AND SPACING b. LOCATION OF ALL JOINTS AND SUPPORT LOCATIONS c. NUMBER OF PLIES IF GREATER THAN ONE 	
A. THE CONTRACTOR SHALL PRACTICE STANDARD COLD-WEATHER METHODS AS PER ACI 306.B. CALCIUM CHLORIDE SHALL NOT BE USED AS AN ACCELERATING ADMIXTURE.	 REQUIRED BEARING WIDTHS DESIGN LOADS AS APPLICABLE, INCLUDING: 	
 CONCRETE DELIVERED TO THE SITE SHALL MEET THE TEMPERATURE REQUIREMENTS OF ASTM C94. CONCRETE SHALL NOT BE PLACED UPON FROZEN SOILS OR SOILS WHICH CONTAIN FROZEN MATERIALS. CONCRETE SHALL BE PROTECTED FROM FREETING UNTIL THE SPECIFIED STRENGTH IS ATTAINED. 	 TOP CHORD LIVE LOAD TOP CHORD DEAD LOAD BOTTOM CHORD LIVE LOAD 	
 E. CONCRETE SHALL BE PROTECTED FROM FREEZING UNTIL THE SPECIFIED STRENGTH IS ATTAINED. 2. SOILS: 	 4. BOTTOM CHORD DEAD LOAD 5. ADDITIONAL LOADS AND LOCATIONS 	
 ALL SNOW AND ICE SHALL BE REMOVED FROM CUT AND FILL AREAS PRIOR TO ANY SITE WORK. B. NO FOUNDATION OR FILL MATERIAL SHALL BE PLACED UPON SOILS, WHICH ARE FROZEN OR CONTAIN FROZEN MATERIAL. 	 ENVIRONMENTAL DESIGN CRITERIA AND LOADS (WIND, RAIN, SNOW, SEISMIC, ETC.) OTHER LATERAL LOADS, INCLUDING DRAG STRUT LOADS 	
 FILL THAT HAS BEEN PLACED AND COMPACTED IN AN UNFROZEN STATE, WHICH SUBSEQUENTLY BECOMES FROZEN, SHALL BE RE-COMPACTED AT THE SURFACE (AFTER THAWING), BEFORE PLACING ADDITIONAL LIFTS. EXPOSED NATIVE SUBGRADE THAT BECOMES FROZEN SHALL BE THAWED AND COMPACTED IN PLACE FRIOR TO FOOTING PLACEMENT. 	 ADJUSTMENTS TO WOOD MEMBER AND METAL CONNECTOR PLATE DESIGN VALUE FOR CONDITIONS OF USE METAL-CONNECTOR-PLATE TYPE, SIZE, AND THICKNESS OR GAGE, AND THE DIMENSIONED LOCATION OF EACH METAL CONNECTOR PLATE. 	
 E. NO FROZEN SOILS SHOULD BE USED AS FILL. F. FOLOWING PLACEMENT OF FOUNDATIONS, AND BEFORE PLACEMENT OF FILL THAT WILL PROVIDE FROST PROTECTION, FROST 	CONNECTOR PLATES SHALL HAVE A CURRENT ICC-ES OR IAPMO EVALUATION REPORT. i. SIZE, SPECIES, AND GRADE FOR EACH WOOD MEMBER i. TRUSS-TO-TRUSS CONNECTIONS AND TRUSS FIELD ASSEMBLY REQUIREMENTS	
SHALL NOT BE PERMITTED TO PENETRATE BELOW FOUNDATIONS.	K. CALCULATED SPAN-TO-DEFLECTION RATIO AND MAXIMUM VERTICAL AND HORIZONTAL DEFLECTION FOR LIVE AND TOTAL LOAD, AS APPLICABLE	
IV. <u>CAST-IN-PLACE CONCRETE</u> 1. CONCRETE: A CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACL 201 JUNI ESS OTHERWISE NOTED	I. MAXIMUM AXIAL TENSION AND COMPRESSION FORCES IN THE TRUSS MEMBERS M. REQUIRED PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT LOCATION AND THE METHOD AND DETAILS OF RESTRAINT/BRACING TO BE	
 A. CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 301, UNLESS OTHERWISE NOTED. B. REQUIRED COMPRESSIVE STRENGTH, F'C: 1. CONCRETE ELEMENTS EXPOSED TO THE EXTERIOR GROUND AND WEATHER OR UNCONDITIONED SPACE OF THE 	USED 2. PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT: CONFORM WITH SECTION 2303.4.1.2 OF THE 2012 IBC. PROJECT-SPECIFIC PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING DESIGN, IF USED, SHALL BE SPECIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE	
 CONCRETE ELEMENTS EXPOSED TO THE EXTENSIVE GROUND AND WEATHER OR UNCONDITIONED SPACE OF THE BUILDING: 3500 PSI AT 28 DAYS, NORMAL WEIGHT. MAXIMUM WATER TO CEMENT RATIO = 0.45. CONCRETE ELEMENTS WITHIN THE CONDITIONED SPACE OF THE BUILDING: 2500 PSI AT 28 DAYS, NORMAL WEIGHT. 	F. TEMPORARY INSTALLATION BRACING/RESTRAINT: THE CONTRACTOR IS RESPONSIBLE FOR THE LATERAL AND INSTALLATION BRACING OF THE TRUSSES. TRUSS	
3. IF THE CONTRACTOR ELECTS TO REPLACE THE CEMENT IN THE CONCRETE MIX WITH HIGH-VOLUME FLY ASH, IT IS PERMISSIBLE TO ESTABLISH F'C AT 56 DAYS. THE CONTRACTOR SHALL COORDINATE THE DURATION OF SHORING	BRACING SHALL COMPLY WITH THE REQUIREMENTS OF TPI DSB-89. TEMPORARY BRACING INCLUDES TOP CHORD LATERAL BRACING, BOTTOM CHORD LATERAL BRACING, DIAGONAL BRACING, CROSS BRACING, AND GROUND BRACING.	
AND TEMPORARY BRACING ACCORDINGLY. C. DESIGN STRENGTH OF CONCRETE: 2500 PSI, THEREFORE NO SPECIAL INSPECTION REQUIRED D. THE CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF CONSTRUCTION OR POUR JOINTS TO THE ARCHITECT AND	G. TRUSSES SPANNING 60 FEET OR GREATER: THE OWNER SHALL CONTRACT WITH ANY QUALIFIED REGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING FOR ALL TRUSSES WITH CLEAR SPANS 60 FEET OR GREATER.	
 D. THE CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF CONSTRUCTION OR POUR JOINTS TO THE ARCHITECT AND ENGINEER FOR REVIEW. E. ROUGHEN CONCRETE SURFACES OF CONSTRUCTION JOINTS AND AT LOCATIONS WHERE CONCRETE IS CAST AGAINST 	3. WOOD STRUCTURAL PANELS	
EXISTING CONCRETE TO 1/4 AMPLITUDE AND CLEAN OF LAITANCE, FOREIGN MATTER, AND LOOSE PARTICLES. F. DURABILITY REQUIREMENTS:	A. ROOF: 19/32" THICK, MINIMUM, 40/20 SPAN RATING; PANEL GRADE: APA RATED SHEATHING. NAILING, UNLESS NOTED OTHERWISE: 1. 8d @ 6" O.C. AT PANEL EDGES. 2. 8d @ 12" O.C. AT INTERMEDIATE RAFTERS.	
 CONCRETE ELEMENTS EXPOSED TO THE EXTERIOR GROUND AND WEATHER OR UNCONDITIONED SPACE OF THE BUILDING: PROVIDE TOTAL AIR CONTENT IN ACCORDANCE WITH EXPOSURE CLASS F1 IN ACCORDANCE W/ ACI 318 ALL OTHER CONCRETE, NO REQUIREMENTS 	 B. FLOOR: 1-1/8" THICK, MINIMUM, 24 O.C. SPAN RATING; PANEL GRADE: APA RATED SHEATHING. GLUE AND NAILING, UNLESS NOTED OTHERWISE: 1. 10d @ 6" O.C. AT PANEL EDGES. 	
2. ALL OTHER CONCRETE: NO REQUIREMENTS 2. REINFORCING STEEL: A. TYPICAL REINFORCING: ASTM A615 GRADE 40 FOR #3 & #4 BARS AND ASTM A615 GRADE 60 FOR #5 BARS TO #7 BARS	2. 10d @ 12" O.C. AT INTERMEDIATE JOISTS. C. WALLS: 7/16" THICK, 24/0 SPAN RATING; PANEL GRADE: APA RATED SHEATHING. NAILING, UNLESS NOTED OTHERWISE: 1. 8d @ 6" O.C. AT PANEL EDGES.	
 B. REINFORCING TO BE WELDED: ASTM A706 GRADE 60 C. DEFORMED BAR ANCHORS: ASTM A496, Fy = 70 KSI. 	1. 50 @ 5" O.C. AT PANEL EUGES. 2. 8d @ 12 " O.C. AT INTERMEDIATE STUDS. D. ROOF SHEATHING OVER T&G: 7/16" THICK, 24/0 SPAN RATING; PANEL GRADE: APA RATED SHEATHING. NAILING, UNLESS NOTED OTHERWISE:	
 D. PROVIDE CLEARANCE AND COVER OF REBAR AS FOLLOWS, UNLESS OTHERWISE NOTED: 1. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES 2. FORMED SURFACES EXPOSED TO EARTH OR WEATHER: # 5 BARS AND SMALLER: 1 1/2 INCHES 	 0.148 X 1-1/2" MAX LENGTH @ 6" O.C. AT PANEL EDGES. 0.148 X 1-1/2" MAX LENGTH @ 12 " O.C. IN THE FIELD OF PANEL. 	
 FORMED SURFACES EXPOSED TO EARTH OR VEATHER, #6 BARS AND LARGER: 2 INCHES INTERIOR SLABS, WALLS, AND JOISTS: 3/4 INCHES 	3. INSTALL PSCL19/32 PANEL SHEATHING CLIPS @ UNSUPPORTED SHEATHING EDGES WHEN SHEATHING MEETS EDGE WITHOUT A RAFTER FOR SUPPORT. E. WOOD STRUCTURAL PANELS SHALL CONFORM TO VOLUNTARY PRODUCT STANDARDS PS 1 AND PS 2 AND APA PRP-108 PERFORMANCE STANDARDS.	
5. BEAMS AND COLUMNS: 1 1/2 INCHES TO TRANSVERSE REINFORCING E. UNLESS OTHERWISE NOTED, REINFORCING BARS SHALL BE SPLICED WITH 50-BAR-DIAMETER LAPS, MINIMUM.	 F. ALL SHEATHING SHALL BEAR THE APA TRADEMARK AND GRADE STAMP G. ALL END JOINTS SHALL BE STAGGERED AND SHALL BUTT ALONG THE CENTER LINES OF FRAMING MEMBERS. THE LONG DIMENSION OF BANELS SHALL BE INSTALLED PERPENDICULAR TO SUPPORTS WITH BANEL CONTINUOUS OVER TWO OF MORE SPANS 	
F. REINFORCING SHALL BE SUPPORTED PRIOR TO CONCRETING IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE, MSP-1. REINFORCING GUALL BE DETAILED IN ACCORDANCE WITH ACL 315	 H. THE LONG DIMENSION OF PANELS SHALL BE INSTALLED PERPENDICULAR TO SUPPORTS WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. I. PANELS SHALL NOT BE LESS THAN 4 x 8, EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING. THE MINIMUM PANEL DIMENSION FOR FLOOR SHEATHING AT BOUNDARIES SHALL BE 24 UNLESS ALL EDGES OF THE UNDERSIZED PANELS ARE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING. 	
 G. REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH ACI 315. H. WELDING OF REINFORCING IS PERMITTED ONLY WHERE SHOWN IN THE DRAWINGS. WELDING SHALL CONFORM TO AMS D1.4, STRUCTURAL WELDING CODE STEEL 	J. NAILS SHALL BE COMMON WIRE NAILS (NOT BOX OR SINKER NAILS) AND BE PLACED 3/8" MINIMUM FROM THE EDGE OF THE PANELS. THE MINIMUM NAIL PENETRATION INTO FRAMING MEMBERS SHALL BE 1 1/2" FOR &d NAILS AND 1 5/8" FOR 10d NAILS.	
3. SLAB ON GRADE CONTROL JOINTS: A. THE CONTRACTOR SHALL INSTALL TOOLED OR SAWCUT CONTROL JOINTS IN THE CONCRETE SLABS ON GRADE. THE JOINTS	K. WHERE SPECIAL INSPECTIONS ARE REQUIRED, PANEL NAILING SHALL BE INSPECTED PRIOR TO COVERING.	
SHALL BE 1/8 WIDE AND t/4 DEEP, WHERE t EQUALS THE SLAB THICKNESS. B. THE JOINTS SHALL SUB-DIVIDE THE SLAB INTO PANELS WITH THE LONGER SIDE NO GREATER THAN 1.5 TIMES THE LENGTH OF THE SHORTER SIDE	 4. WOOD DECKING BOARDS A. BOARDS SHALL BE LAID AT 90 DEGREES TO SUPPORTING MEMBERS. NAULING TO EACH INTERIOR SUPPORTING MEMBERS. 	
THE SHORTER SIDE. C. JOINTS IN INTERIOR SLABS SHALL BE SPACED AT NO FURTHER THAN 12-0" APART AND JOINTS IN EXTERIOR SLABS SHALL BE SPACED AT NO FURTHER THAN 6'-0".	 B. NAILING TO EACH INTERIOR SUPPORTING MEMBER: 1. 2x6 BOARDS: (2) GALVANIZED 16d NAILS OR #12 WOOD DECK SCREWS. 2. 2x8 BOARDS: (3) GALVANIZED 16d NAILS OR #12 WOOD DECK SCREWS. 	
DE SPACED AT NO FURTHER THAN 5-0. D. THE CONTRACTOR SHALL SUBMIT THEIR CONTROL JOINT PLAN TO THE ARCHITECT AND ENGINEER FOR REVIEW PRIOR TO THE FIRST SLAB ON GRADE CONCRETE POUR.	 NAILING TO EDGE SUPPORTING MEMBER: GALVANIZED 16d NAILS OR #12 WOOD DECK SCREWS AT 4 O.C. END JOINTS IN ADJACENT BOARDS SHALL BE SEPARATED BY AT LEAST ONE JOIST. THERE SHALL BE AT LEAST TWO BOARDS BETWEEN JOINTS ON THE SAME 	
 WELDED WIRE REINFORCEMENT: ASTM A1064, SHEETS ONLY FIBER-REINFORCED CONCRETE: ASTM C1116 TYPE III 4.1.3, 100% HOMOPOLYMER POLYPROPYLENE MD FIBRILLATED FIBERS, 1.5 	SUPPORT. 5. FASTENERS AND FRAMING ANCHORS AND CONNECTORS:	
POUND PER CUBIC YARD, MINIMUM APPLICATION RATE	A. NAILS: BOX NAILS 1. 8d = 0.113 DIA. x 2 3/8" LONG 2. 10d = 0.131 DIA. x 3" LONG	
 V. <u>POST-INSTALLED ANCHORS</u> 1. ADHESIVE ANCHORS AND DOWELS IN CONCRETE: SET-XP (ICC-ES ESR-2508) OR AT-XP (IAPMO UES ER-263) BY SIMPSON STRONG-TIE OR HIT-HY 200 (ICC-ES 	2. 104 = 0.131 DIA. X 3" LONG 3. 164 = 0.162 DIA. X 3 1/2" LONG. B. LAG BOLTS AND THRU-BOLTS: ASTM A307	
ESR-3187) BY HILTI. 2. EXPANSION ANCHORS IN CONCRETE: STRONG-BOLT 2 (ICC-ES ESR-3037) BY SIMPSON STRONG-TIE OR KWIK BOLT TZ (ICC-ES ESR-1917) BY HILTI. 3. SCREW ANCHORS IN CONCRETE: TITEN HD (ICC ES ESR-3213) BY SIMPSON STRONG TE OR KWIK BUL TZ (ICC-ES ESR-1917) BY HILTI.	1. THRU-BOLT HOLES SHALL BE 1/16 LARGER THAN BOLT DIAMETER. PROVIDE STANDARD CUT WASHER UNDER ALL HEAD AND NUTS FOR BOLTS BEARING ON WOOD.	
 SCREW ANCHORS IN CONCRETE: TITEN HD (ICC-ES ESR-2713) BY SIMPSON STRONG-TIE OR KWIK HUS-EZ (ICC-ES ESR-3027) BY HILTI. FOLLOW MANUFACTURERS INSTALLATION INSTRUCTIONS FOR ALL POST-INSTALLED ANCHORS PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN EXPOSED TO WEATHER. PROVIDE ELECTRO-PLATED CARBON STEEL ANCHORS AT OTHER 	 INSTALL LAG BOLTS IN DRILLED PILOT HOLES EQUAL TO 3/4 TIMES THE BOLT SHANK DIAMETER. DO NOT HAMMER OR OVER-DRIVE BOLTS. PROVIDE STANDARD CUT WASHER UNDER ALL LAG BOLT HEADS BEARING ON WOOD. WOOD SCREWS, AS SPECIFIED ON PLANS. 	
 PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN EXPOSED TO WEATHER. PROVIDE ELECTRO-PLATED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS NOTED OTHERWISE. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF (2) 	 C. WOOD SCREMS: AS SPECIFIED ON PLANS 1. SDS SCREMS: 2" EMBEDMENT FOR STEEL SIDE PLATE AND SHEAR WALL CONNECTIONS. 3" 	
ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE ANCHOR AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, SEEK GUIDANCE FROM THE ENGINEER.	EMBEDMENT FOR LEDGER TO NARROW FACE OF STUD CONNECTIONS. 2. SDWC SCREWS: 2" EMBEDMENT FROM TOP PLATE TO ROOF RAFTER OR ROOF TRUSS. INSTALL PER SIMPSON SPECIFICATIONS.	
1. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH POST-INSTALLED ANCHORS.	D. FRAMING ANCHORS AND CONNECTORS: SIMPSON STRONG-TIE, ICC-ES ESR 2523, OR APPROVED EQUAL	
8. SUBSTITUTIONS: SUBSTITUTE PRODUCTS SHALL HAVE AN ASSOCIATED ICC-ES OR IAPMO EVALUATION REPORT AND THE CONTRACTOR MUST DEMONSTRATE PERFORMANCE IS EQUIVALENT TO THE SPECIFIED PRODUCTS. SUBSTITUTIONS WILL NOT BE CONSIDERED UNLESS THIS INFORMATION IS SUBMITTED.	E. METAL CONNECTORS AND TREATED LUMBER: 1. ALL METAL CONNECTORS IN CONTACT WITH TREATED LUMBER SHALL BE STAINLESS STEEL, BATCH/POST HOT-DIP GALVANIZED PER ASTM A123 OR A153, OR PROPRIETARY EQUIVALENT.	
VI. STRUCTURAL STEEL FRAMING 1. MATERIALS:	 ADD, OK FROFRIETAKT EQUIVALENT. 2. FASTENERS ARE TO MATCH THE FINISH AND MATERIAL OF THE CONNECTORS. 6. CUTTING, BORING, AND NOTCHING OF WOOD MEMBERS: 	
A. WIDE FLANGE AND WT SHAPES: ASTM A992 B. Channels, angles, plates, and bars: Astm A36 C. GRADE EO RI ATES, ASTM A572 EU - EO LISE ONLY WHERE INDICATED ON THE RUANS	 A. STUDS: 1. IN EXTERIOR WALLS AND BEARING PARTITIONS, ANY WOOD STUD MAY BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25 PERCENT OF ITS WIDTH. 	
 GRADE 50 PLATES: ASTM A572, Fy = 50 KSI. USE ONLY WHERE INDICATED ON THE PLANS. D. PIPE: ASTM A53, GRADE B E. HSS: ASTM A500 OR ASTM A1085, GRADE B, Fy = 42 KSI FOR ROUNDS AND 46 KSI FOR RECTANGULAR AND SQUARE 	CUTTING OR NOTCHING OF STUDS TO A DEPTH NOT GREATER THAN 40 PERCENT OF THE WIDTH OF THE STUD IS PERMITTED IN NONBEARING PARTITIONS SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE PARTITION. 2. A HOLE NOT GREATER IN DIAMETER THAN 40 PERCENT OF THE STUD WIDTH MAY BE BORED IN ANY WOOD STUD. BORED HOLES NOT GREATER THAN	
2. FASTENERS: A. MACHINE BOLTS: ASTM A307	2. A HOLE NOT GREATER IN DIAMETER THAN 40 PERCENT OF THE STUD WIDTH MAY BE BORED IN ANY WOOD STUD. BORED HOLES NOT GREATER THAN 60 PERCENT OF THE WIDTH OF THE STUD ARE PERMITTED IN NONBEARING PARTITIONS OR IN ANY WALL WHERE EACH BORED STUD IS DOUBLED, PROVIDED NOT MORE THAN TWO SUCH SUCCESSIVE DOUBLED STUDS ARE SO BORED. IN NO CASE SHALL THE EDGE OF THE BORED HOLE BE NEARER	
 BOLTS: ASTM A325-X ANCHOR RODS: ASTM F1554, GRADE 36, THREADED WITH NUT, UNLESS OTHERWISE NOTED, AND HOOKED FOR ANCHORING WOOD SOLE PLATES. 	THAN 5/8 INCH TO THE EDGE OF THE STUD. BORED HOLES SHALL NOT BE LOCATED AT THE SAME SECTION OF STUD AS A CUT OR NOTCH. B. JOISTS AND RAFTERS:	
 D. SHEAR STUD CONNECTORS AND WELDED THREADED STUDS: ASTM A108, GRADE 1010 THRU 1020 E. NUTS: ASTM A563 F. HARDENED PLAIN AND BEVELED WASHERS: ASTM E436 	1. NOTCHES AT THE ENDS OF JOISTS AND RAFTERS SHALL NOT EXCEED ONE FOURTH THE DEPTH. NOTCHES IN THE TOP OR BOTTOM OF JOISTS OR RAFTERS SHALL NOT EXCEED ONE SIXTH THE DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE ONE THIRD OF THE SPAN, EXCEPT THAT A NOTCH	
F. HARDENED PLAIN AND BEVELED WASHERS: ASTM F436 3. WELDING A. ARC-WELDING ELECTRODES AND/OR FILLER METALS TO BE LOW HYDROGEN TYPES E7XTX, E7XTXX, OR E70XXX, MINIMUM, AS APPLICABLE.	NOT EXCEEDING ONE THIRD OF THE DEPTH IS PERMITTED IN THE TOP OF A RAFTER OR CEILING JOIST NOT FURTHER FROM THE FACE OF THE SUPPORT THAN THE DEPTH OF THE MEMBER. 2. HOLES BORED IN JOISTS OR RAFTERS SHALL NOT BE WITHIN 2 INCHES OF THE TOP AND BOTTOM AND THEIR DIAMETER SHALL NOT EXCEED ONE THIRD	
 B. WELDING SHALL CONFORM TO AMS D1.1, STRUCTURAL WELDING CODE - STEEL. C. ALL WELDING SHALL BE PERFORMED BY A WELDER CERTIFIED BY AMS AND THE GOVERNING JURISDICTION, IF APPLICABLE. 	 HOLES BORED IN JOISTS OR RAFTERS SHALL NOT BE WITHIN 2 INCHES OF THE TOP AND BOTTOM AND THEIR DIAMETER SHALL NOT EXCEED ONE THIRD THE DEPTH OF THE MEMBER. C. BEAMS: 	
D. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AMS SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTORS RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD	 DLAMS. NOTCHES ARE NOT PERMITTED UNLESS APPROVED OR DETAILED BY THE ENGINEER, SUBJECT TO THE FOLLOWING LIMITATIONS. NOTCHES IN SAWN LUMBER BENDING MEMBERS SHALL NOT EXCEED ONE SIXTH THE DEPTH OF THE MEMBER AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE 	
WELDS. 4. FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH AISC 360 AND AISC 303 A FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH AISC 360 AND AISC 303	SPAN. WHERE MEMBERS ARE NOTCHED AT THE ENDS, THE NOTCH DEPTH SHALL NOT EXCEED ONE FOURTH THE BEAM DEPTH. THE TENSION SIDE OF SAWN LUMBER BENDING MEMBERS OF 4 INCHES IN NOMINAL THICKNESS SHALL NOT BE NOTCHED UNLESS SPECIFICALLY APPROVED BY THE ENGINEER.	
 A. FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH AISC 360 AND AISC 303. B. UNLESS NOTED OTHERWISE, PRIME STRUCTURAL STEEL AND FASTENERS THAT ARE PERMANENTLY EXPOSED TO THE WEATHER WITH A HIGH- PERFORMANCE ACRYLIC COATING, PRO CRYL UNIVERSAL BY SHERWIN WILLIAMS, OR EQUAL. FIELD CUT, WELDED, AND/OR DAMAGED SURFACES SHALL 	 HOLES FOR PIPES, ETC. SHALL NOT BE BORED IN SAWN LUMBER BENDING MEMBERS OF 4 INCHES OR GREATER WITHOUT SPECIFIC DETAILS FROM THE ENGINEER. D. ENGINEERED LUMBER AND PREFABRICATED WOOD I-JOISTS: CONFORM TO MANUFACTURERS RESTRICTIONS FOR CUTTING. BORING. AND NOTCHING. 	
DERFORMANCE ACKILIC COATING, FRO CKIL UNVERSAL DI SHERVIN MILLIAMS, OR EQUAL FIELD CUT, MELDED, AND/OR DAMAGED SURFACES SHALL BE TOUCHED UP WITH PRIMER PRIOR TO APPLYING FINISH COAT OF PAINT. C. STRUCTURAL STEEL AND FASTENERS INDICATED ON THE DRAWINGS TO BE HOT-DIP GALVANIZED SHALL BE COATED IN ACCORDANCE WITH ASTM A123	2. ENVINEENED EVIDEN AND I NEI AUNIATED NUUD FJUIDID; UUNT UNMITU MAINUFAUTUKEKO KEDIKICHUNO FUK UUTTING, BUKING, AND NOTCHING.	
AND ASTM A153. REPAIR AND TOUCH UP GALVANIZING AFTER ERECTION ACTIVITIES ARE COMPLETE IN ACCORDANCE WITH ASTM A780.		

OOD MEMBERS NOT SHOWN ON THESE DRAWINGS OR IN THESE NOTES, USE THE IBC FASTENING SCHEDULE, TABLE 2304.9.1. L BE PRESSURE TREATED, PAINTED OR STAINED. MAINTENANCE SHALL BE THE RESPONSIBILITY OF THE OWNER. FOLLOW THE ENDATIONS FOR EXTERIOR APPLICATIONS. BELOW FLOOR FRAMING AND PREFABRICATED TRUSSES SHALL BE SLIP CONNECTED TO ALLOW FOR POTENTIAL FRAMING

RMANCE WITH THE TFEC 1-2019 CODE OF STANDARD PRACTICE FOR TIMBER FRAME STRUCTURES. RADED BY A GRADER CERTIFIED BY AN APPROVED LUMBER GRADING AGENCY OR A QUALIFIED INDIVIDUAL WHO HAS COMPLETED NG COURSE. TIMBERS SHALL BEAR A GRADE STAMP OR A CERTIFICATE OF GRADE FROM THE LUMBER GRADER.

S DELIVERY AND STORAGE. STORE TIMBERS OFF THE GROUND AND KEEP COVERED. 50 AS NOT TO ENCOURAGE THE GROWTH OF SAP-STAIN FUNGI, MOLD OR MILDEW. STACK TIMBERS WITH STICKERS AND SPACERS OW AIR CIRCULATION. OTECTION DURING TRANSPORTATION, STORAGE, HANDLING AND ERECTION OF THE TIMBERS TO AVOID MARRING, STAINING, OR THE MOISTURE, DIRT AND FOREIGN MATTER.

RS AND EXPOSED LUMBER SHALL BE KILN DRIED TO A MAXIMUM SURFACE MOISTURE CONTENT OF 19%. ER SHALL BE FREE OF HEART CENTER (FOHC). TIMBERS LARGER THAN 8X12 MAY BE BOXED HEART. COORDINATE RELIED CUTS

POSED LUMBER SIZES ARE ACTUAL DIMENSIONS. RAINED WHITE OAK CONFORMING TO ASTM D&023. IF TAPERED PEGS ARE USED, PEGS SHALL BE TAPERED FOR AT LEAST 1/3 OF ST FOUR INCHES LONGER THAN THE THICKNESS OF THE TIMBER IN WHICH THEY ARE DRIVEN SO THAT THE TAPERED SECTION

1BER. SCREWS ARE ACCEPTABLE FOR GENERAL USE. ALL SCREWS SHALL HAVE MINIMUM 3 EMBEDMENT INTO MAIN MEMBER.

CREWS: 3" EMBEDMENT FOR TIMBER CONNECTIONS. 1-1/2" EMBEDMENT FOR LEDGER TO RIM CONNECTIONS. 3" EMBEDMENT FOR LEDGER OF STUD CONNECTIONS. USE "EXTERIOR RATED" SCREWS FOR ALL TIMBER CONNECTIONS. LTS: ASTM A307

ES SHALL BE 1/16 LARGER THAN BOLT DIAMETER. PROVIDE STANDARD CUT WASHER UNDER ALL HEAD AND NUTS FOR BOLTS TS IN DRILLED PILOT HOLES EQUAL TO 3/4 TIMES THE BOLT SHANK DIAMETER. DO NOT HAMMER OR OVER DRIVE BOLTS. PROVIDE NASHER UNDER ALL LAG BOLT HEADS BEARING ON WOOD.

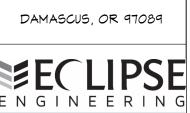
CATED IN STRICT CONFORMANCE TO APPROVED SHOP DRAWINGS. FABRICATION PRIOR TO RECEIPT OF APPROVED SHOP HE CONTRACTORS RISK.

BERS SHALL BE FABRICATED TO JOIN TIGHTLY AND IN PROPER ALIGNMENT AT THE TIME OF ASSEMBLY. SMALL (1/8 OR LESS) NCES OF JOINTS COME TOGETHER (IN ALIGNMENT OR SEPARATION) ARE ACCEPTABLE. PROVISIONS SHALL BE MADE IN THE ALLOW FOR DIMENSIONAL CHANGES (JOINT SEPARATION AND ALIGNMENT) ASSOCIATED WITH TIMBER DRYING SHRINKAGE.

REVIEWED FOR CODE OMPLIANCE 04/28/2025



04/16/2025



A CUSHING TERRELL COMPANY

2140 SW. JEFFERSON ST., SUITE 200 PORTLAND, OR 97201 ECLIPSE-ENGINEERING.COM (503) 395-1229



1



BERLET

'SHASTA'

38980 MAIN STREET MILNER, CO 80477

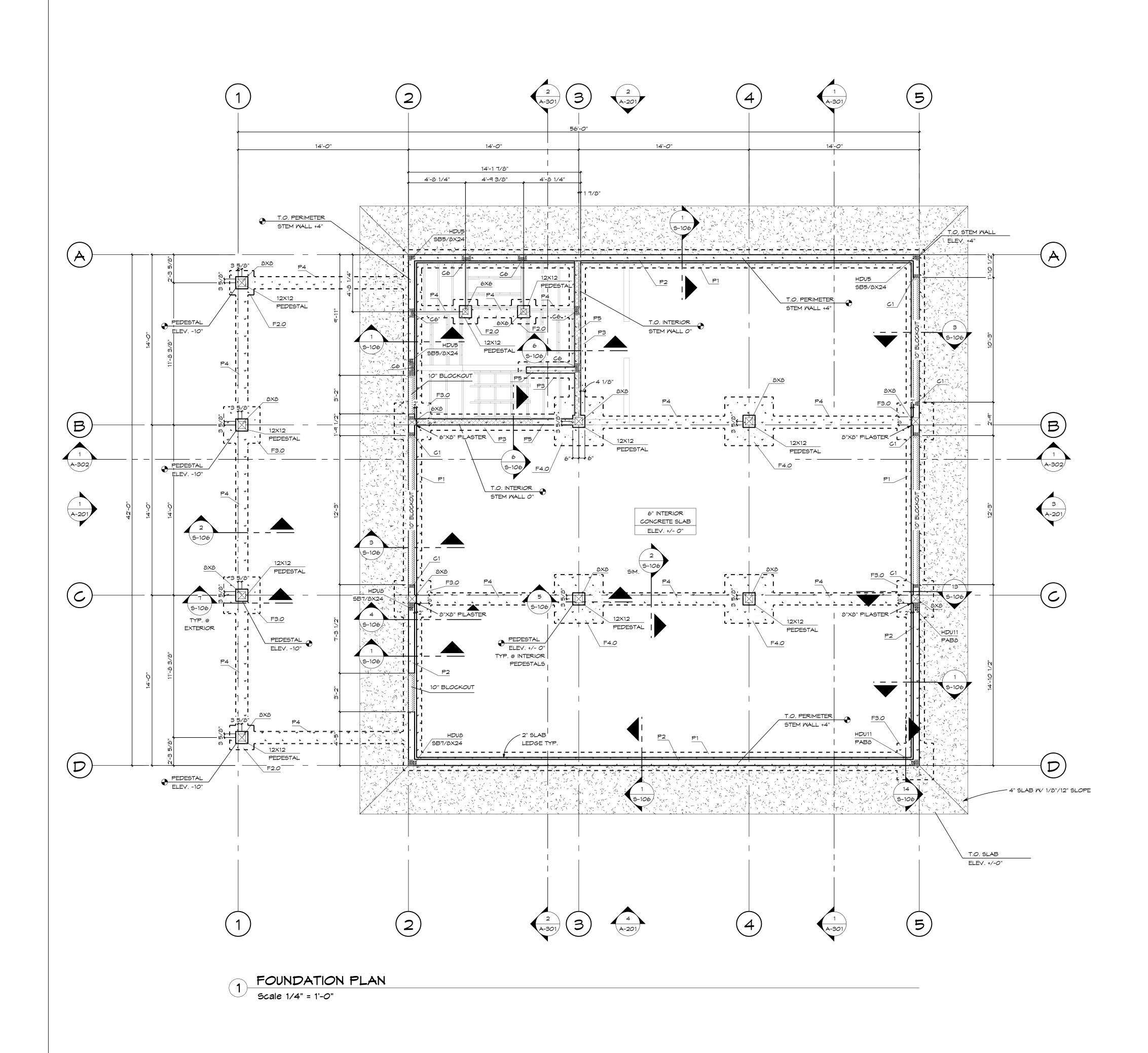
STRUCTURAL NOTES

DRAWN BY RJF

04/15/2025

NAILING/ FASTENING SCHEDULE

CONNECTION FASTENING LOCATION JOISTS TO SILL OR GIRDER 3- 8d BOX (2 1/2" x 0.131") TOENAIL BRIDGING TO JOIST TOENAIL EACH END 2- 8d BOX (2 1/2" x 0.131") . SOLE PLATE TO JOIST OR BLOCKING 3" x 0.131" NAILS @ 8" O.C. TYPICAL FACE NAIL SOLE PLATE TO JOIST OR BLOCKING @ BRACED WALL PANEL 4- 3" x 0.131" NAILS @ 16" 0.0 BRACED WALL PANELS . TOP PLATE TO STUD 3- 3" x 0.131" NAILS END NAIL 5. STUD TO SOLE PLATE 4- 8d BOX (2 1/2" x 0.131") TOENAIL 4- 3" 0.131" NAILS 3- 3" x 0.131" NAILS END NAIL FACE NAIL TYPICAL FACE NAIL DOUBLE STUDS 3" x 0.131" NAIL @ 8" O.C DOUBLE TOP PLATES 3" x 0.131" NAIL @ 12" O.C. BLOCKING BETWEEN JOIST OR RAFTERS TO TOP PLATE 3- 8d BOX (2 1/2" x 0.131") TOENAIL 3- 3" x 0.131" NAILS 9. RIM JOIST TO TOP PLATE 8d (2 1/2" x 0.131") @ 6" O.C. TOENAIL 3" x 0.131" NAIL @ 6" O.C. 10. DOUBLED TOP PLATES, TYPICAL FACE NAIL 3- 3" x 0.131" NAILS FACE NAIL 1. CEILING JOISTS TO PLATE 3- 8d BOX (2 1/2" x 0.131") TOENAIL 5- 3" x 0.131" NAILS 12. CEILING JOISTS, LAPS OVER PARTITIONS FACE NAIL 4- 3" x 0.131" NAILS 13. RAFTER TO PLATE 3- 8d BOX (2 1/2" x 0.131 TOENAIL 3- 3" x 0.131" NAILS 14. BUILT-UP CORNER STUDS 3" x 0.131" NAILS 16" O.C. FACE NAIL @ TOP & BTM STAGGERED ON OPPOSITE SIDES 15. BUILT-UP GIRDER & BEAMS 3" x 0.131" NAILS @ 24" 0. 3- 3" x 0.131" NAILS 3- 3" x 0.131" NAILS FACE NAIL @ ENDS & @ EACH SPLICE 16. ROOF RAFTER TO 2X RIDGE BEAM TOENAIL



				REVIEWED FOR CODE COMPLIANC 04/28/2025	04/16/2025 ADO L/C AND ADO L/
					DAMASCUS, OR 97089
				OLUMN SCHEDULE	
	PLA				
		C1 C2		-UP (2) 2X6 -UP (3) 2X6	A CUSHING TERRELL COMPANY
		C2 C3		-UP @ 2' SMALL CORBEL POST	2140 SW. JEFFERSON ST., SUITE 200 PORTLAND, OR 97201 ECLIPSE-ENGINEERING.COM (503) 395-1229
		C4		-UP @ CORNER 2' SMALL CORBEL POST	
		C5	BUILT	-UP @ CORNER 4' LARGE CORBEL POST	\searrow
		C6	BUILT	-UP @ BEAM POCKET POST	$\left\langle \right\rangle$
					\geq
EMERAT	WALL OR SL			TO BE USED IN HOLDOWNS ONLY OF CONCRETE OR SLAB TOP OF FOOTING NOTE: ALL PAB ANCHORS @ PERIMETER STEM WALLS NEED TO BE INSTALLED INTO SPREAD FOOTING	
<u>SB ANCHOR</u>	<u>SSTB AN</u>		SCHEDU	PAB ANCHOR	$\langle \cdot \rangle$
HOLDOWN	ANCHOR I			MINIMUM WOOD MEMBER	
MSTA, AND CMST STRAPS		FACTURE		(2) 2x STUD	
HDU5-SDS2.5	SB5/8X24	W/ 18" EN	IBED.	(2) 2x STUD	$\langle \rangle$
HDU8-SD52.5	5B7/8X24	W/ 18" EN	IBED.	(3) 2x STUD	
HDU11-SDS2.5	PAB8 W	/ 11" EMB	ED.	(3) 2x STUD	
REF. DE	TALS OF	N 5-10	06 FC	D BE CAST IN PLACE OR FOUNDATION OWN LOCATIONS	
NOTE: HOLL	7011 AN	ICHO	RBC	NLT LOCATIONS ON	\succ
PLANS AF	RE APPR	OXIM	ATE	ONLY. REFER TO	<u> </u>
				PLAN PROVIDED BY	Drawing Index
-	-			DIMENSIONS AND	No. Date Description 1 03.14.25 REV 1
LOCATIC	NS OF H	OLD	AME	5 AND ANCHORS.	
FOUNDAT	ION SC	HED	NLF		
TYPE		· · <u></u>	F	REINFORCEMENT	<u>OMNER INFORMATION</u> LUKE & CARISSA
6" SLAB 24"X24"X12"				#4 EACH WAY, BTM.	BERLET
36"X36"X12" 48"X48"X12"			(4)	#4 EACH WAY, BTM. #4 EACH WAY, BTM.	
18"X10" CONT FOO				2) #4 CONT BTM	

TYPE MARK	COUNT	TYPE	REINFORCEMENT
		6" SLAB	#3 @ 18" O.C. EACH WAY
F2.0	4	24"X24"X12"	(3) #4 EACH WAY, BTM.
F3.0	7	36"X36"X12"	(4) #4 EACH WAY, BTM.
F4.0	4	48"X48"X12"	(6) #4 EACH WAY, BTM.
P 1	1	18"X10" CONT. FOOTING	(2) #4 CONT., BTM.
P2	1	8" STEM WALL	#4 VERT. @ 24" O.C. & #4 HORZ. @ 12" O.C.
P3	1	16"X12" CONT. FOOTING	(2) #4 CONT., BTM.
P4	1	12"X12" GRADE BEAM	(2) #5 BARS, TOP & BTM.
P5	1	6" STEM WALL	#4 VERT. @ 24" O.C. & #4 HORZ. @ 12" O.C.

FRAMING LEGEND

\boxtimes	DENOTES POST ABOVE AND/OR CONTINOUS BELOW
{······	DENOTES EXTERIOR WALL
<u>}</u>	DENOTES INTERIOR WALL
	DENOTES BEARING WALL
	DENOTES BLOCKOUT IN FOUNDATION WALL
	DENOTES CONCRETE WALL
	DENOTES CONCRETE FOOTING
	DENOTES EXTERIOR CONCRETE SLABS

FOUNDATION PLAN GENERAL NOTES NOTES APPLY TO SHEETS 5-101 ONLY.

1. ITEMS LISTED ARE TO BE PROVIDED BY OTHERS, CONTACT OWNER FOR SPECIFICATIONS, DCS IS NOT RESPONSIBLE U.N.O.

-FOUNDATION: CONCRETE, ANCHOR BOLTS, FIBER MESH, WATER PROOF MEMBRANE, REBAR, INSULATION -ROOFING: ROOFING MATERIAL MOISTURE BARRIER, VENTING, GUTTERS, FLASHING, NAILS, WATER PROOFING. -GENERAL: INSULATION, STONE MASONRY, NAILS & SCREMS, PLUMBING & FIXTURES, ELECTRICAL & FIXTURES, -MECHANICAL & FIXTURES, CASEMORK, FLOOR FINISH, WALL FINISH, & CEILING FINISH

DRAWN BY RJF

04/15/2025

5-101

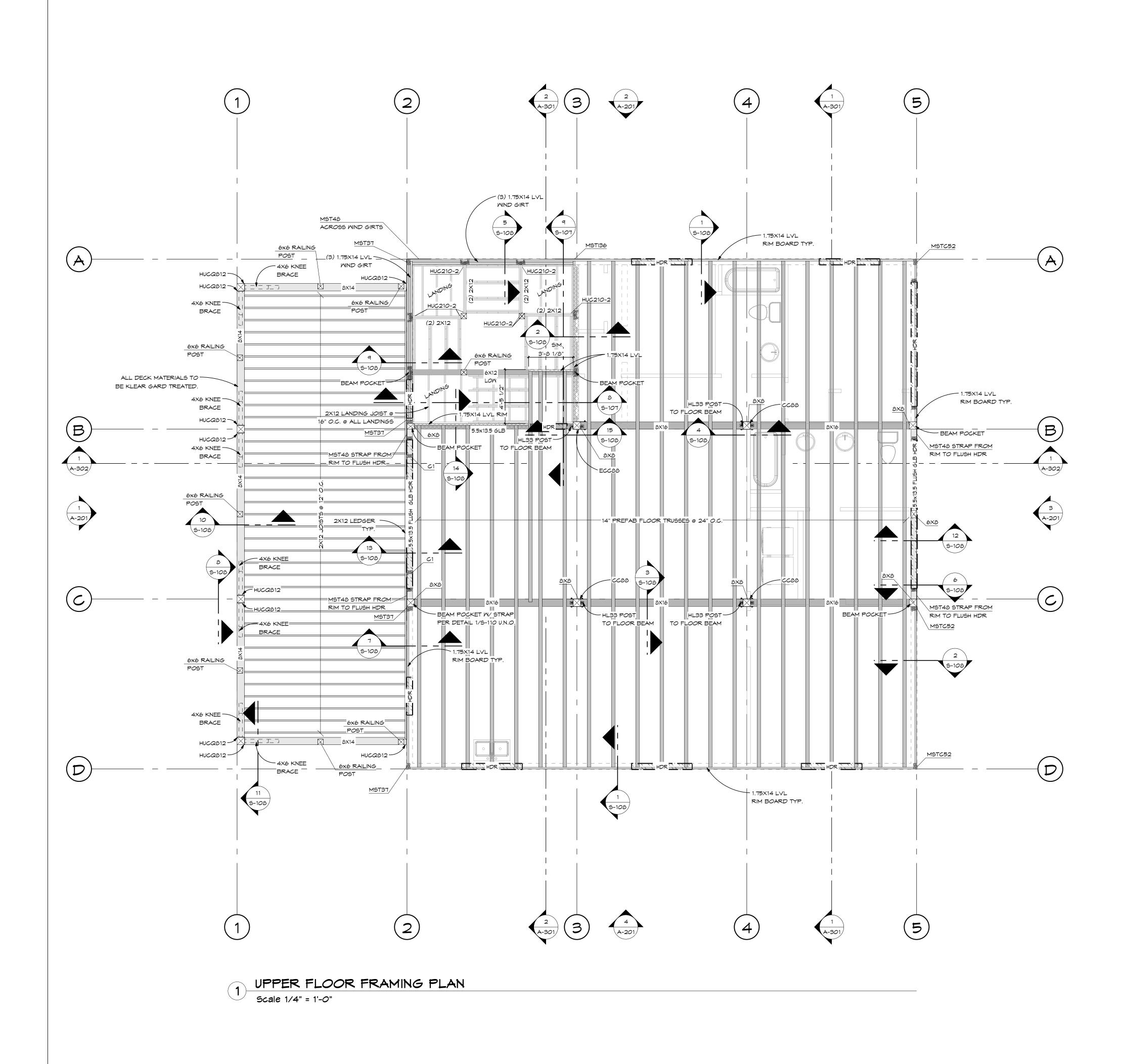
BERLET

'SHASTA'

38980 MAIN STREET MILNER, CO 80477

FOUNDATION

PLAN



REVIEWED FOR CODE COMPLIANCE 04/28/2025	04/16/2025 ADO L/C ADO L/C ADO L/C ADO L/C ADO L/C ADO D D S S S S S S S S S S S S S
	DAMASCUS, OR 97089
BEL POST IALL CORBEL POST RGE CORBEL <u>POST</u> POST	Drawing Index Drawing Index No. Date 1 03.14.25
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	OWNER INFORMATION LUKE & CARISSA BERLET BERLET 'SHASTA'
CONTINOUS BELOW	38980 MAIN STREET MILNER, CO 80477 UPPER FLOOR ERAMING
5 14' TYPE.	FRAMING PLAN 5-102 DRAWN BY RJF 04/15/2025

COLUMN SCHEDULE					
PLAN	TAG	DESCRIPTION			
XX	C 1	BUILT-UP (2) 2X6			
XXX	С2	BUILT-UP (3) 2X6			
Mark Mark	СЗ	BUILT-UP @ 2' SMALL CORBEL POST			
	C4	BUILT-UP @ CORNER 2' SMALL CORBEL POST			
MæM	C5	BUILT-UP @ CORNER 4' LARGE CORBEL POST			
N AN	C6	BUILT-UP @ BEAM POCKET POST			

KING STUD SCHEDULE NUMBER OF STUDS

NORIBER OF STODS								
	STUD HEIGHT							
ROUGH OPENING		6" WALL						
WIDTH	≤ 6'-0"	≤ 8'-0"	≤ 10'-0"	≤ 12'-0"	≤ 14'-0"	≤ 16'-0"		
LESS THAN 4'-0"	1	1	1	1	2	2		
4'-0" - 5'-11"	1	1	1	2	2	m		
6'-0" - 7'-11"	1	1	1	2	m	4		
8'-0" - 9'-11"	1	1	1	2	m	4		
10'-0" - 11'-11"	1	1	2	2	n	ы		
12'-0" - 13'-11"	1	1	2	з	4	6		

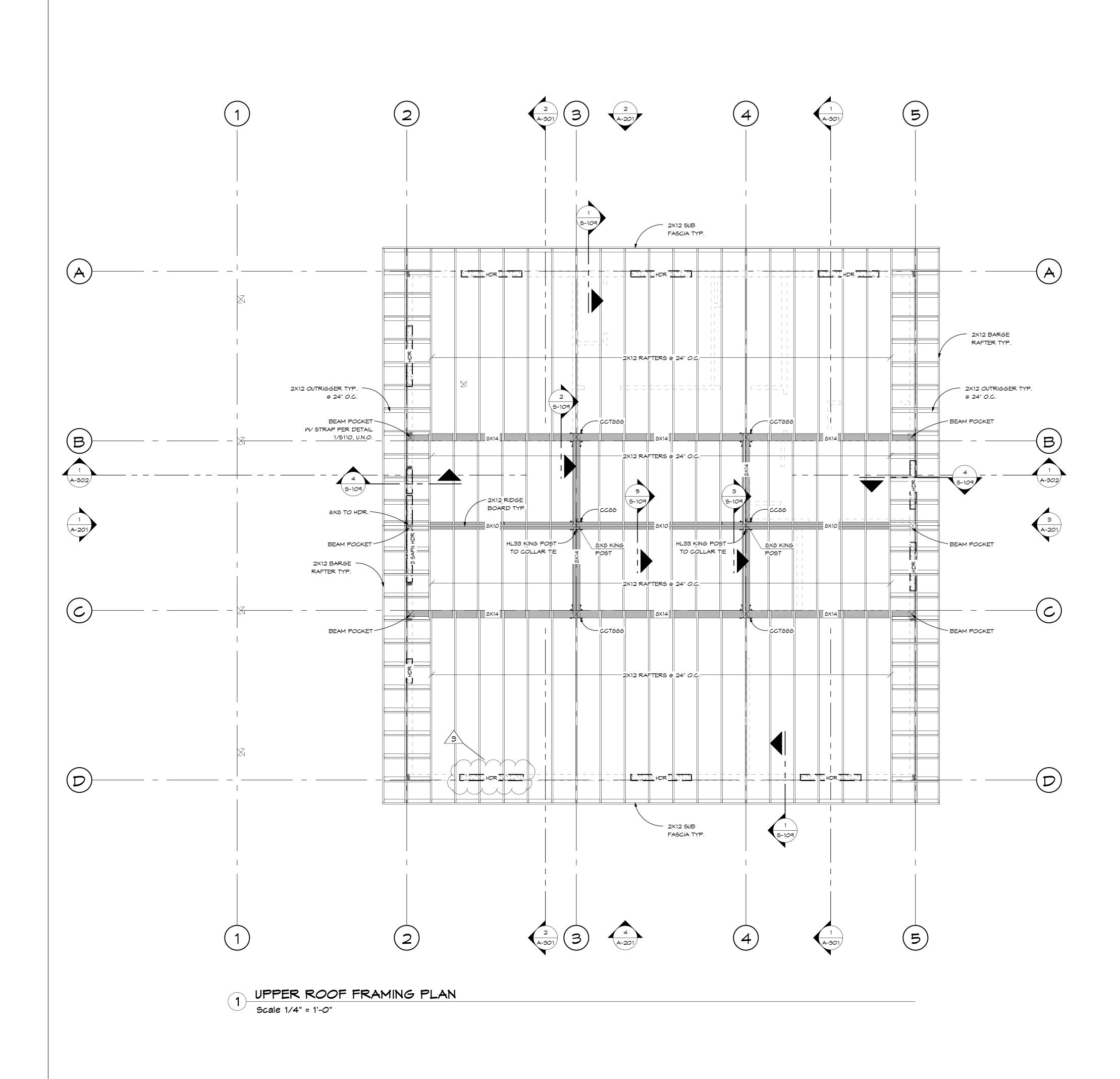
FRAMING LEGEND

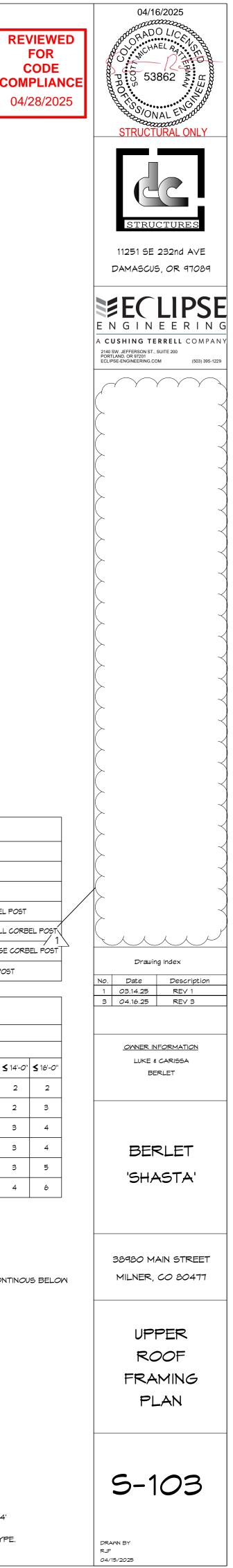
\boxtimes	DENOTES POST BELOW
\boxtimes	DENOTES POST ABOVE AND/OR CONTINOUS BELOW
$\{\times\!\times\!\times\!\times\!\times\!\times\!\times\!$	DENOTES LAYON OVER-FRAMING
$\left\{ \begin{array}{cccccccccccccccccccccccccccccccccccc$	DENOTES WALL BELOW
	DENOTES BEARING WALL BELOW
	DENOTES WALL ABOVE
	DENOTES HEADER BELOW
	DENOTES ROOF BELOW

FRAMING PLAN GENERAL NOTES NOTES APPLY TO SHEETS 5-102, 5-103 ONLY

- 1. ALL HEADERS TO BE 5.5 X 6 GLB U.N.O.
- 2. PROVIDE (2) 2X6 STUDS @ 16" O.C. FOR STUDS TALLER THAN 14'

3. VERIFY VENTILATION DESIGN REQUIREMENTS PER INSULATION TYPE.





		COLUMN SCHEDULE
PLAN	TAG	DESCRIPTION
XX	C1	BUILT-UP (2) 2X6
XXX	С2	BUILT-UP (3) 2X6
Mark Mark	СЗ	BUILT-UP @ 2' SMALL CORBEL POST
	C4	BUILT-UP @ CORNER 2' SMALL CORBEL POST
MæM	C5	BUILT-UP @ CORNER 4' LARGE CORBEL POST
N N	С6	BUILT-UP @ BEAM POCKET POST

REVIEWED FOR CODE

04/28/2025

KING STUD SCHEDULE NUMBER OF STUDS

	STUD HEIGHT							
ROUGH OPENING		6" MALL						
MIDTH	≤ 6'-0"	≤ 8'-0"	≤ 10'-0"	≤ 12'-0"	≤ 14'- <i>0</i> "	≤ 16'-0"		
LESS THAN 4'-O"	1	1	1	1	2	2		
4'-0" - 5'-11"	1	1	1	2	2	n		
6'-0" - 7'-11"	1	1	1	2	m	4		
8'-0" - 9'-11"	1	1	1	2	m	4		
10'-0" - 11'-11"	1	1	2	2	n	IJ		
12'-0" - 13'-11"	1	1	2	n	4	6		

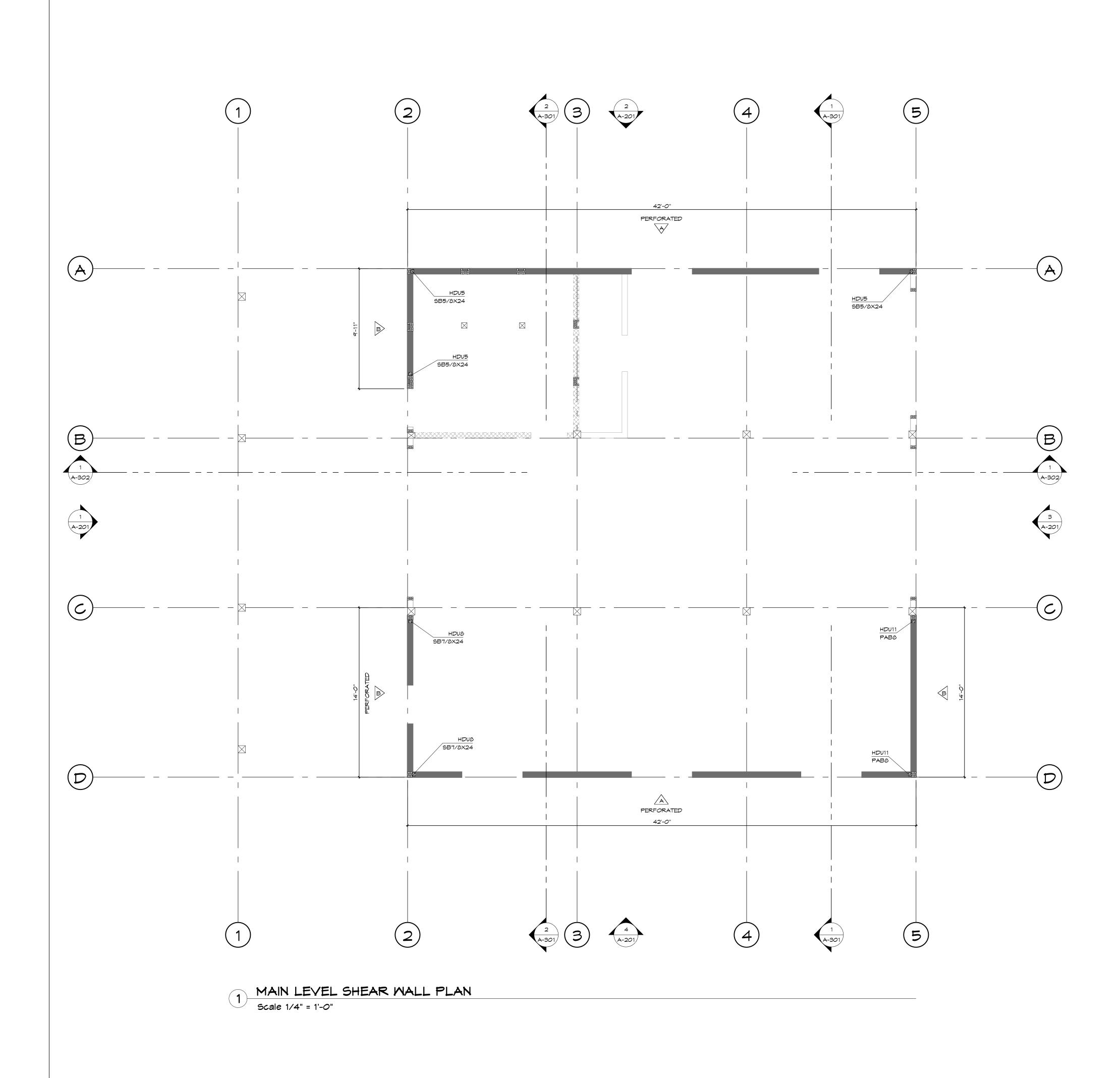
FRAMING LEGEND

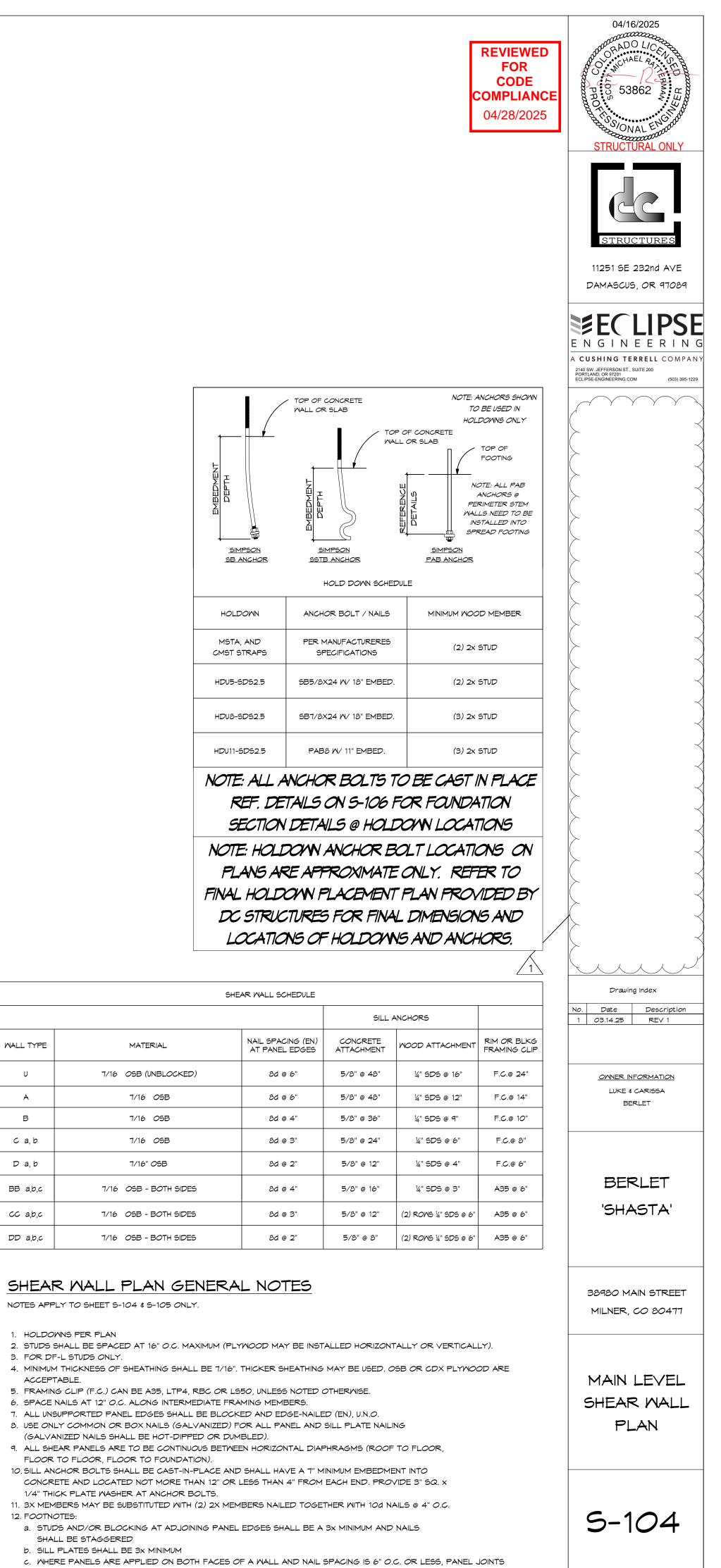
\boxtimes	DENOTES POST BELOW	
\boxtimes	DENOTES POST ABOVE AND/OR CONTINOUS BELOW	
$\times \times \times \times \times \times \times $	DENOTES LAYON OVER-FRAMING	
	DENOTES WALL BELOW	
	DENOTES BEARING WALL BELOW	
}	DENOTES WALL ABOVE	
	DENOTES HEADER BELOW	
	DENOTES ROOF BELOW	

FRAMING PLAN GENERAL NOTES NOTES APPLY TO SHEETS 5-102, 5-103 ONLY

- 1. ALL HEADERS TO BE 5.5 X 6 GLB U.N.O.
- 2. PROVIDE (2) 2X6 STUDS @ 16" O.C. FOR STUDS TALLER THAN 14'

3. VERIFY VENTILATION DESIGN REQUIREMENTS PER INSULATION TYPE.





b. SILL PLATES SHALL BE 3X MINIMUM C. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS 6" O.C. OR LESS, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS.

13. ALL WALLS NOT LABELED ARE TO BE FRAMED PER TYPE "U".

MATERIAL

7/16 OSB

7/16 OSB

7/16 OSB

7/16" OSB

WALL TYPE

U

A

в

Ca,b

Da,b

BB a,b,c

CC a,b,c

DD a,b,c

1. HOLDOWNS PER PLAN

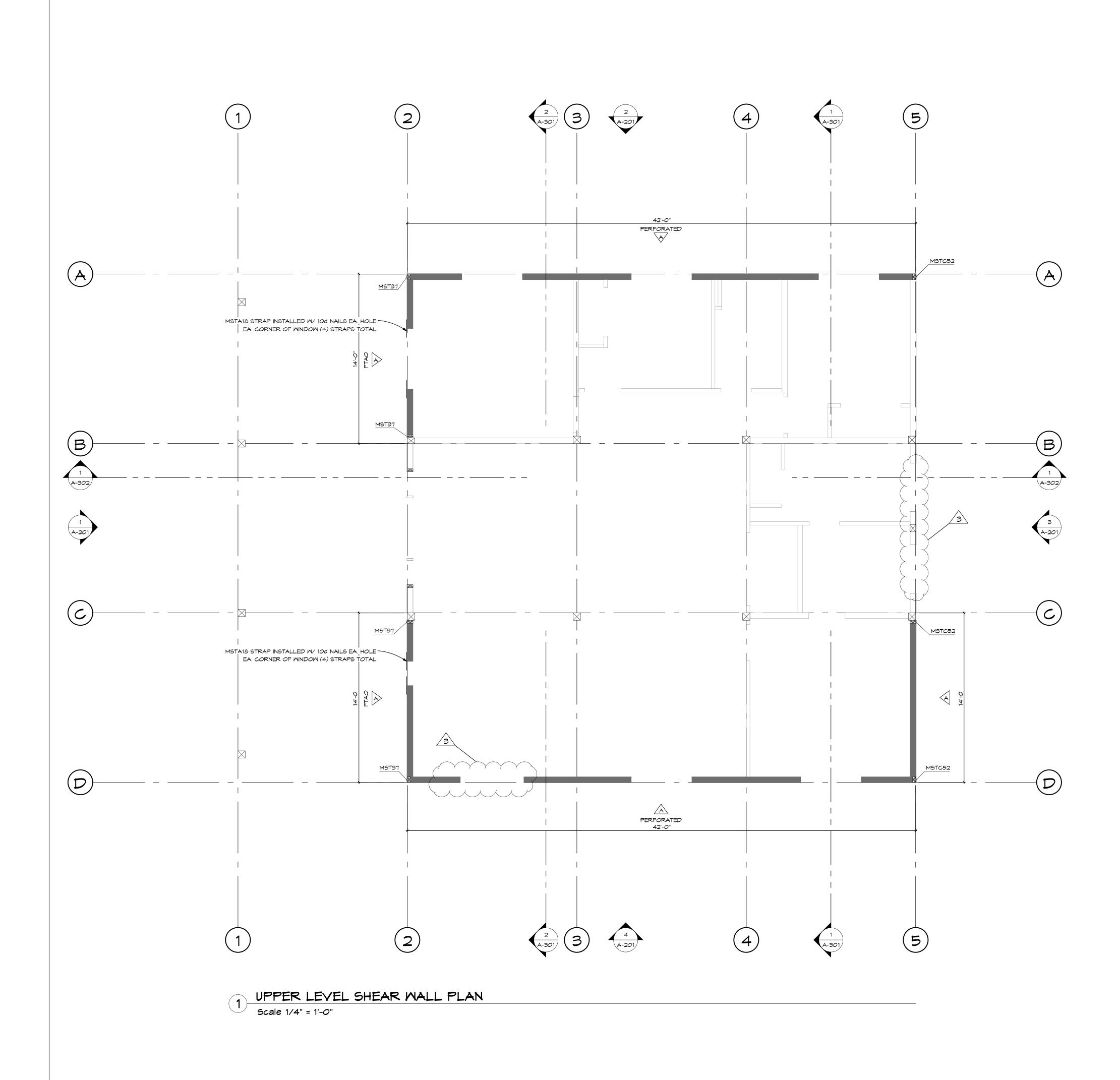
3. FOR DF-L STUDS ONLY.

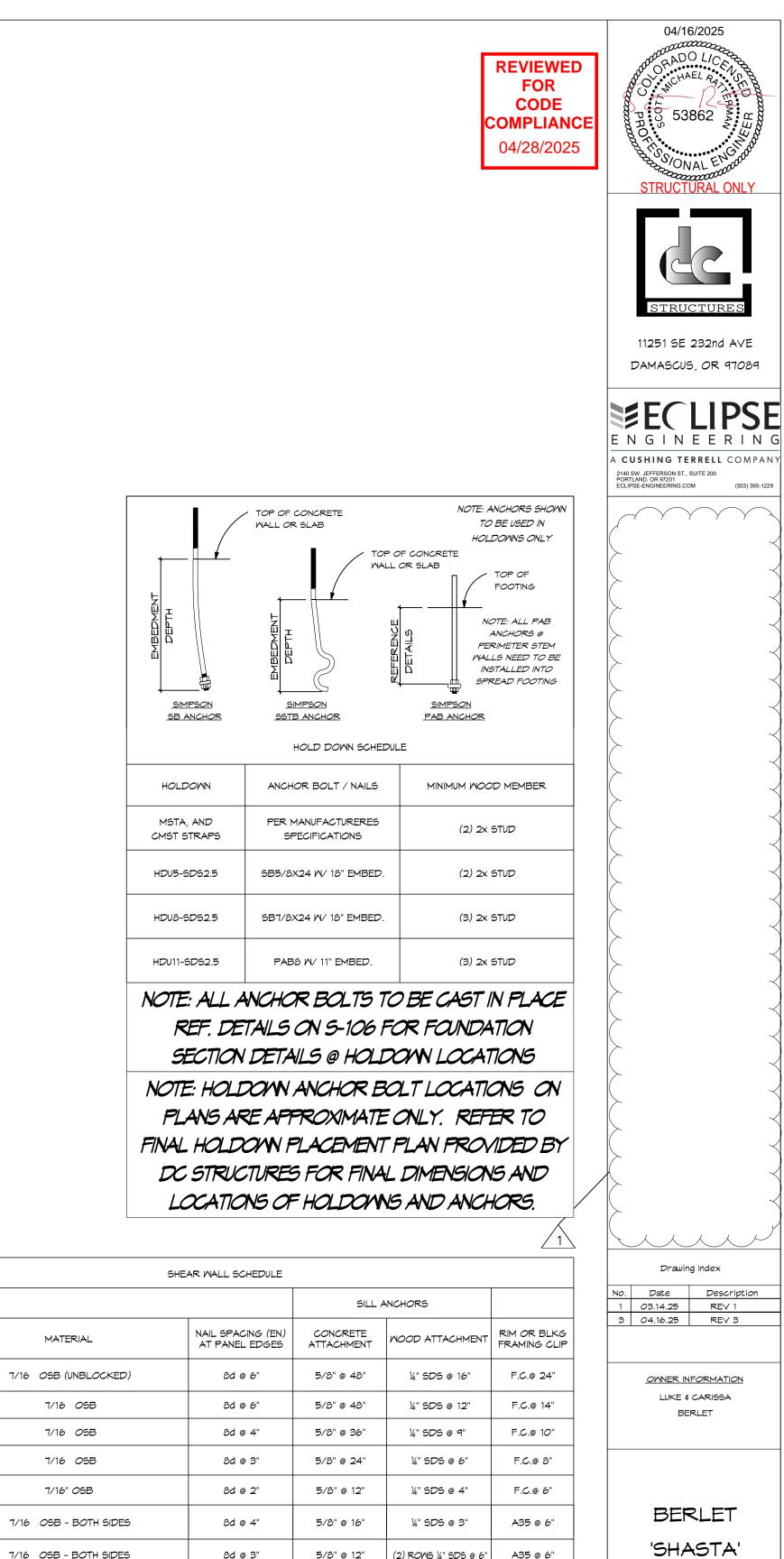
ACCEPTABLE.

12. FOOTNOTES:

SHALL BE STAGGERED

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В	7/16 OSB	8d @ 4"	5/8" @ 36"	14" SDS @ 9"	F.C.@ 10"
Са, b	7/16 OSB	8d @ 3"	5/8" @ 24"	14" SDS @ 6"	F.C.@ 8"
D a, b	7/16" <i>O</i> SB	8d @ 2"	5/8" @ 12"	¼" SDS @ 4"	F.C.@ 6"
BB a,b,c	7/16 OSB - BOTH SIDES	8d @ 4"	5/8" @ 16"	¼" SDS @ 3"	A35 @ 6"
CC a,b,c	7/16 OSB - BOTH SIDES	8d @ 3"	5/8" @ 12"	(2) ROWS ¼" SDS @ 6"	A35 @ 6"
DD a,b,c	7/16 OSB - BOTH SIDES	8d @ 2"	5/8" @ 8"	(2) ROWS ¼" SDS @ 6"	A35 @ 6"

SHEAR WALL PLAN GENERAL NOTES NOTES APPLY TO SHEET S-104 & S-105 ONLY.

MATERIAL

7/16 OSB

1. HOLDOWNS PER PLAN

WALL TYPE

U

A

- 2. STUDS SHALL BE SPACED AT 16" O.C. MAXIMUM (PLYWOOD MAY BE INSTALLED HORIZONTALLY OR VERTICALLY).
- 3. FOR DF-L STUDS ONLY. 4. MINIMUM THICKNESS OF SHEATHING SHALL BE 7/16". THICKER SHEATHING MAY BE USED. OSB OR CDX PLYWOOD ARE ACCEPTABLE.
- 5. FRAMING CLIP (F.C.) CAN BE A35, LTP4, RBC OR LS50, UNLESS NOTED OTHERWISE.
- 6. SPACE NAILS AT 12" O.C. ALONG INTERMEDIATE FRAMING MEMBERS. 7. ALL UNSUPPORTED PANEL EDGES SHALL BE BLOCKED AND EDGE-NAILED (EN), U.N.O.
- 8. USE ONLY COMMON OR BOX NAILS (GALVANIZED) FOR ALL PANEL AND SILL PLATE NAILING
- (GALVANIZED NAILS SHALL BE HOT-DIPPED OR DUMBLED). 9. ALL SHEAR PANELS ARE TO BE CONTINUOUS BETWEEN HORIZONTAL DIAPHRAGMS (ROOF TO FLOOR, FLOOR TO FLOOR, FLOOR TO FOUNDATION).
- 10. SILL ANCHOR BOLTS SHALL BE CAST-IN-PLACE AND SHALL HAVE A 7" MINIMUM EMBEDMENT INTO CONCRETE AND LOCATED NOT MORE THAN 12" OR LESS THAN 4" FROM EACH END. PROVIDE 3" SQ. \times 1/4" THICK PLATE WASHER AT ANCHOR BOLTS.
- 11. 3X MEMBERS MAY BE SUBSTITUTED WITH (2) 2X MEMBERS NAILED TOGETHER WITH 10d NAILS @ 4" O.C. 12. FOOTNOTES:
- a. STUDS AND/OR BLOCKING AT ADJOINING PANEL EDGES SHALL BE A 3X MINIMUM AND NAILS SHALL BE STAGGERED b. SILL PLATES SHALL BE 3X MINIMUM
- C. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS 6" O.C. OR LESS, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS. 13. ALL WALLS NOT LABELED ARE TO BE FRAMED PER TYPE "U".
- DRAWN BY RJF 04/15/2025

38980 MAIN STREET

MILNER, CO 80477

UPPER

LEVEL

SHEAR WALL

PLAN

5-105

