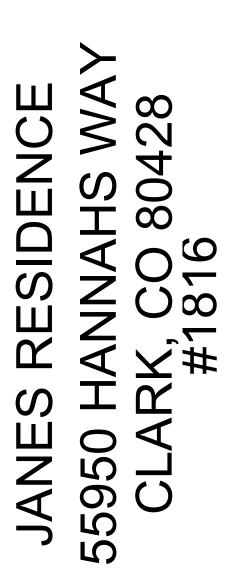


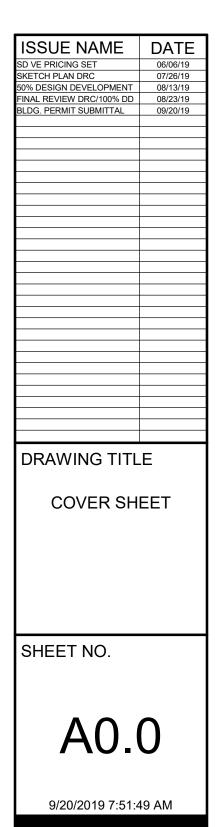
JANES RESIDENCE 55950 HANNAHS WAY CLARK, CO 80428 #1816

BLDG. PERMIT SUBMITTAL 09/20/19









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H TO, TOP OF HB HOSE BIBB T.O.D. TOP OF DECKING HGT HANDICAPPED T.O.W. TOP OF SLAB HGT HEIGHT TOP OF WALL HM HOULOW METAL TVP HP. HIGH POINT U HR HOUR UN.O. UNF UNF UNFINISHED UV UNF UNFINISHED I.D. INSIDE DIAMETER V INFO INFORMATION VCT VINVL COMPOSITE TILE J JANITOR CLOSET V VARIES JC JANITOR CLOSET W WASHER NETR JC JANITOR CLOSET W WASHER RNYER WD WOD WASHER RNYER WD WOD WASHER RNYER WD WOD WASHER RNYER WD WATER PROOF/WATERPROOFING			ТНК	THICK
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Independence HANDICAPPED TYP TYPICAL HDCP HEIGHT HEIGHT U HGT HOLQW METAL U U H.P. HIGH POINT U HR HOUR U.N.O. UNLESS NOTED OTHERWISE HXAC HEATING & VENTILATION U.N.O. UNLESS NOTED OTHERWISE HVAC HEATING & VENTILATION & AIR UNF UNFINISHED ID. INSIDE DIAMETER V VARIES INFO INFORMATION VCT VINTV COMPOSITE TILE J INSUL INSULATION VI.F. VENTLATOR J JOINT VP VENTILATOR JC JANITOR CLOSET W WASHER W/ WITH WITH W/O WITHOUT WO WO WASHER W/ W/ WITH WOOD WH WATER PROOF/WATERPROOF/MATER			T.O.S.	TOP OF SLAB
HM H,P. H,R HR HR HVACHOLLOW METAL HIGH POINTU U L UR HEATING & VENTILATION CONDITIONINGU U,N.O. UNF U,V.UNLESS NOTED OTHERWISE UNF UNFISHED UNF UNFISHED U,V.I.D. I.D. INFO INFO INFORMATION INSULINSIDE DIAMETER INFORMATION INSULATIONV V VCT VCT VENT. VENT. VENT. VENT. VENT. VENT. VENTILATOR VENT. VENT. VENT. VENTILATOR VI.F. VPJ.J.C. J.C. J.C. J.C. J.C. J.C. J.C. J.C. J.C. J.C.JANITOR CLOSET JOINTW V V W VENT VENT VENT VENT VENTILATOR V.I.F. VPW VENT PIPEJ.C. V.T.F. V.C. V.T.F. VENT PIPEJANITOR CLOSET VPW V/PW WASHER W/ W/THHOUT WCC WASHER.DRYER WD WATER HEATER W/P.WATER HEATER W/P.	HDCP	HANDICAPPED		
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JT JOINT W W WASHER W/ WITH W/O WITHOUT WC WATER CLOSET W/D WASHER.DRYER WD WOOD WH WATER HEATER W.P. WATERPROOF/WATERPROOFING	<u>J</u>			
W WASHER W/ WITH W/O WITHOUT WC WATER CLOSET W/D WASHER.DRYER WD WOOD WH WATER HEATER W.P. WATERPROOF/WATERPROOFING			W	
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WHWATER HEATERW.P.WATERPROOF/WATERPROOFING			W/D	WASHER.DRYER
			WH	WATER HEATER

FULL WARRANTIES.

6. THE CONTRACTOR SHALL CONFINE HIS/HER OPERATIONS ON THE SITE TO AREAS PERMITTED BY THE OWNER. 7. THE JOB SITE SHALL BE MAINTAINED IN A CLEAN, ORDERLY CONDITION, FREE OF DEBRIS AND LITTER AND SHALL NOT BE UNREASONABLY ENCUMBERED WITH ANY MATERIALS OR EQUIPMENT. EACH SUB-CONTRACTOR IMMEDIATELY UPON COMPLETION OF EACH PHASE OF HIS/HER WORK SHALL REMOVE ALL TRASH AND DEBRIS AS OF RESULT OF HIS/HER OPERATION.

DETERIORATION. FAILURE TO PROTECT MATERIALS MAY BE CAUSE FOR REJECTION OF WORK. 9. THE CONTRACTOR SHALL DO ALL CUTTING, FITTING OR PATCHING OF HIS/HER WORK THAT MAY BE REQUIRED TO MAKE ITS SEVERAL PARTS FIT TOGETHER PROPERLY AND SHALL NOT ENDANGER ANY OTHER WORK BY CUTTING, EXCAVATING OR OTHERWISE ALTERING THE TOTAL WORK OR ANY OTHER PART OF IT. ALL PATCHING. REPAIRING AND REPLACING OF MATERIALS AND SURFACES CUT OR DAMAGED IN EXECUTION OF WORK SHALL BE DONE WITH APPLICABLE MATERIALS SO THAT SURFACES REPLACED WILL, UPON COMPLETION, MATCH SURROUNDING SIMILAR SURFACES. 10. NO PORTION OF THE WORK REQUIRING A SHOP DRAWING OR SAMPLE SUBMISSION SHALL BE COMMENCED UNTIL THE

11. DIMENSIONS: OTHERWISE.

13. WHERE LARGER STUDS OR FURRING ARE REQUIRED TO COVER PIPING AND CONDUITS, THE LARGER STUD SIZE OR FURRING SHALL BE EXTENDED THE FULL SURFACE OF THE WALL WIDTH AND LENGTH WHERE THE FURRING OCCURS. 14. PROVIDE ALL ACCESS PANELS AS REQUIRED BY GOVERNING CODES TO ALL CONCEALED SPACES, VOIDS, ATTICS, ETC. VERIFY TYPE REQUIRED WITH ARCHITECT PRIOR TO INSTALLATION.

CODES.

18. FIREBLOCKS AND DRAFT STOPS SHALL BE PROVIDED AS REQUIRED BY GOVERNING CODES. 19. THE BUILDING THERMAL ENVELOPE SHALL BE DURABLY SEALED TO LIMIT AIR INFILTRATION PER I.E.C.C. R402.4 20. RECESSED LIGHT SHALL BE SEALED PER I.E.C.C. R402.4.5.

GENERAL NOTES:

1. ALL CONSTRUCTION AND MATERIALS SHALL BE AS SPECIFIED AND IN ACCORDANCE WITH ALL APPLICABLE CODES, ORDINANCES, LAWS, PERMITS AND THE CONTRACT DOCUMENTS.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURATE PLACEMENT OF ALL NEW CONSTRUCTION ON THE SITE. ALL MATERIALS AND COMPONENTS SHALL BE INSTALLED PER MANUFACTURES INSTRUCTIONS AND SPECIFICATIONS WITH

3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. SHOULD A DISCREPANCY APPEAR IN THE CONTRACT DOCUMENTS, OR BETWEEN THE CONTRACT DOCUMENTS AND EXISTING CONDITIONS, NOTIFY THE ARCHITECT AT ONCE FOR INSTRUCTION ON HOW TO PROCEED.

4. CHANGES FROM THE PLANS MADE WITHOUT CONSENT OF THE ARCHITECT ARE UNAUTHORIZED AND SHALL RELIEVE THE ARCHITECT OF RESPONSIBILITY FOR ALL CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

5. SHOULD A CONFLICT OCCUR IN OR BETWEEN DRAWINGS AND SPECIFICATIONS, THE SPECIFICATIONS SHALL TAKE PRECEDENCE UNLESS A WRITTEN DECISION FROM THE ARCHITECT HAS BEEN OBTAINED WHICH DESCRIBES A CLARIFICATION OR ALTERNATE METHOD AND/OR MATERIALS.

8. ALL MATERIALS STORED ON THE SITE SHALL BE PROPERLY STACKED AND PROTECTED TO PREVENT DAMAGE AND

SUBMISSION HAS BEEN REVIEWED BY THE ARCHITECT. ALL SUCH PORTIONS OF THE WORK SHALL BE IN ACCORDANCE WITH REVIEWED SHOP DRAWINGS AND SAMPLES.

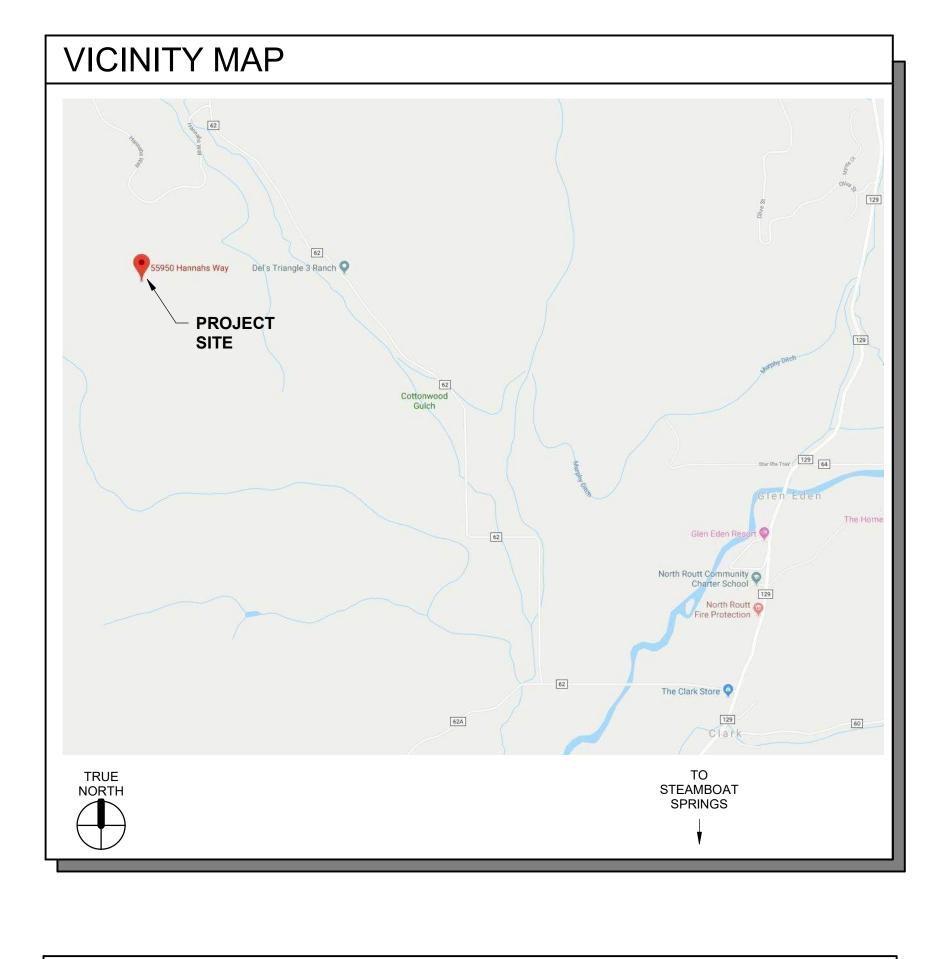
A) ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE OF DRAWINGS. B) ALL DIMENSIONS ARE TO FACE OF STUD, FACE OF C.M.U. OR FACE OF CONCRETE U.N.O. C) CEILING HEIGHT DIMENSIONS ARE FROM FINISHED FLOOR TO FACE OF FINISH CEILING MATERIALS UNLESS NOTED

12. CONTRACTOR TO PROVIDE ALL NECESSARY BLOCKING, BACKING AND FRAMING FOR LIGHT FIXTURES, ELECTRICAL UNITS, A.C. EQUIPMENT, RECESSED ITEMS AND ALL OTHER ITEMS AS REQUIRED.

15. PROVIDE ACCESS AND MINIMUM VENTILATION REQUIREMENTS TO ALL CRAWL SPACES AS REQUIRED BY GOVERNING

16. INSTALL TEMPERED GLASS AS REQUIRED BY GOVERNING CODES.

17. STRUCTURAL AND FIRE RESISTIVE INTEGRITY SHALL BE MAINTAINED AS REQUIRED BY GOVERNING CODES.



DRAWING SYMBOLS

100'-0" T.O. PLYWOOD	FLOOR OR SPOT ELEVATION
100	DOOR TAG
100	WINDOW TAG
	WALL TAG
CRPT CRPT	FLOOR FINISH
\frown	PROPOSED CONTOUR
/	EXISTING CONTOUR

GRAPHIC KEY TO MATERIALS <u>EARTH</u> COMPACTED FILL POROUS FILL

<u>CONCRETE</u> CAST GROUT LIGHWEIGHT CONCRETE <u>MASONRY</u> CONCRETE BLOCK BRICK

STONE

PROJECT TEAM

<u>OWNER</u> JAN & ANDY JANES 642 S. BODIN STREET HINSDALE, IL 60521

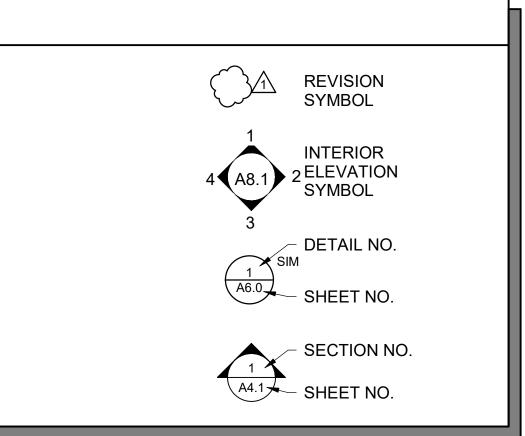
ARCHITECT

630.733.6155

VERTICAL ARTS, INC. 690 MARKETPLACE PLAZA SUITE 1 STEAMBOAT SPRINGS, CO 80487 CONTACT: CHANDLER DEIMUND 970.871.0056

CONTRACTOR FAIR & SQUARE CONSTRUCTION 2673 JACOB CIRCLE UNIT 700 STEAMBOAT SPRINGS, CO 80487 CONTACT: RON DAVIES 970.879.7725

STRUCTURAL ENGINEER ANTHEM STRUCTURAL ENGINEERS 2155 RESORT DRIVE SUITE 245 STEAMBOAT SPRINGS. CO 80487 CONTACT: CHARLIE ROOS, PE 970.300.3338



INSULATION	
	BATT
	RIGID
WOOD	
	FINISH
	ROUGH
	BLOCKING
	GLU-LAM
	PLYWOOD
<u>METAL</u>	STEEL

SUSTAINABILITY CONSULTANT SUSTAINABLY BUILT, LLC 1600 38TH. STREET, SUITE #101 BOULDER, COLORADO 80301 CONCTACT: ANDY MAZAL 303.447.0237 LIGHTING DESIGN

LS GROUP 931 SANTA FE DRIVE, SUITE 200 DENVER, COLORADO 80204 CONTACT: ELISE STREEB 303.573.0059

LANDSCAPE ARCHITECT VERTICAL ARTS, INC.

690 MARKETPLACE PLAZA SUITE 1 STEAMBOAT SPRINGS, CO 80487 CONTACT: MITCH REWOLD 970.871.0056

GEOTECHNICAL ENGINEER WESTERN SLOPE GEOTECH, INC. P.O. BOX 771330 STEAMBOAT SPRINGS, COLORADO 80477 CONTACT: HAROLD SCHLICHT, PE 970.875.4075

SHEET INDEX

	ARCHITECTURAL	
	A0.0	COVER SHEET
	A0.1	GENERAL INFORMATION SHEET
	A0.2	AREA PLANS
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	L-1	TREE REMOVAL AND EROSION CONTROL PLAN
	L-2	LANDSCAPE PLAN
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	SP-1	SITE AND GRADING PLAN
	SP-2	MANAGEMENT PLAN
	A2.1	LOWER LEVEL FLOOR PLAN
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	A2.3	UPPER LEVEL FLOOR PLAN
	A2.4	ROOF PLAN
	A2.5	BUILDING HEIGHT CALCULATION
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	A9.4	DETAILS
	A9.5	DETAILS
	A9.6	DETAILS
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	S 0.02	3D VIEWS
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	S 1.03	UPPER LEVEL & LOW ROOF FRAMING PLAN
	S 1.04	HIGH ROOF FRAMING PLAN
	S 5.01	TYPICAL DETAILS
	S 5.07	

TYPICAL DETAILS DETAILS SCHEDULES

S 5.02

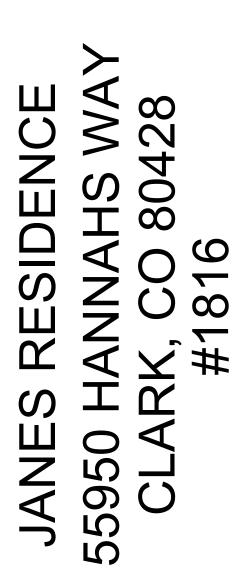
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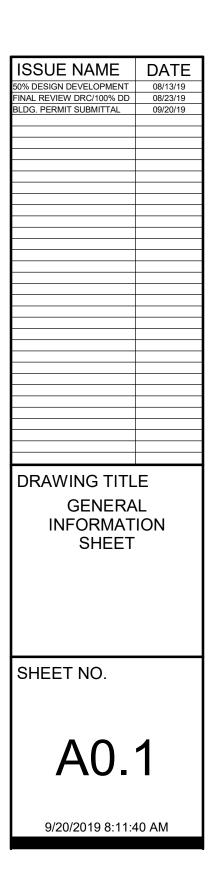
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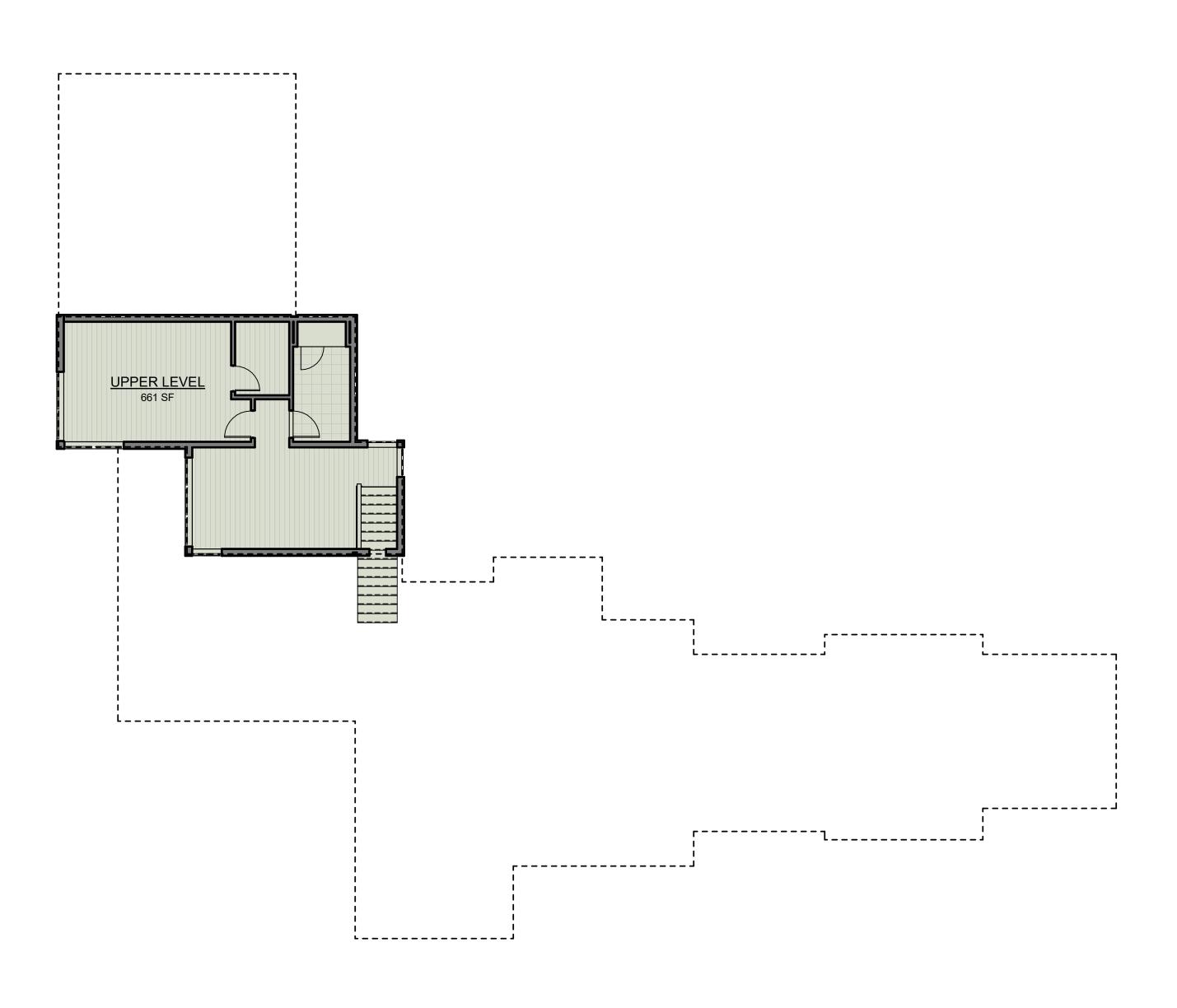
CODE ANALYSIS

TYPE OF CONSTRUCTION	TYPE VB	
OCCUPANCY	RESIDENTIAL GROUP: R-3	
ZONING	AF - AGRICULTURE AND FORESTRY	
BUILDING HEIGHT	ROUTT COUNTY ZONING: • STRUCTURE HEIGHT: 40' - 0" MURPHY - LARSON RANCH DESIGN GUIDELINES: • OVERALL BUILDING HEIGHT: 28' - 0"	
ACTUAL BUILDING HEIGHT	28' - 0" (REFER SHEET A2.5)	
BUILDING CODES	 2015 INTERNATIONAL RESIDENTIAL CODE (I.R.C.) 2015 INTERNATIONAL ENERGY CODE (I.E.C.C.) ALL ROUTT COUNTY REGIONAL BUILDING DEPARTMENT'S CODE AMENDMENTS 	
DATUM		
MAIN LEVEL 100'-0" (PROJECT) = 7651.56' (U.S.G.S.)		









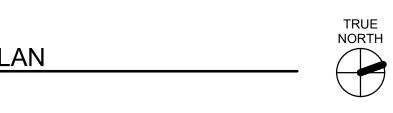
 $\bigcirc \frac{\text{UPPER LEVEL AREA PLAN}}{1/8" = 1'-0"}$

GROSS AREA CALCULATIONS MAIN LEVEL UPPER LEVEL LOWER LEVEL TOTAL GARAGE TOTAL

GRAND TOTAL



GARAGE LOWER LEVEL MAIN LEVEL UPPER LEVEL

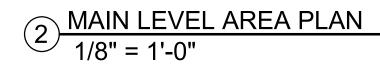


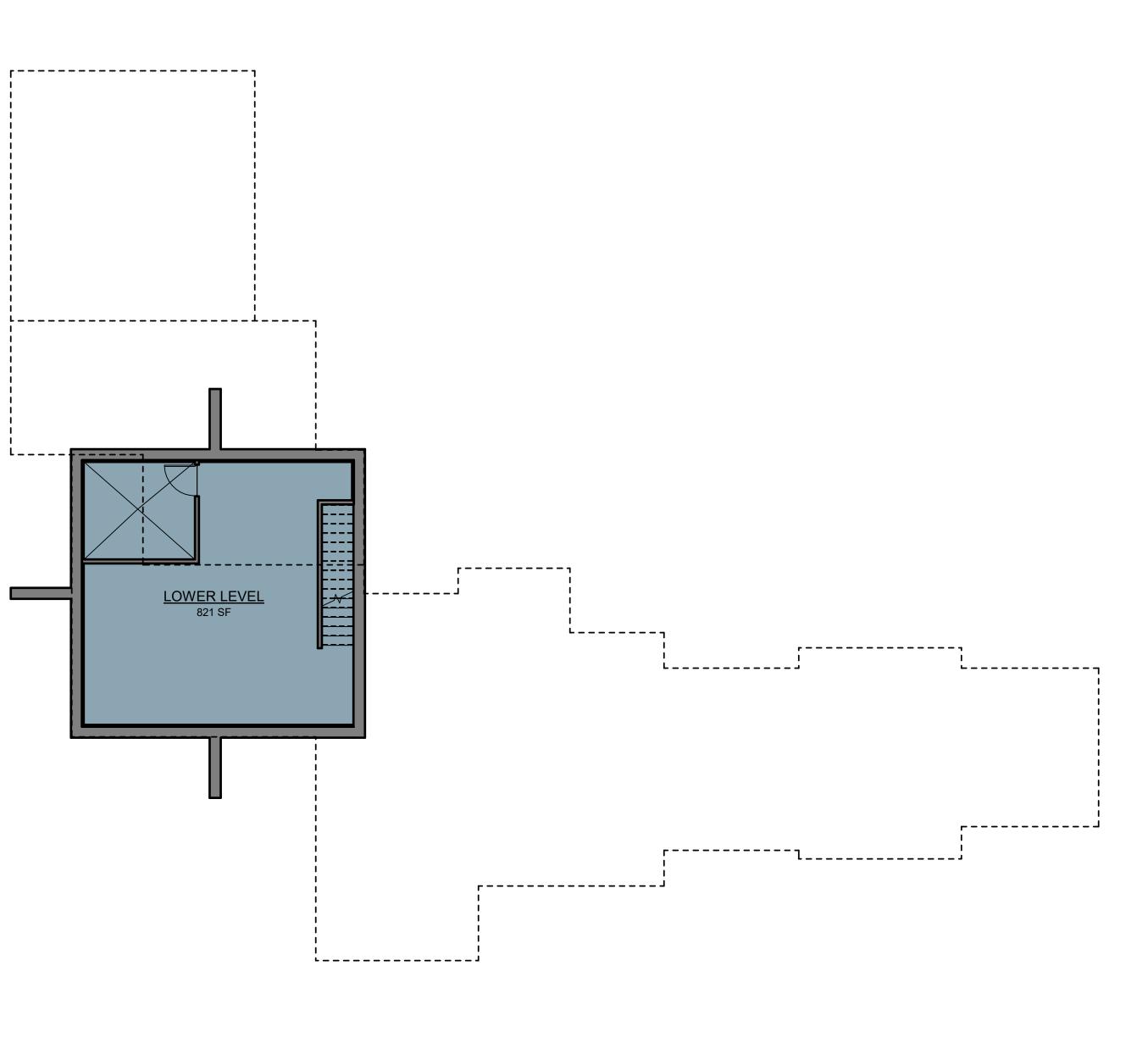
2,607 SF
661 SF
821 SF
4,089 SF
985 SF
985 SF 985 SF

GROSS FLOOR AREA LEGEND



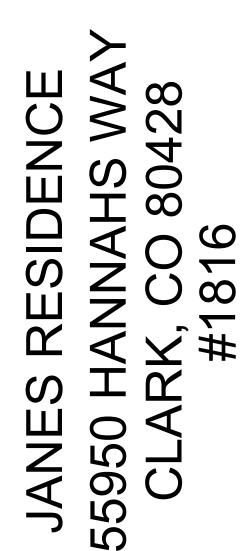
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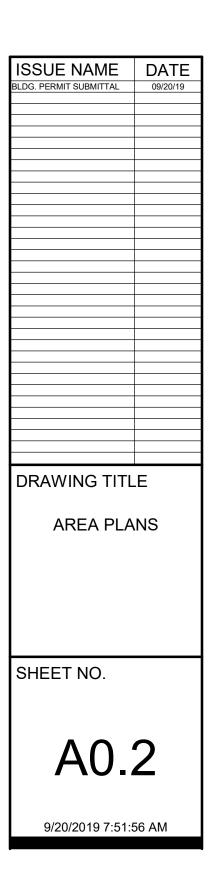




1 LOWER LEVEL AREA PLAN 1/8" = 1'-0"

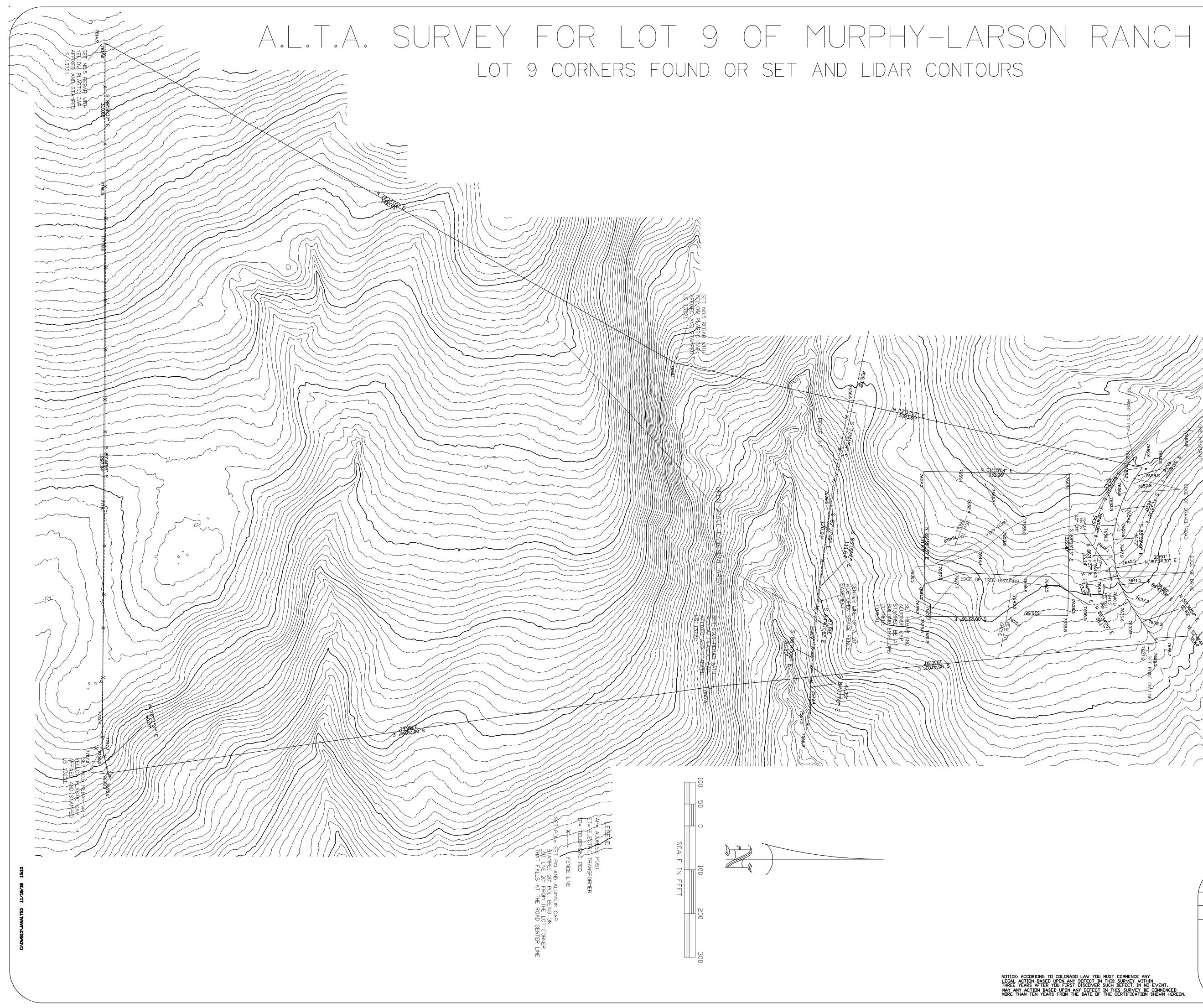


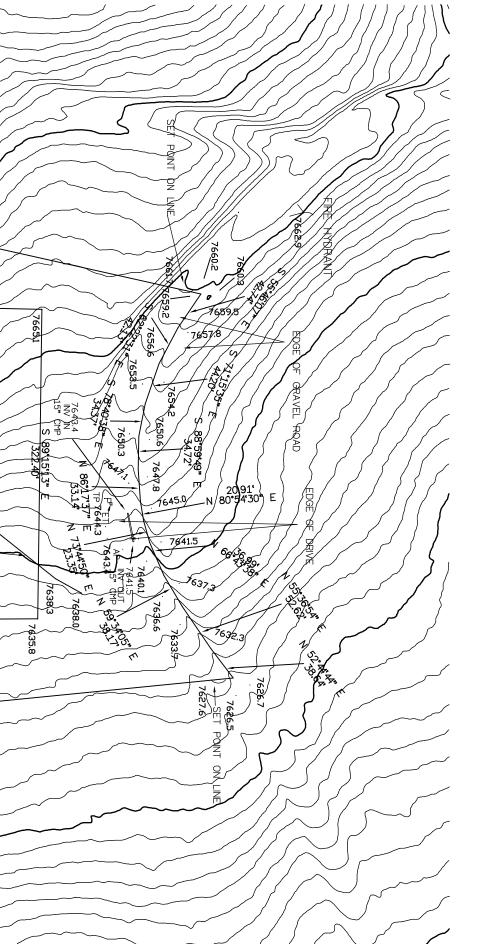












LAND SURVEY PLAT OF LOT 9 OF MURPHY-LARSEN RANCH LOCATED IN THE SE1/4 OF SECTION 18 AND IN THE N1/2N1/2 OF SECTION 19, T9N, R85W, OF THE 6TH P.M., ROUTT COUNTY, COLORADO.

Revised	Description		
Date:	11-15-2018	JANICE J. JANES AND T. ANDREW JANES	
/ Drawn:	JANALTS3.DWG		
File:	JANASPN.WO ASPNGRV.WO	A.L.T.A. SURVEY AND LAND SURVEY PLAT OF LOT 9 OF MURPHY-LARSON RANCH, ROUTT COUNTY, COLORADO.	
Scale:	1"=100"		
		A PROFESSIONAL LA P.O. Box 775008 S	& D, Inc. AND SURVEYING & PLANNING CO. teamboat Springs, Co. 80477 803) 879-2715
		Sheet No.	Project No. 4894-1

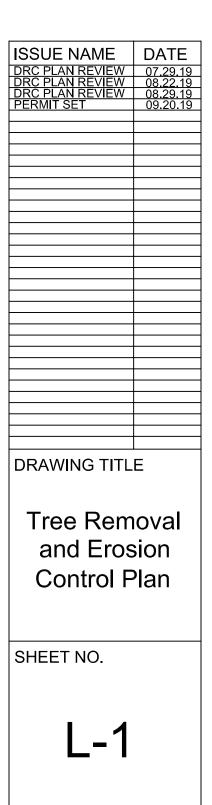
NOTICE[,] ACCORDING TO COLORADO LAW YOU MUST COMMENCE ANY LEGAL ACTION BASED UPON ANY DEFECT IN THIS SURVEY WITHIN THREE YEARS AFTER YOU FIRST DISCOVER SUCH DEFECT. IN NO EVENT, MAY ANY ACTION BASED UPON ANY DEFECT IN THIS SURVEY BE COMMENCED MORE THAN TEN YEARS FROM THE DATE OF THE CERTIFICATION SHOWN HEREON.

Sheet No.







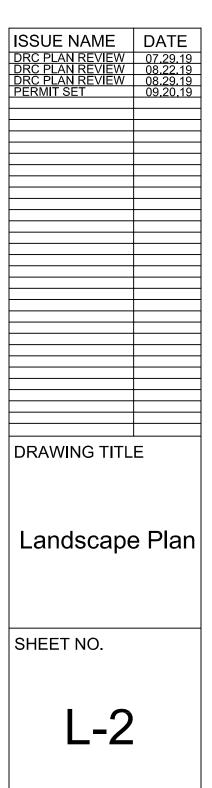




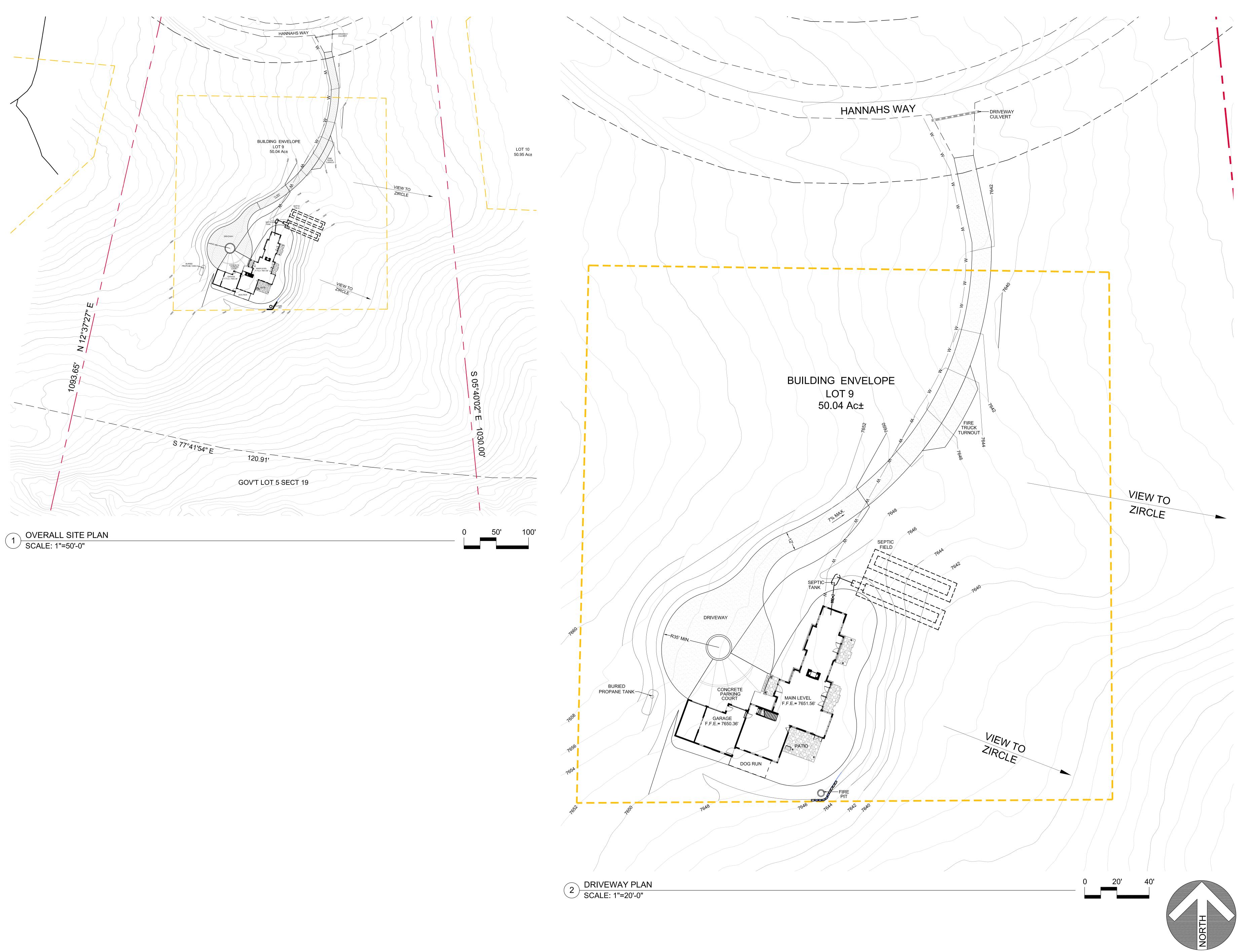






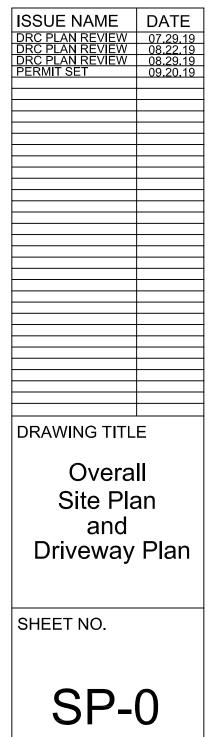


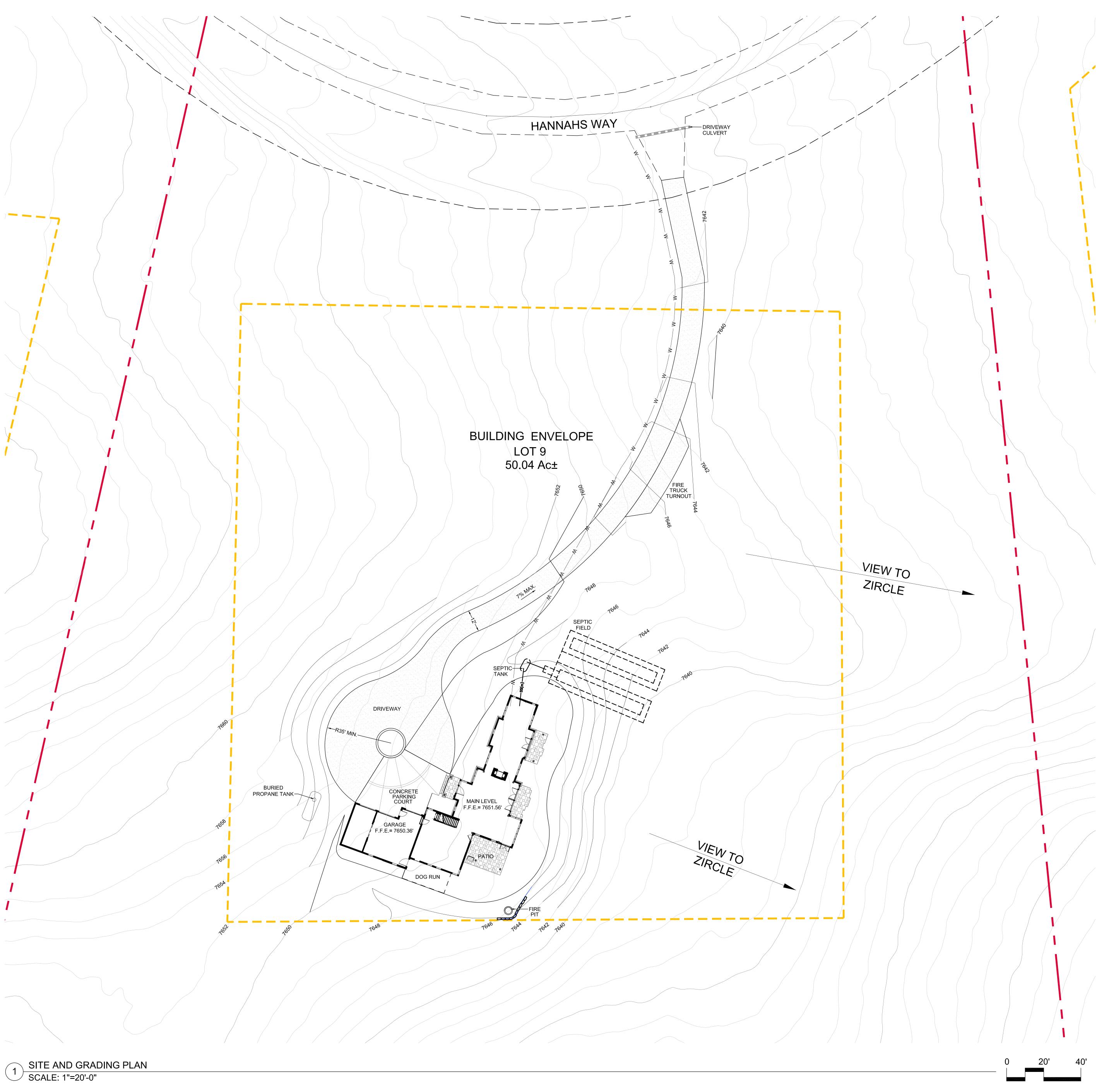






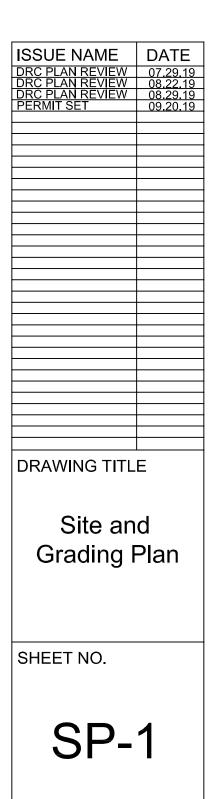






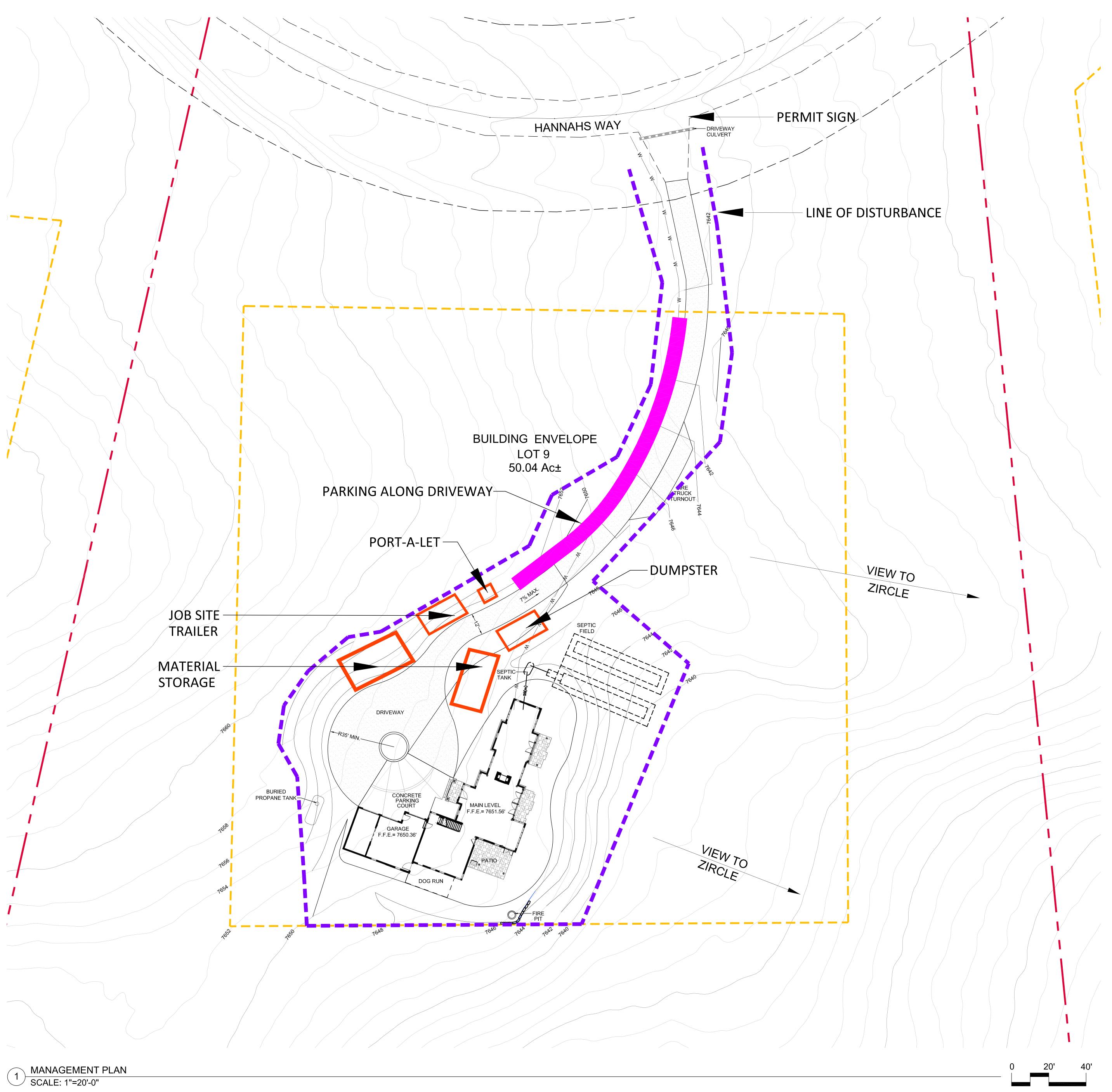






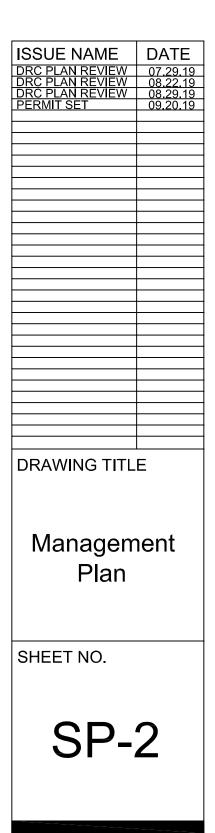


Start date: October 1st, 2019 Completion date: April 1st, 2021 (18 months after start date) Occupancy date: May 1st, 2021

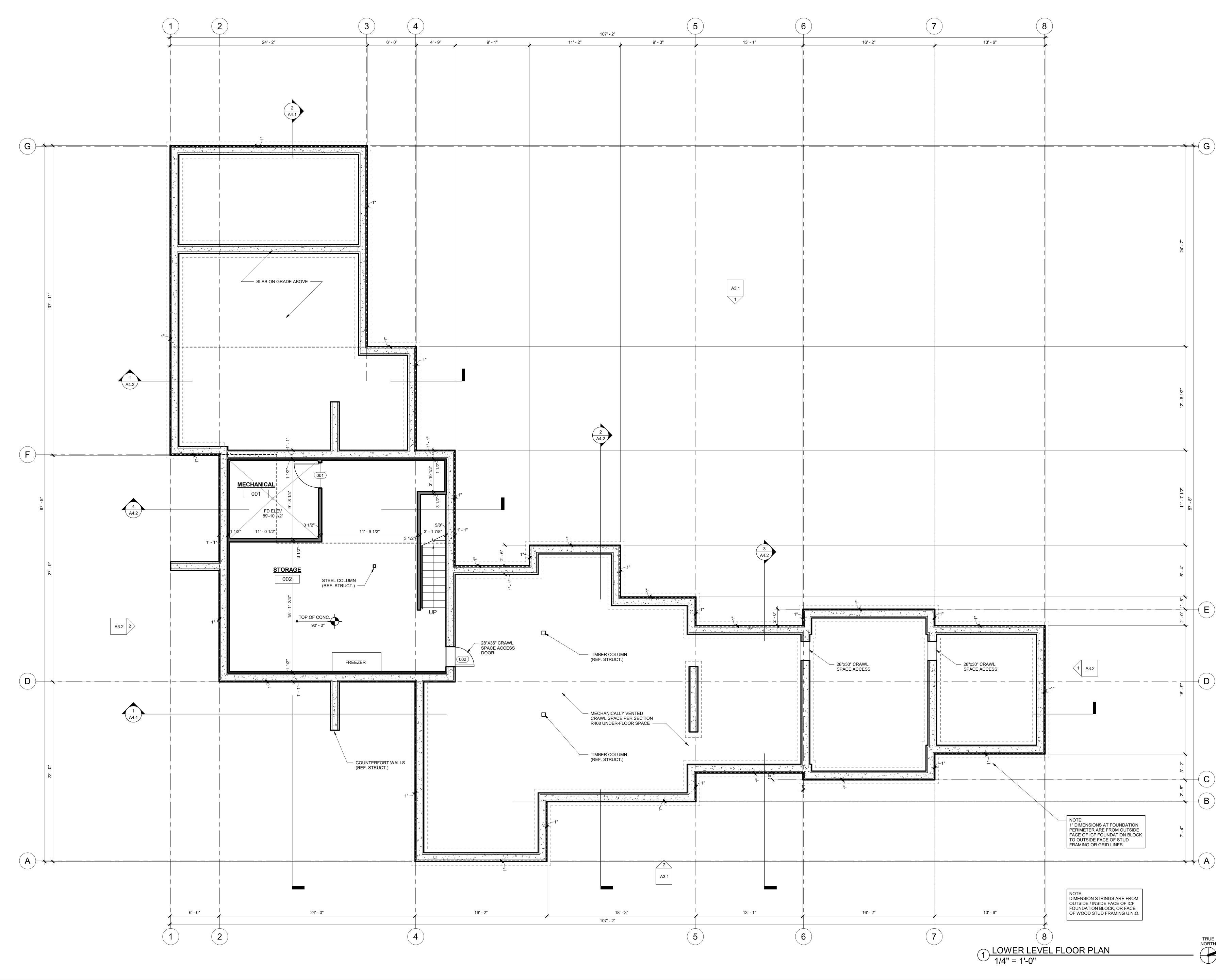




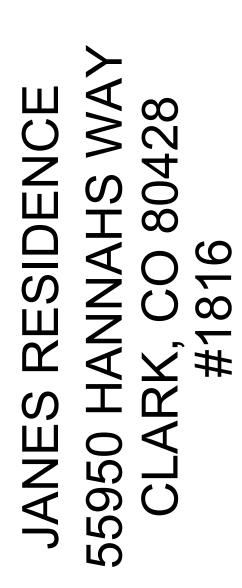


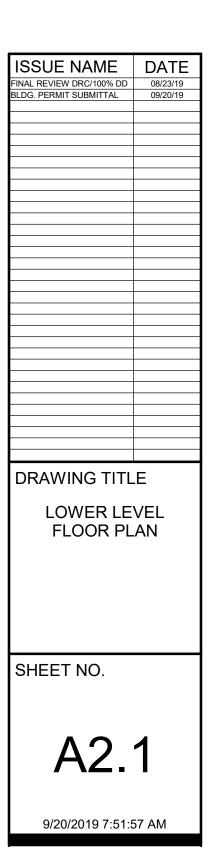












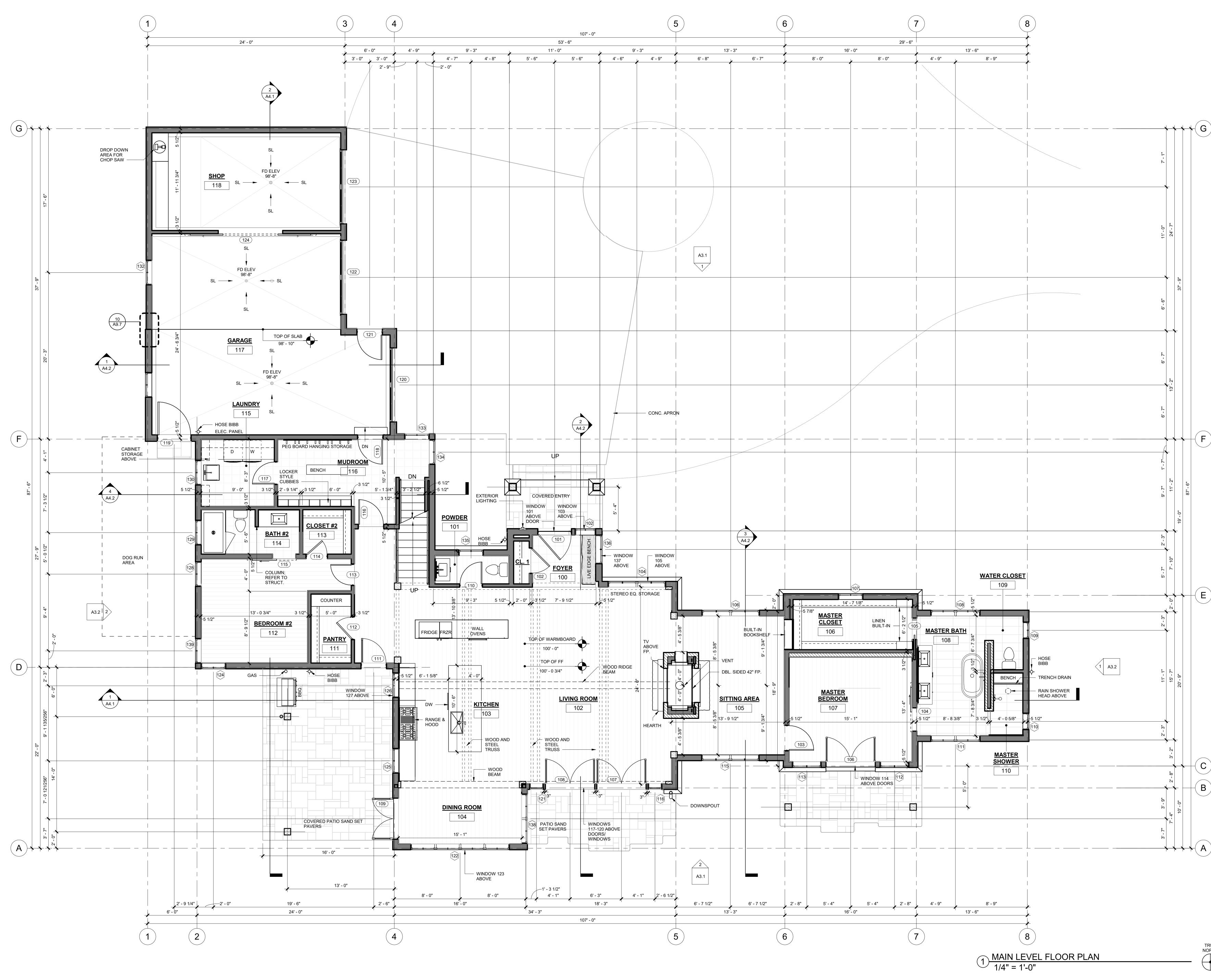


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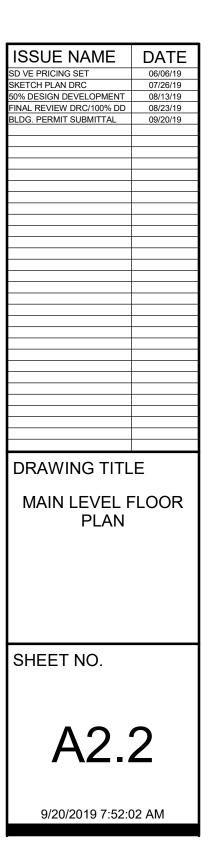


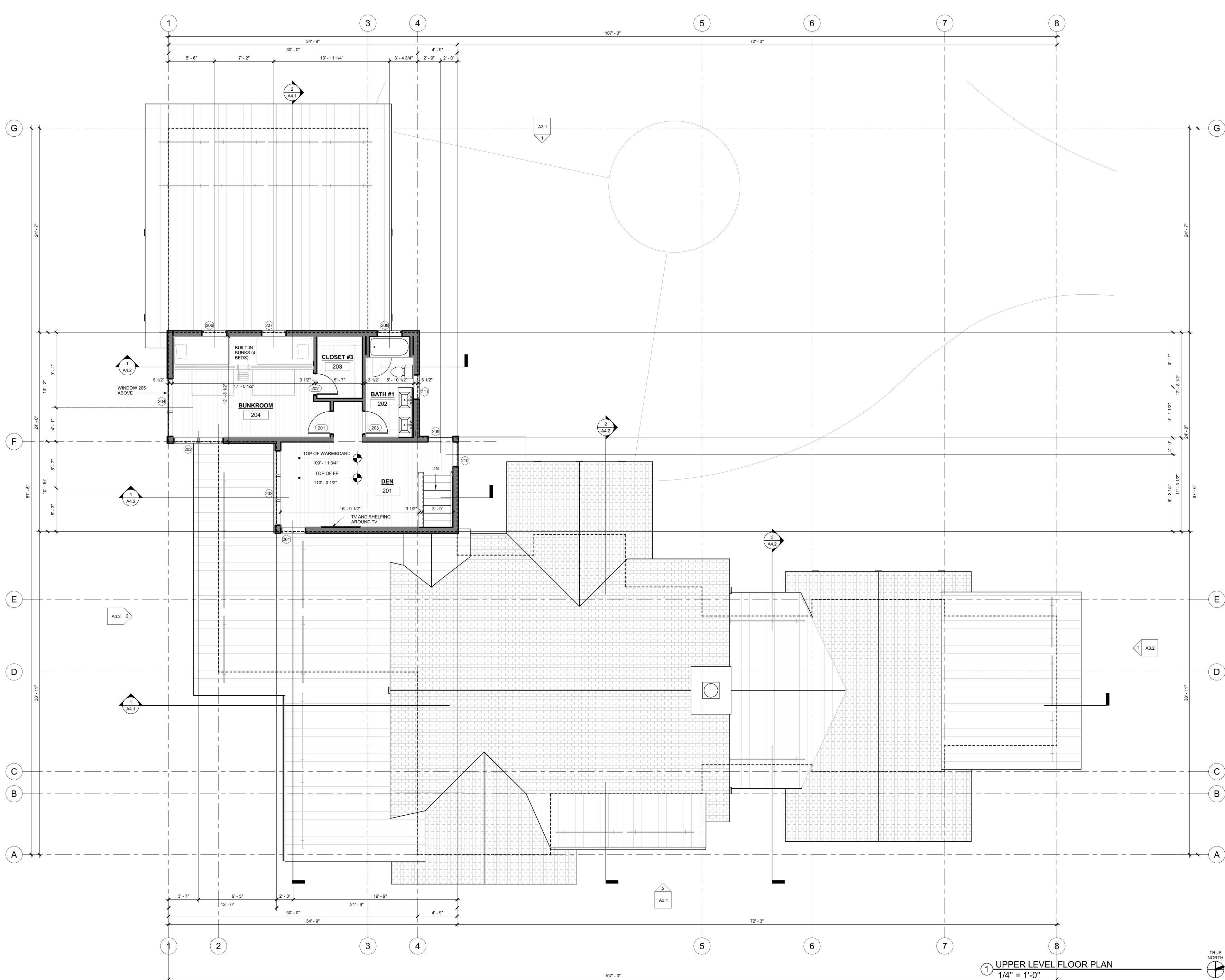


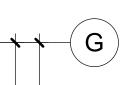
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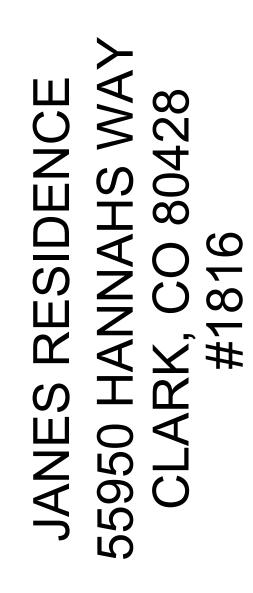


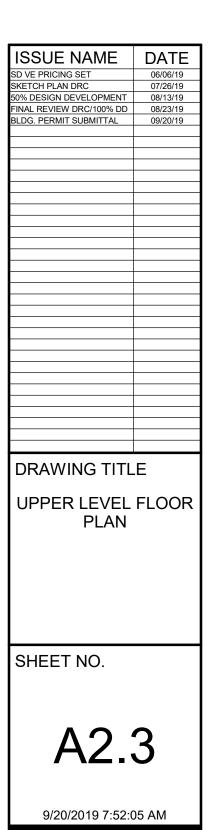






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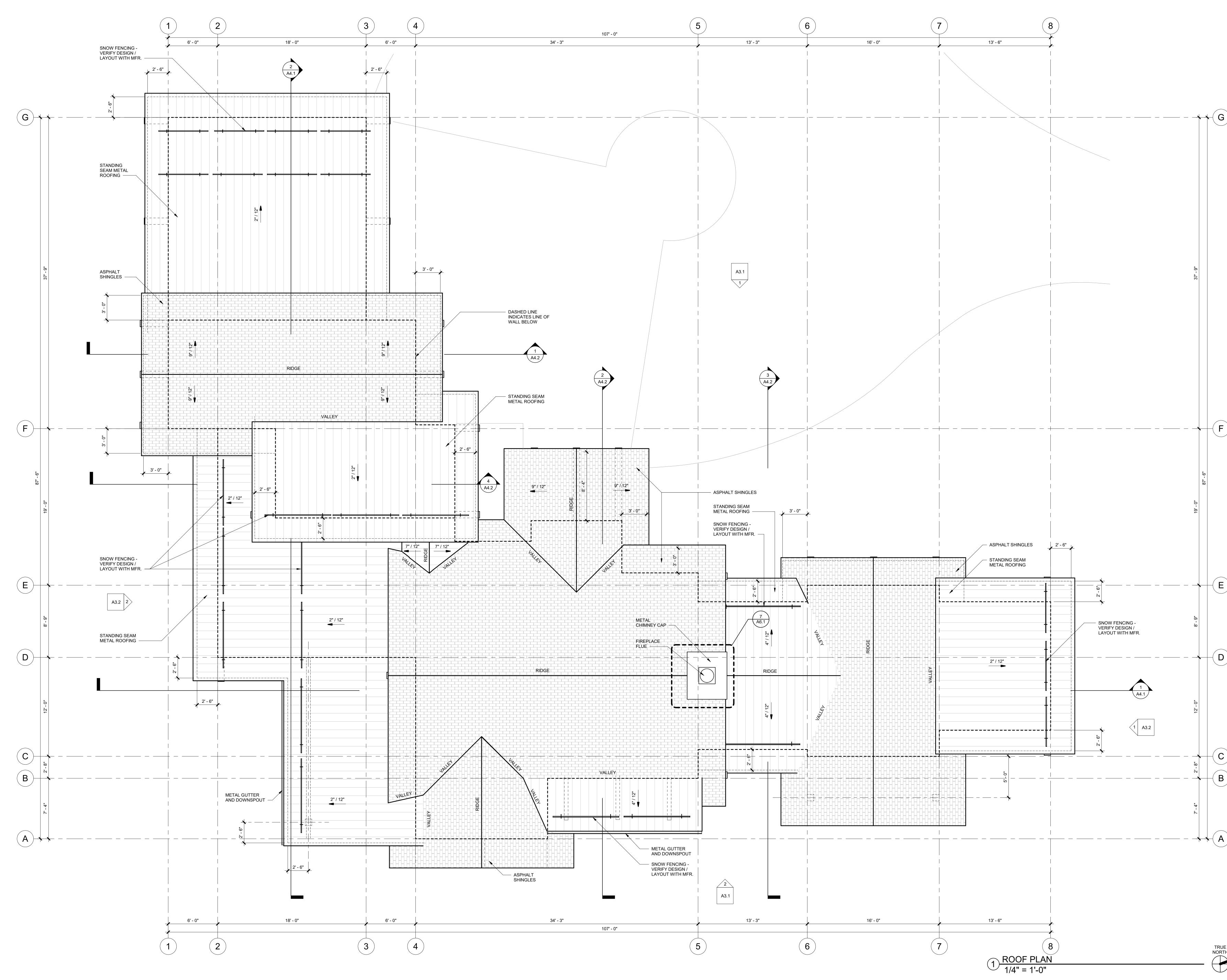


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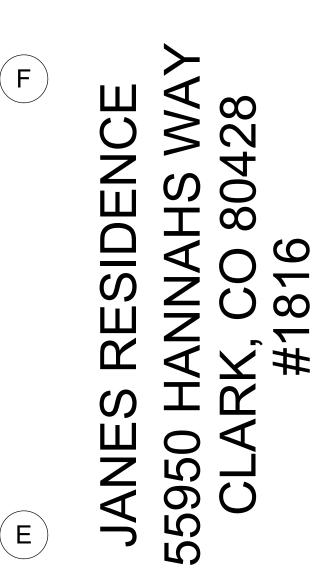
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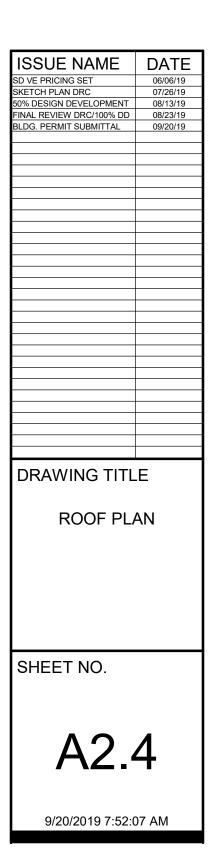
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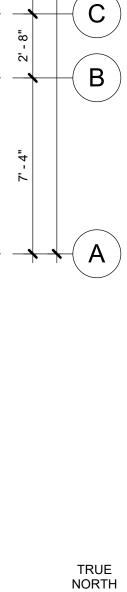
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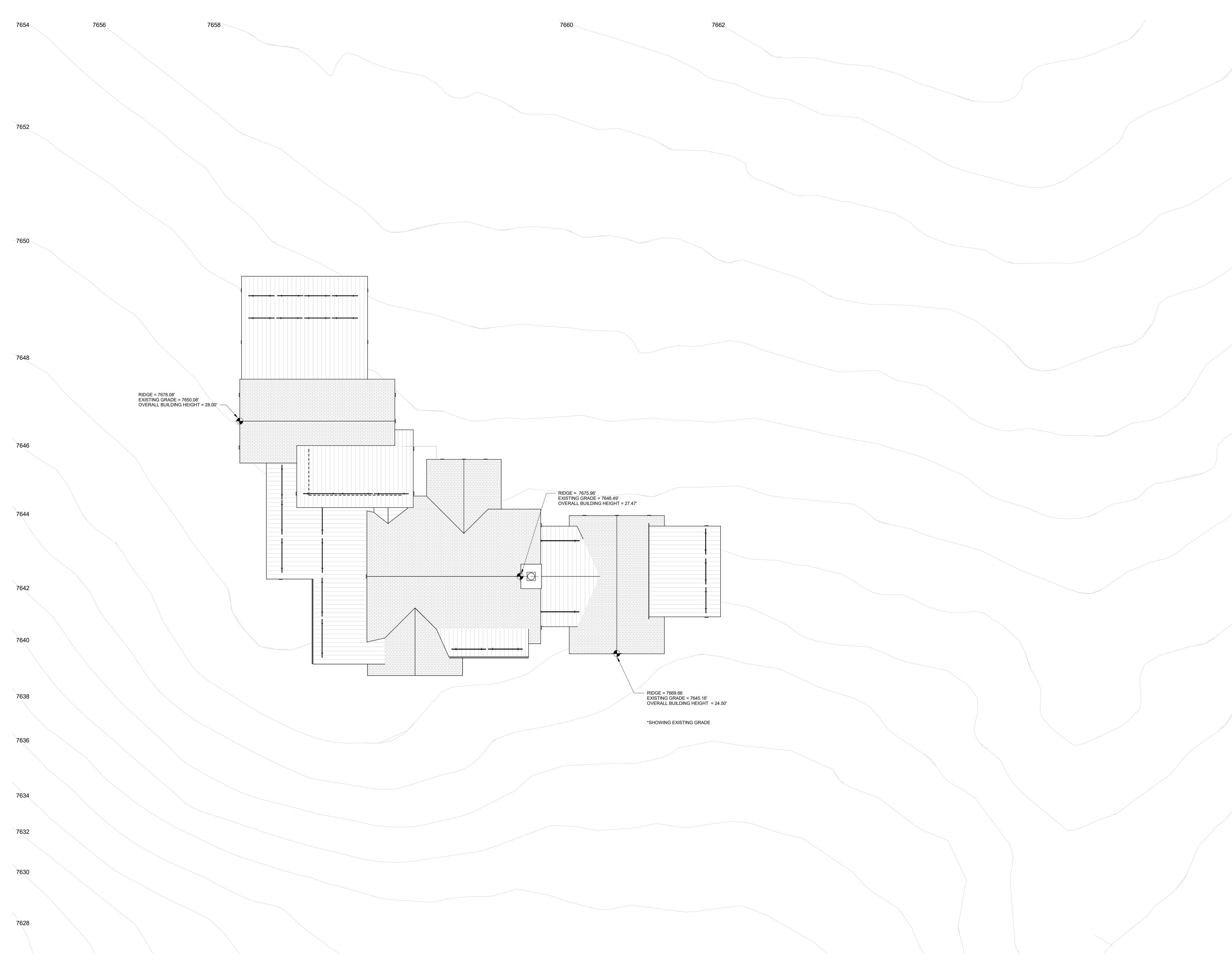




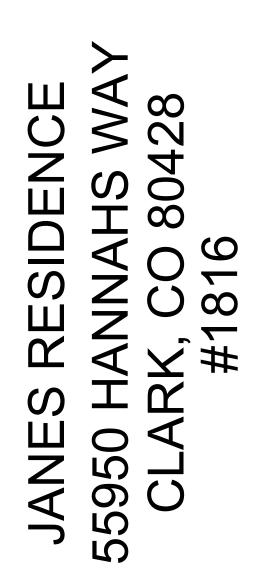


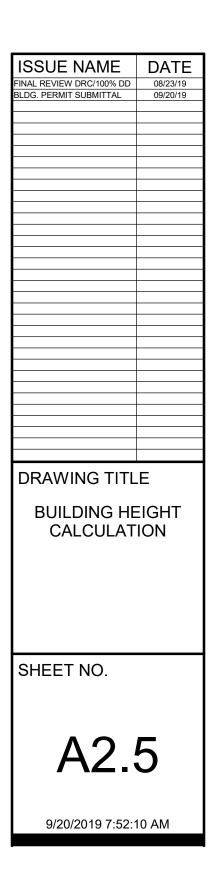
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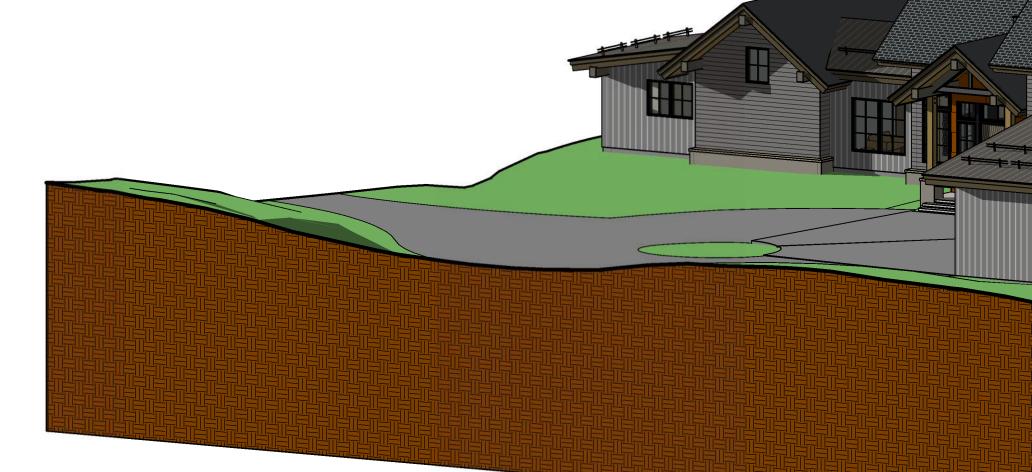




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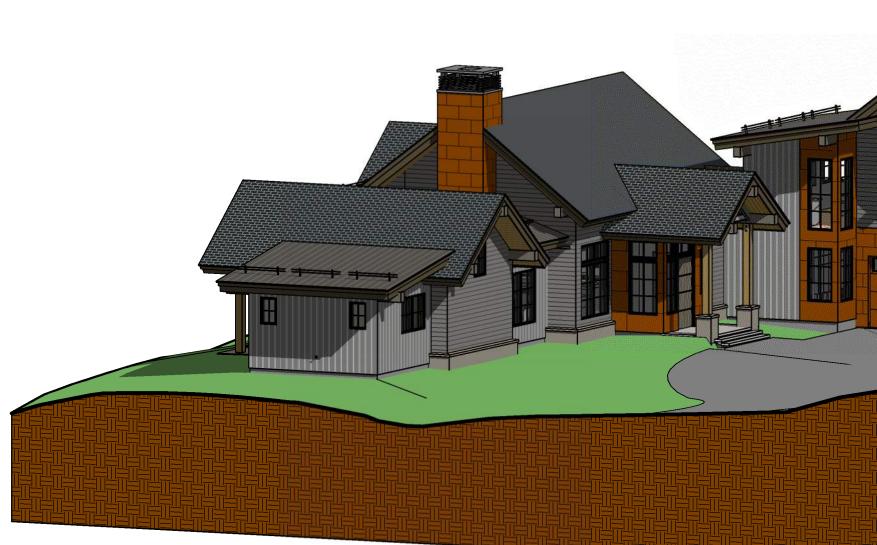
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4 PERSPECTIVE - 4

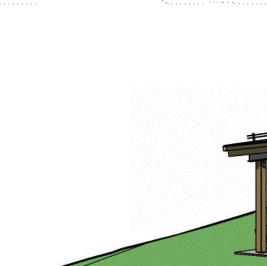


3 PERSPECTIVE - 3

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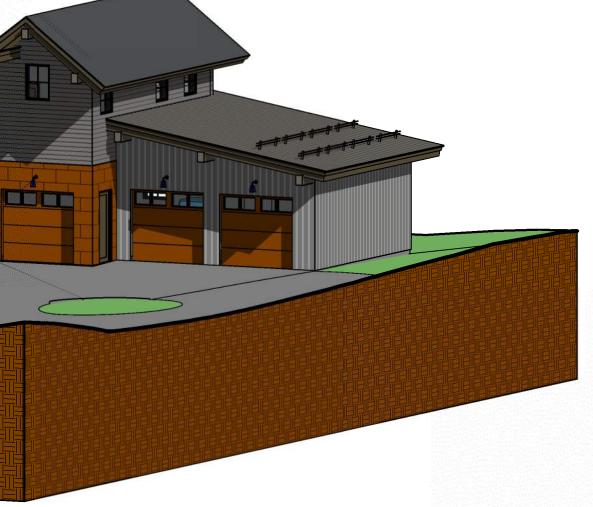
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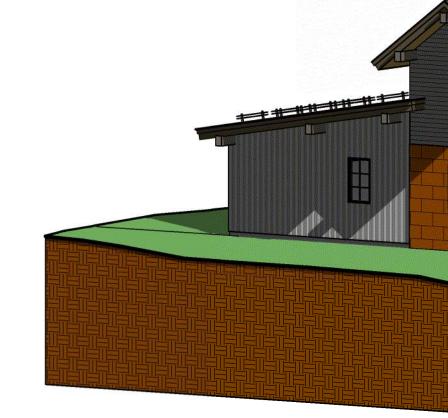
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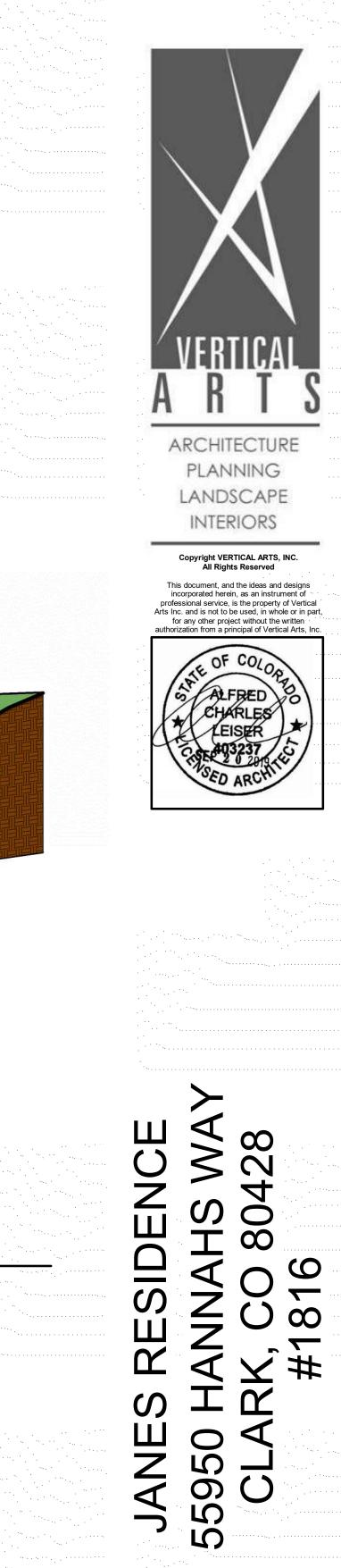
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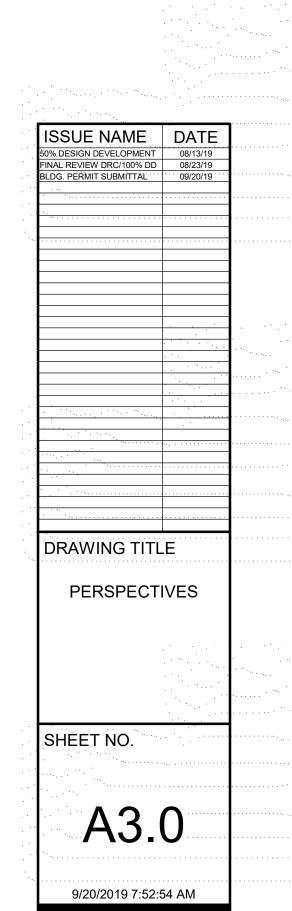
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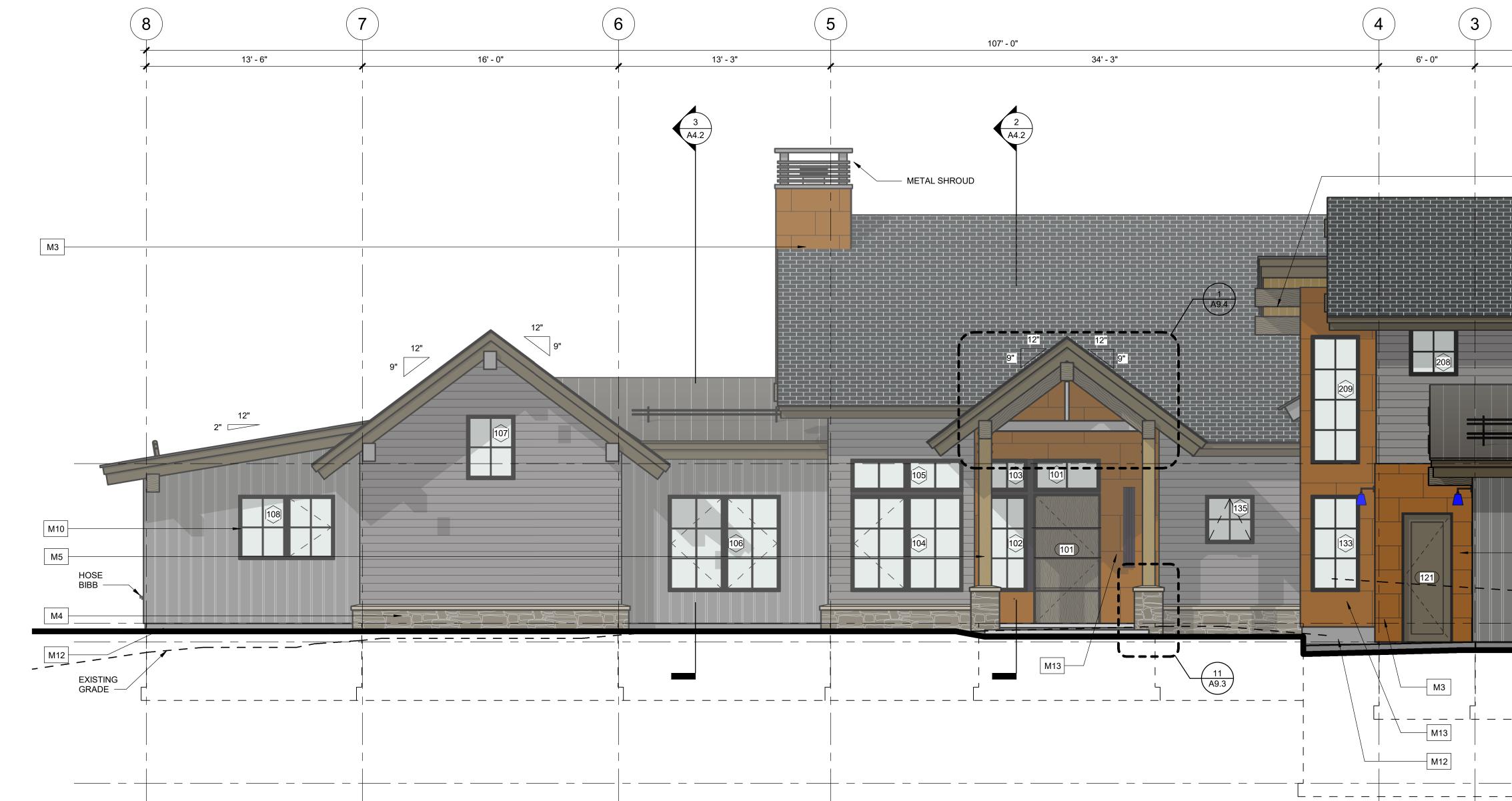
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1 PERSPECTIVE - 1









 $1 \frac{\text{NORTH ELEVATION}}{1/4" = 1'-0"}$

EXTERIOR MATERIALS

MANUFACTURER

MATERIAL

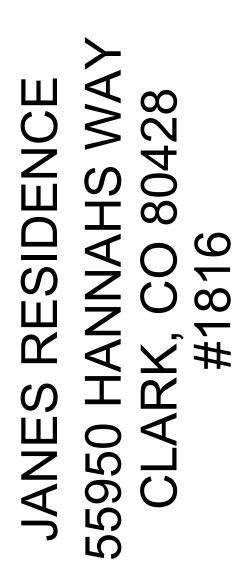
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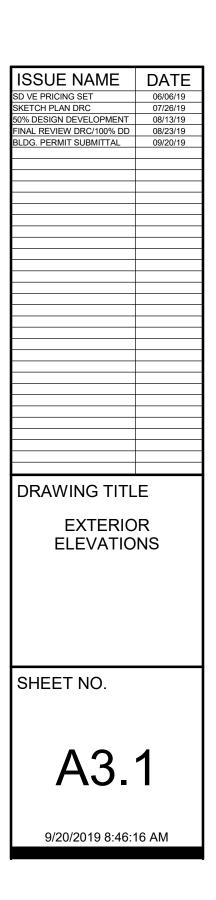


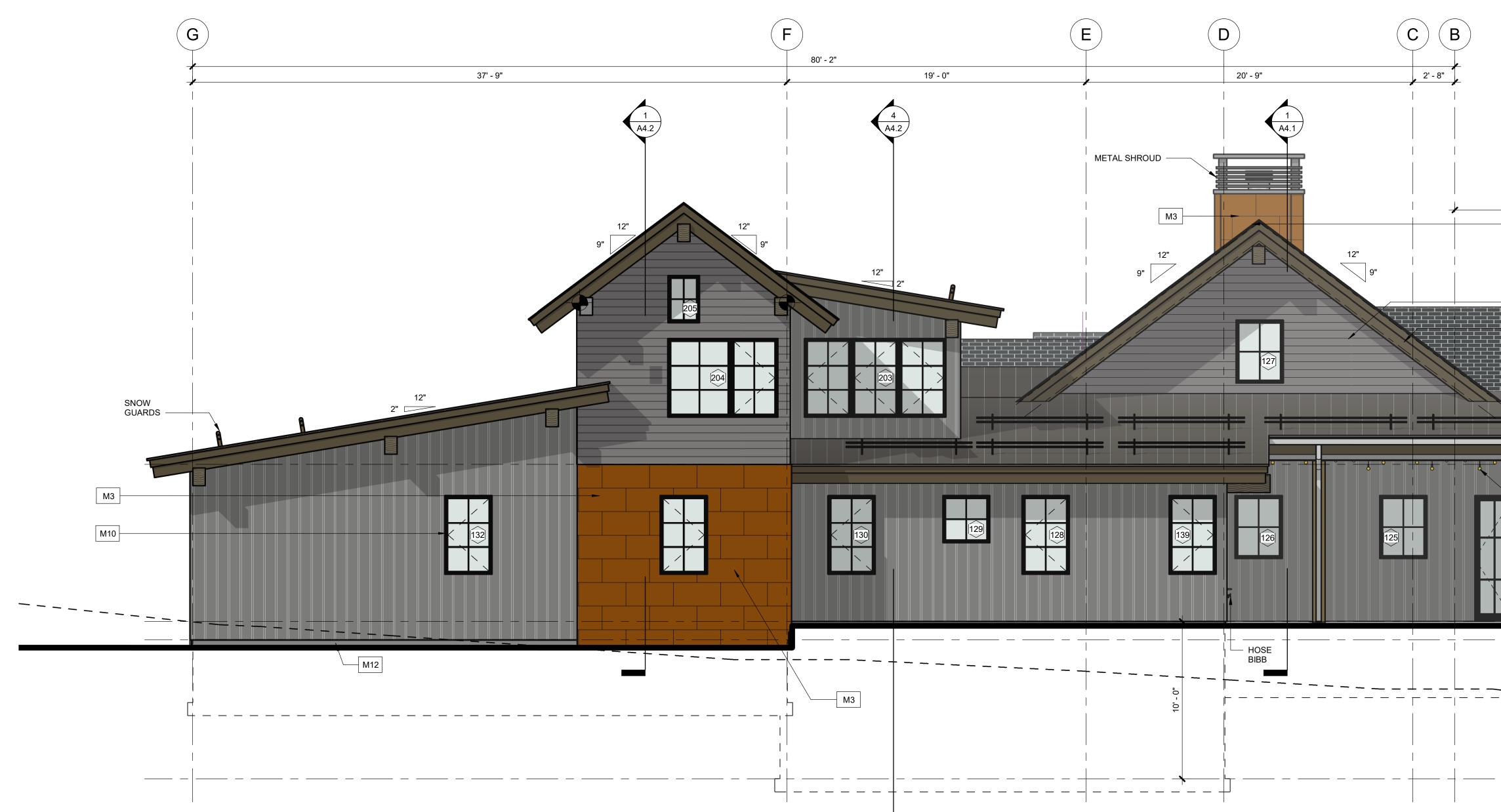
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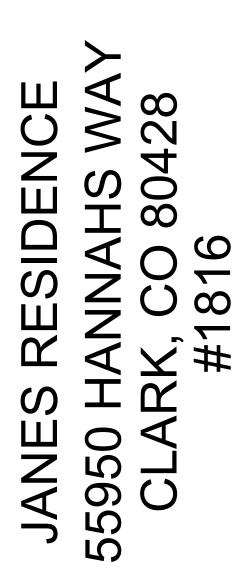


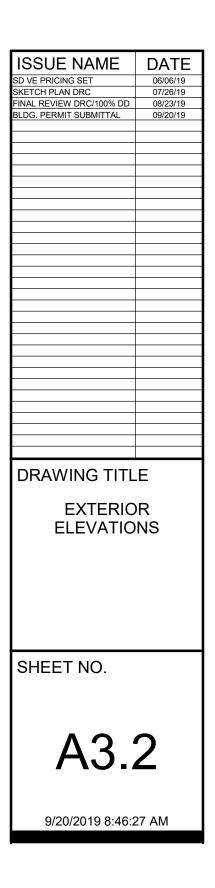


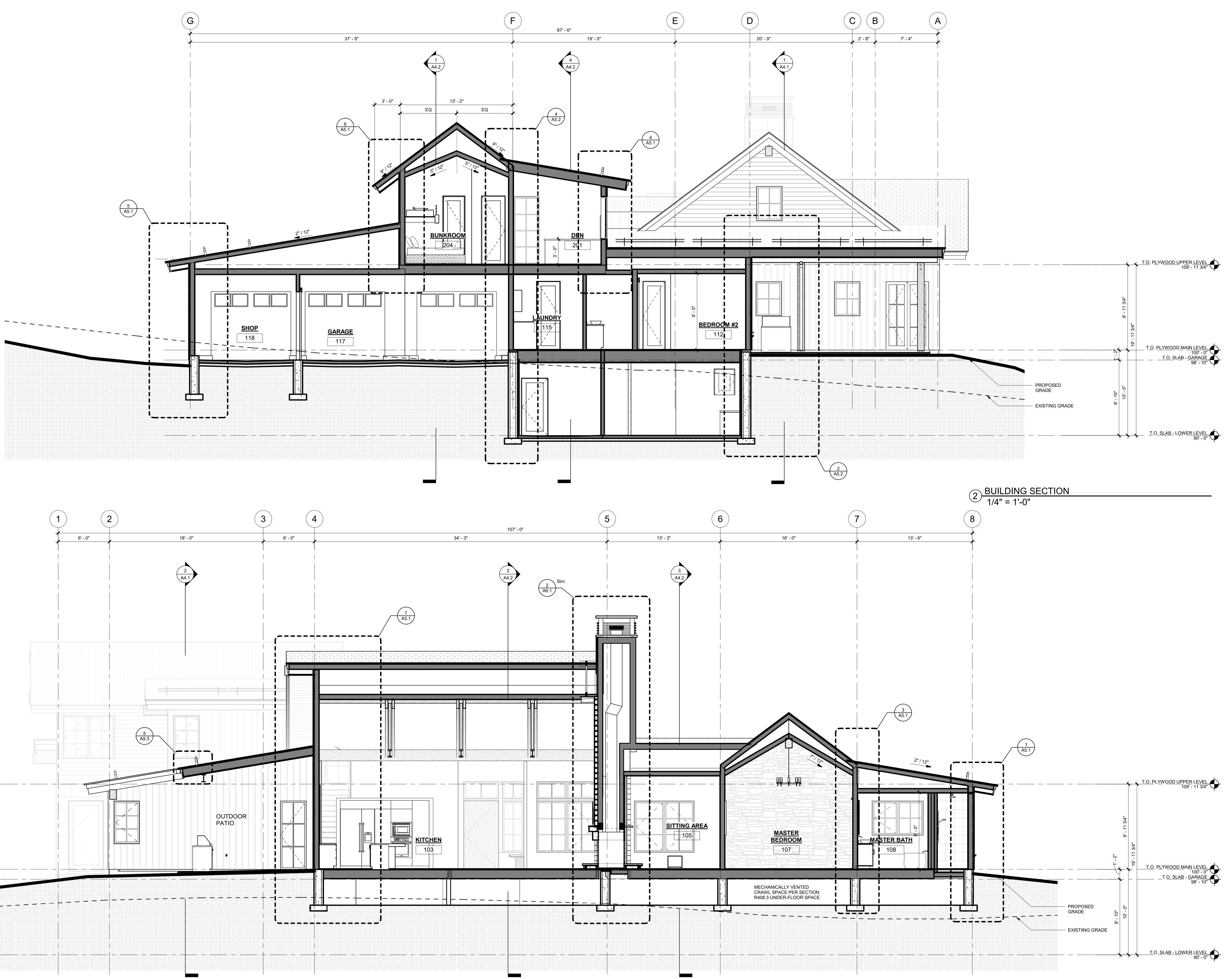


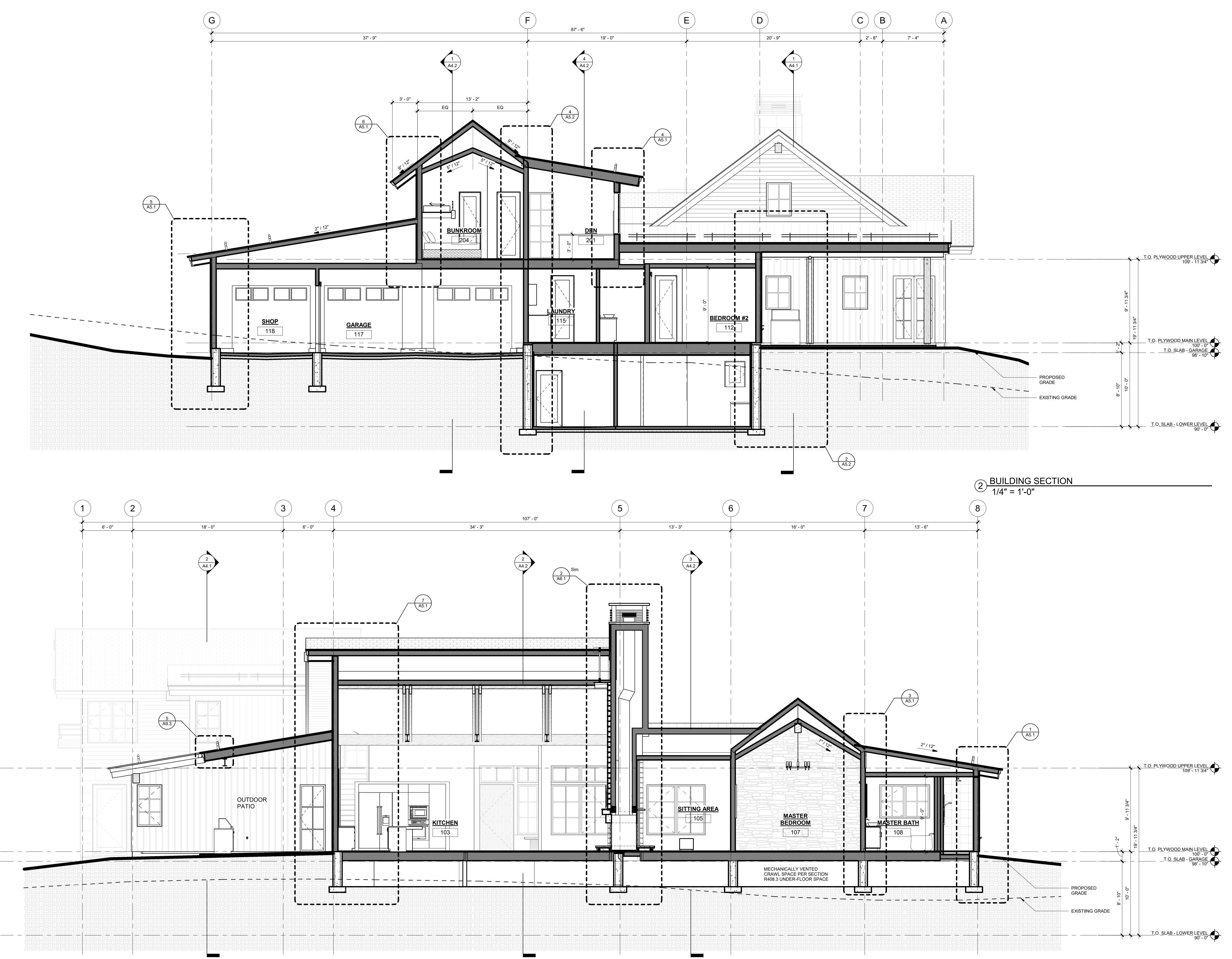






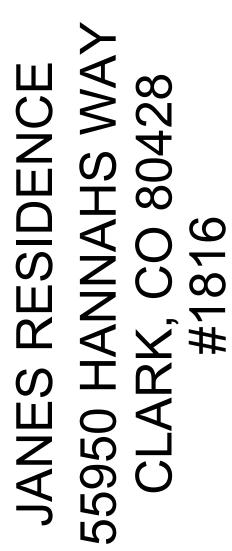


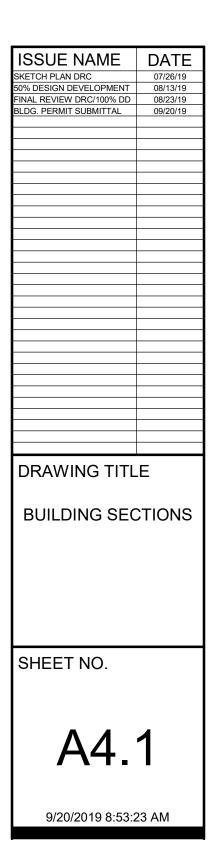


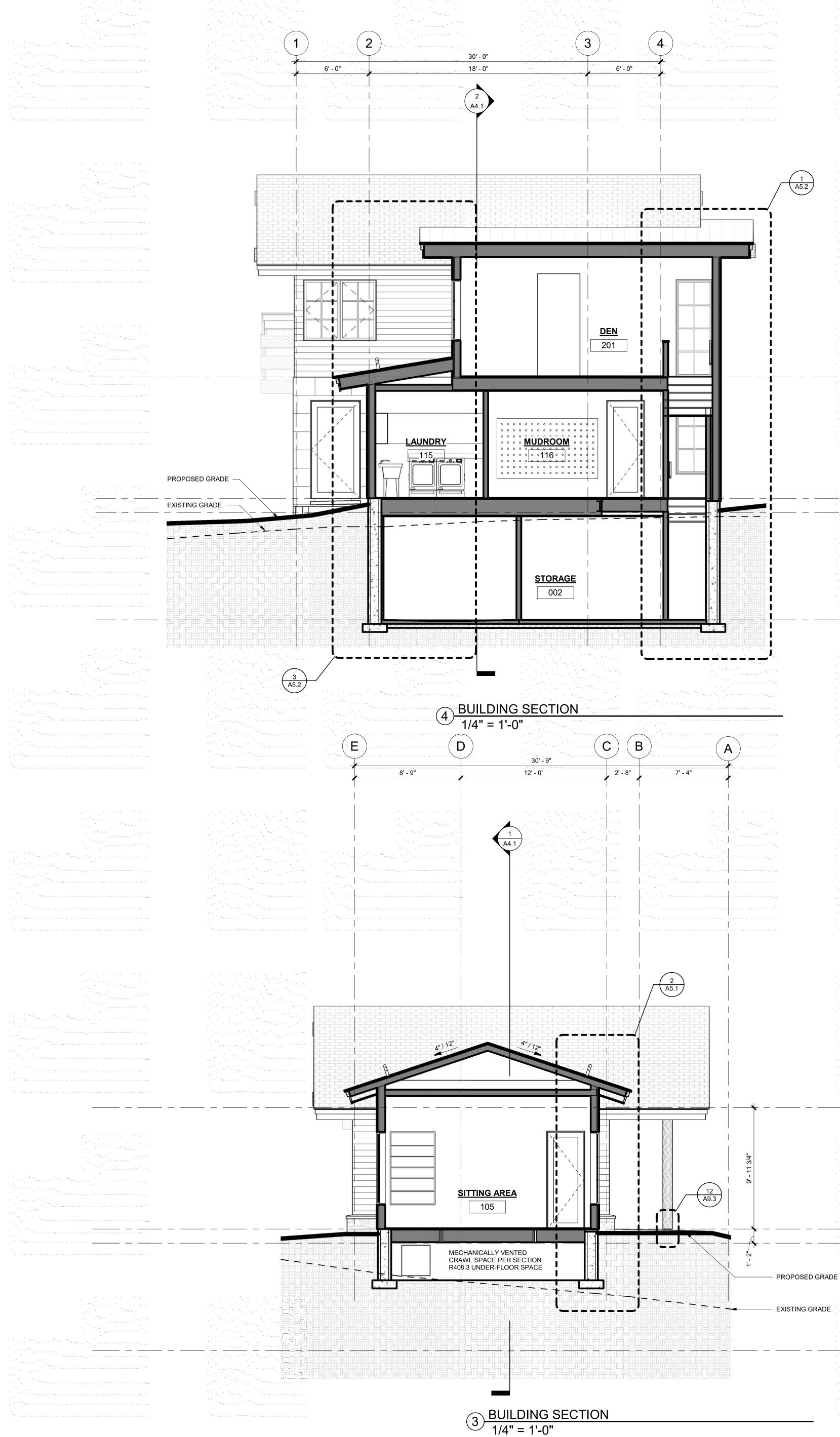


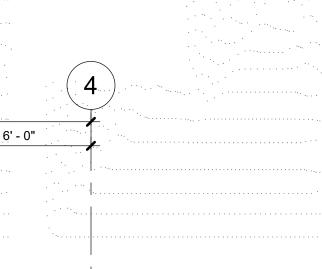
 $1 \frac{\text{BUILDING SECTION}}{1/4" = 1'-0"}$









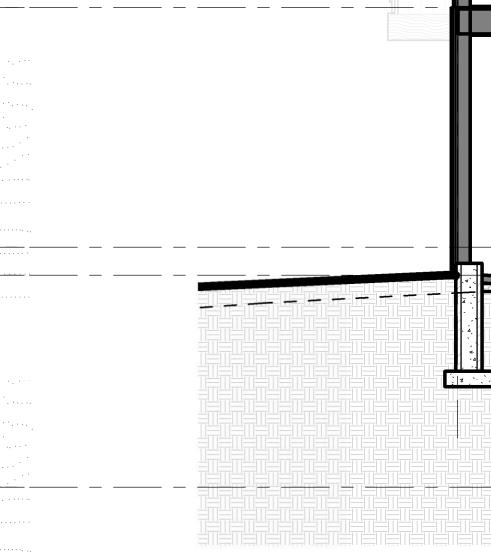


- PROPOSED GRADE

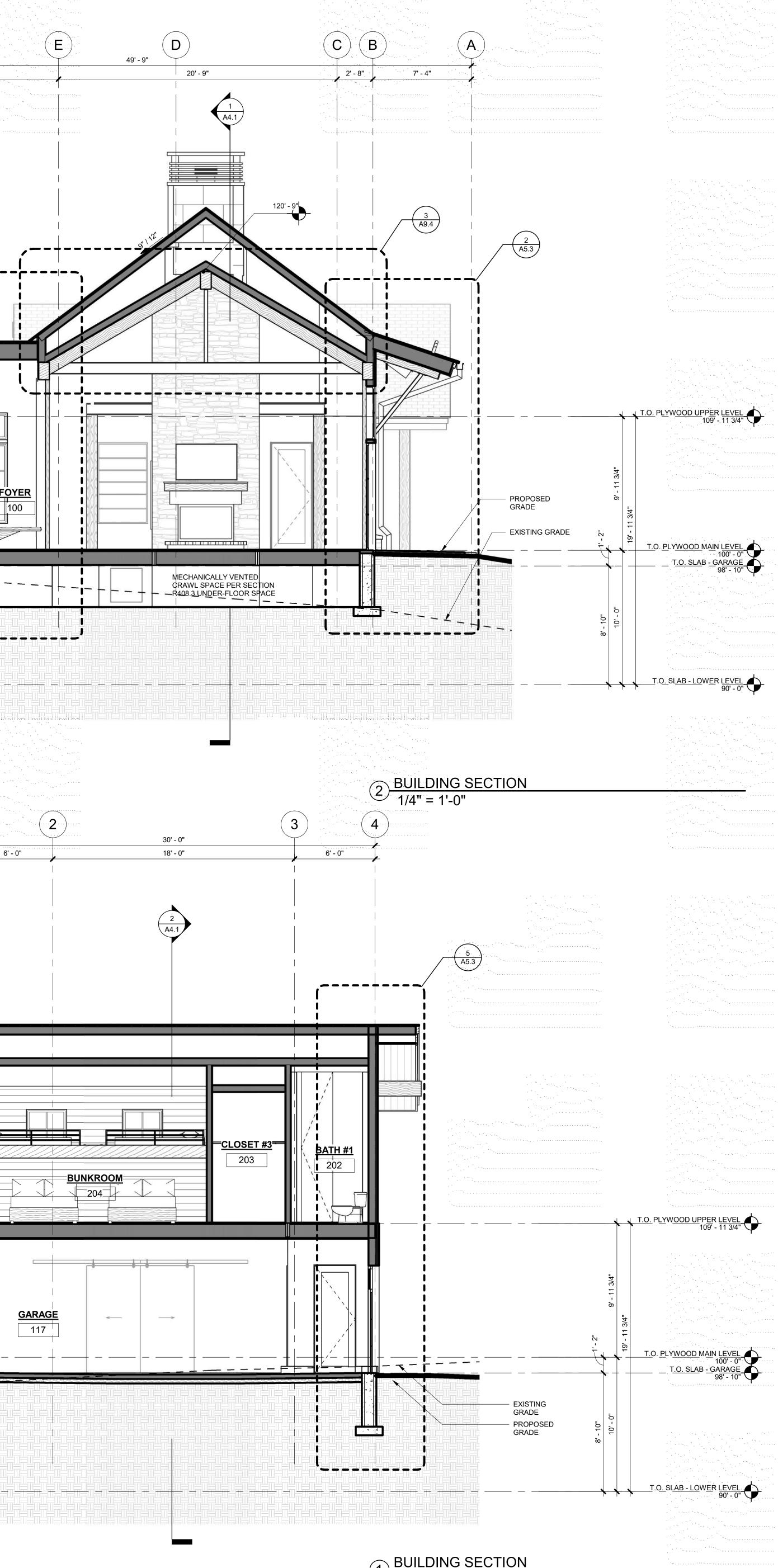
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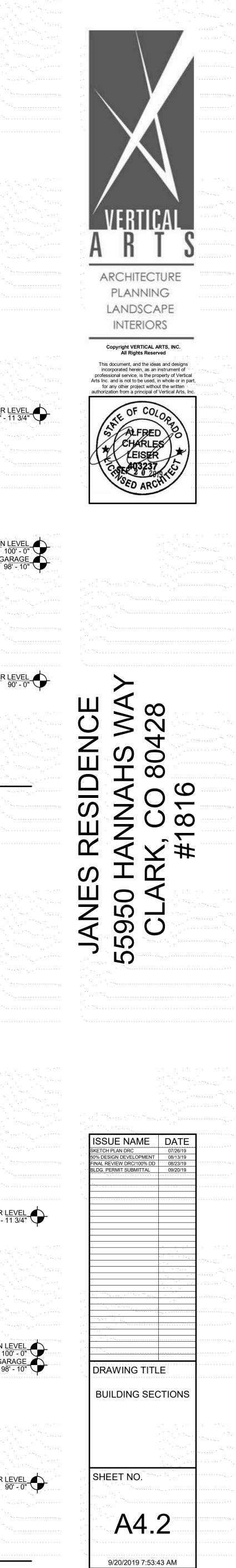
19' - 0" A9.3 100 ----



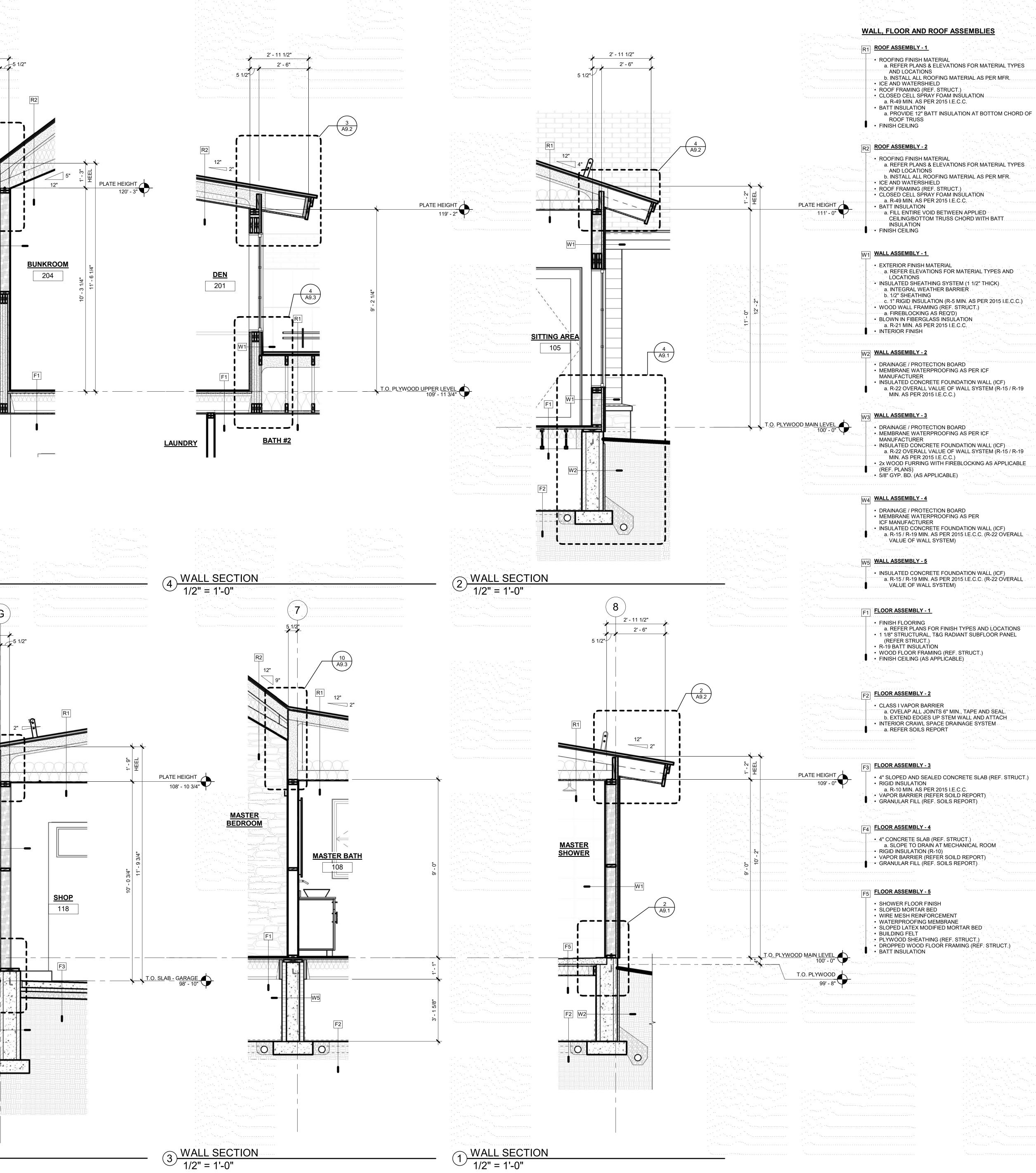
6' - 0"



 $1 \frac{\text{BUILDING SECTION}}{1/4" = 1'-0"}$



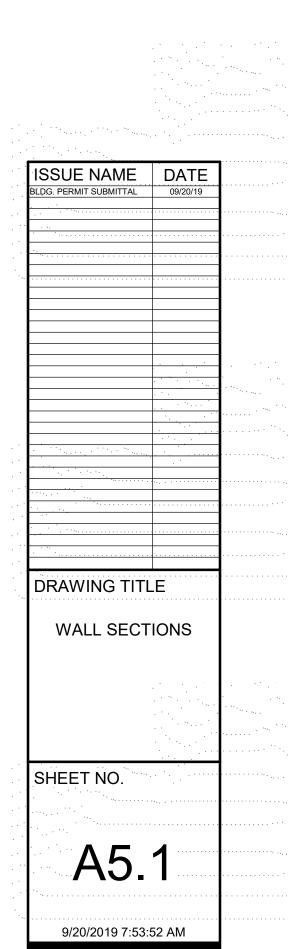
			3' - 5 1/2" <u>3' - 0"</u>
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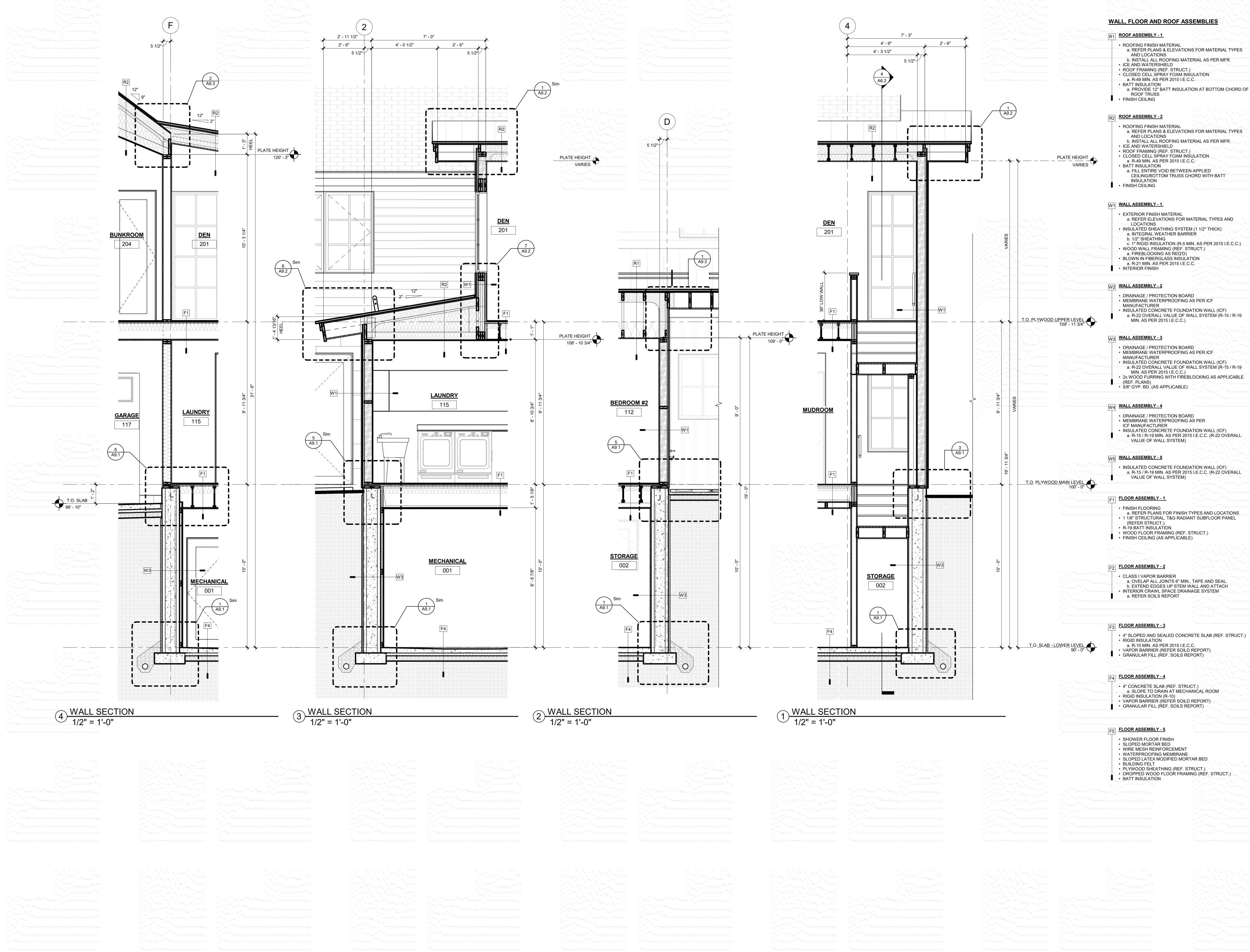




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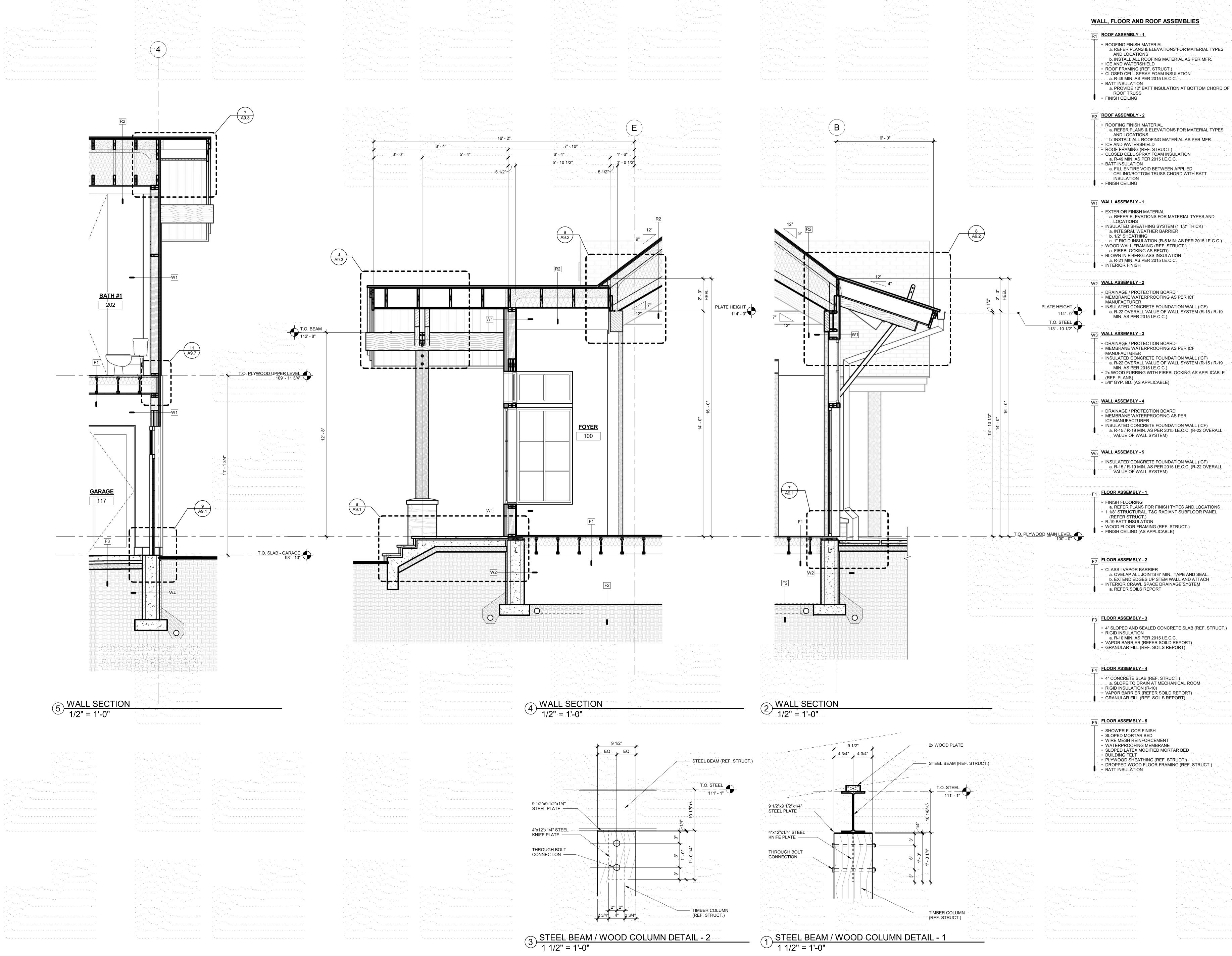
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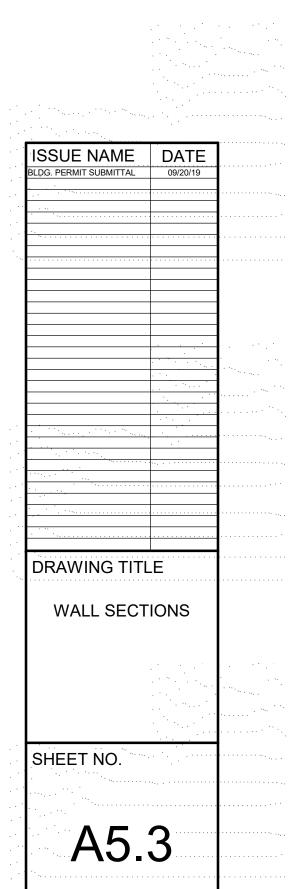
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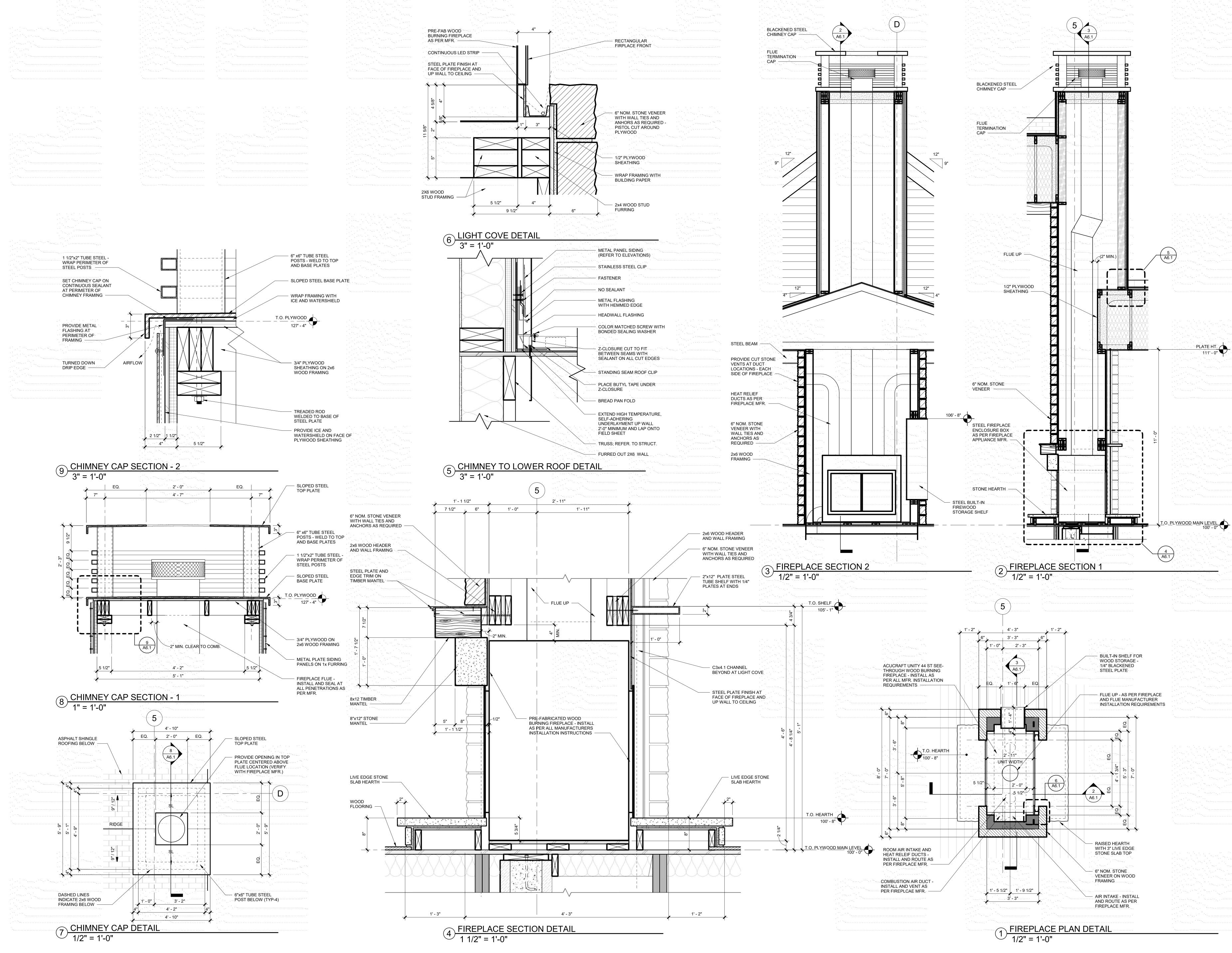


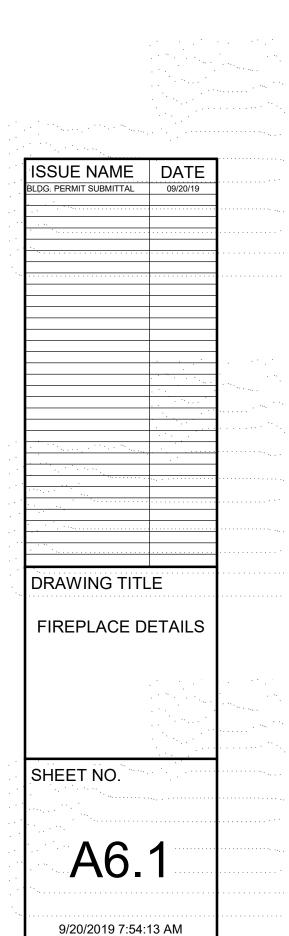
PLATE HT. 111' - 0"

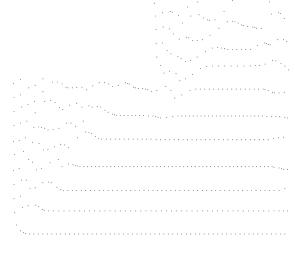


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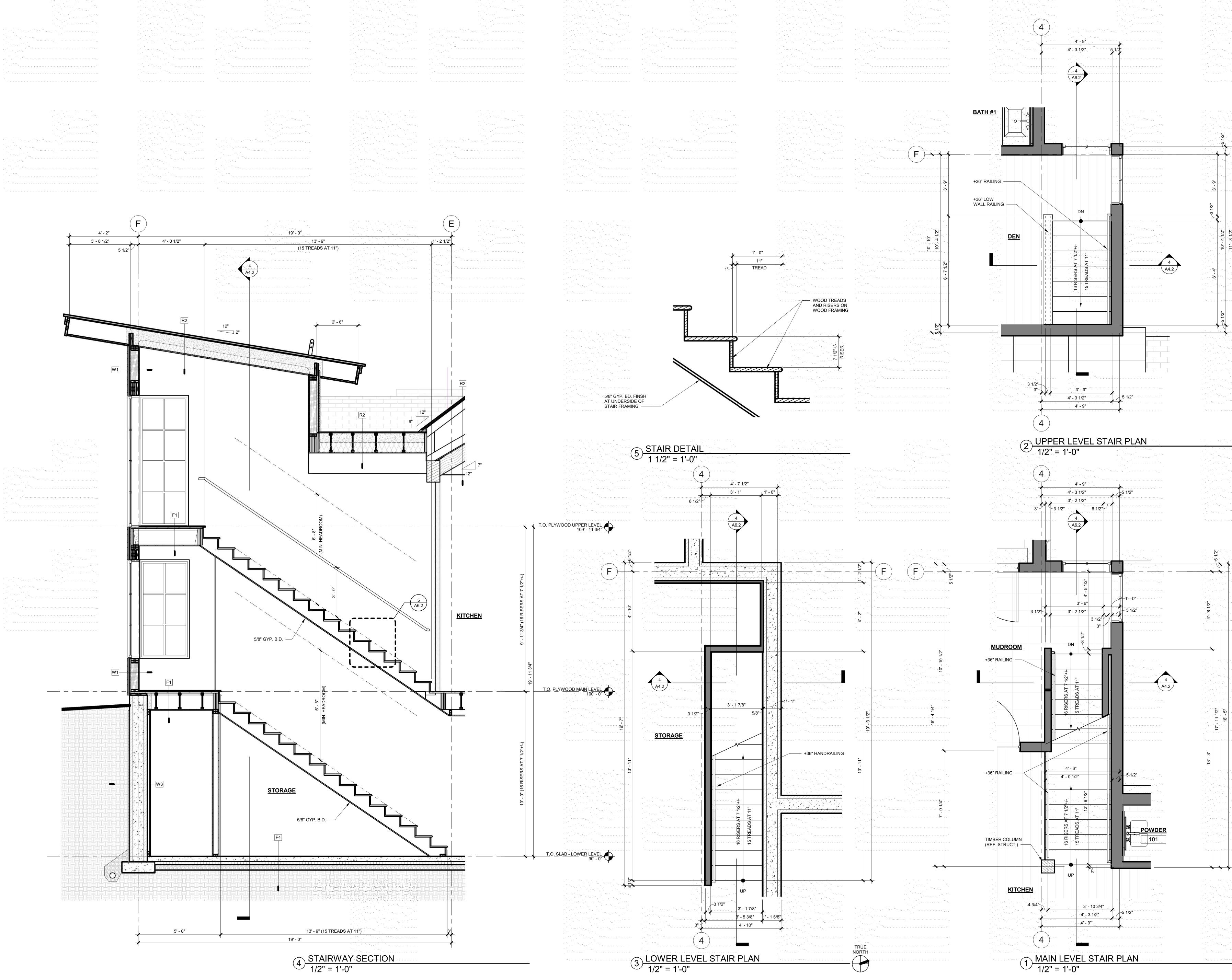
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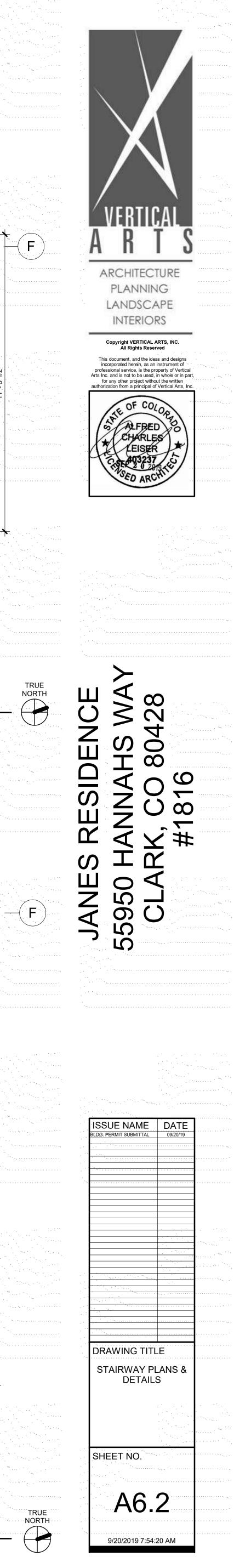
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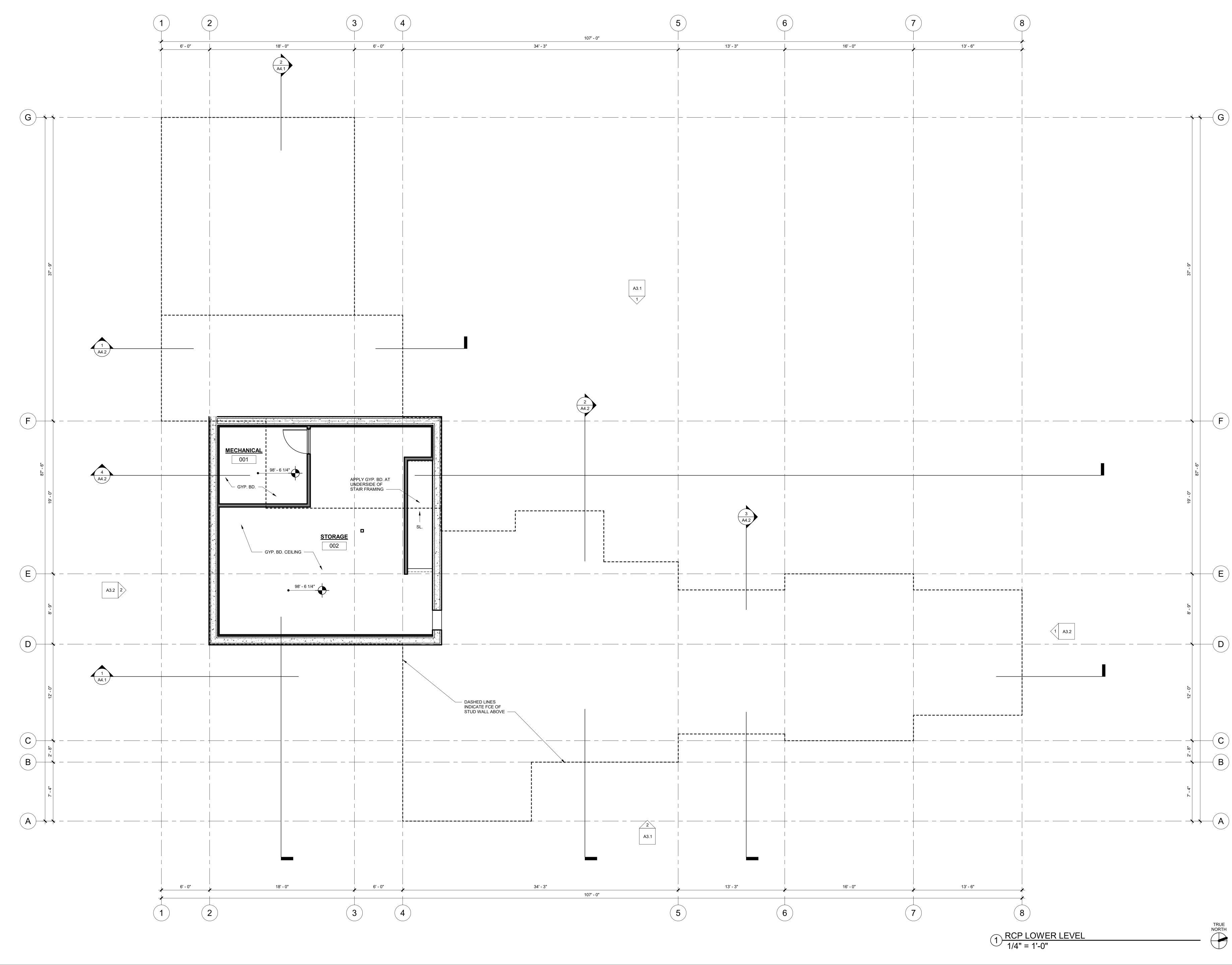




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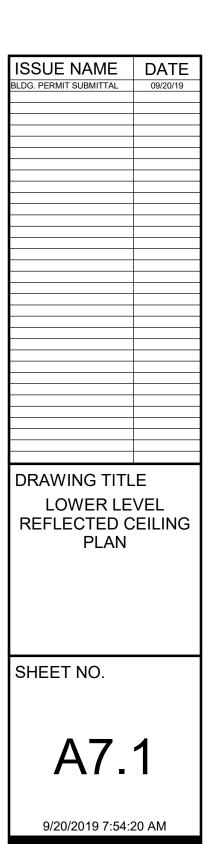
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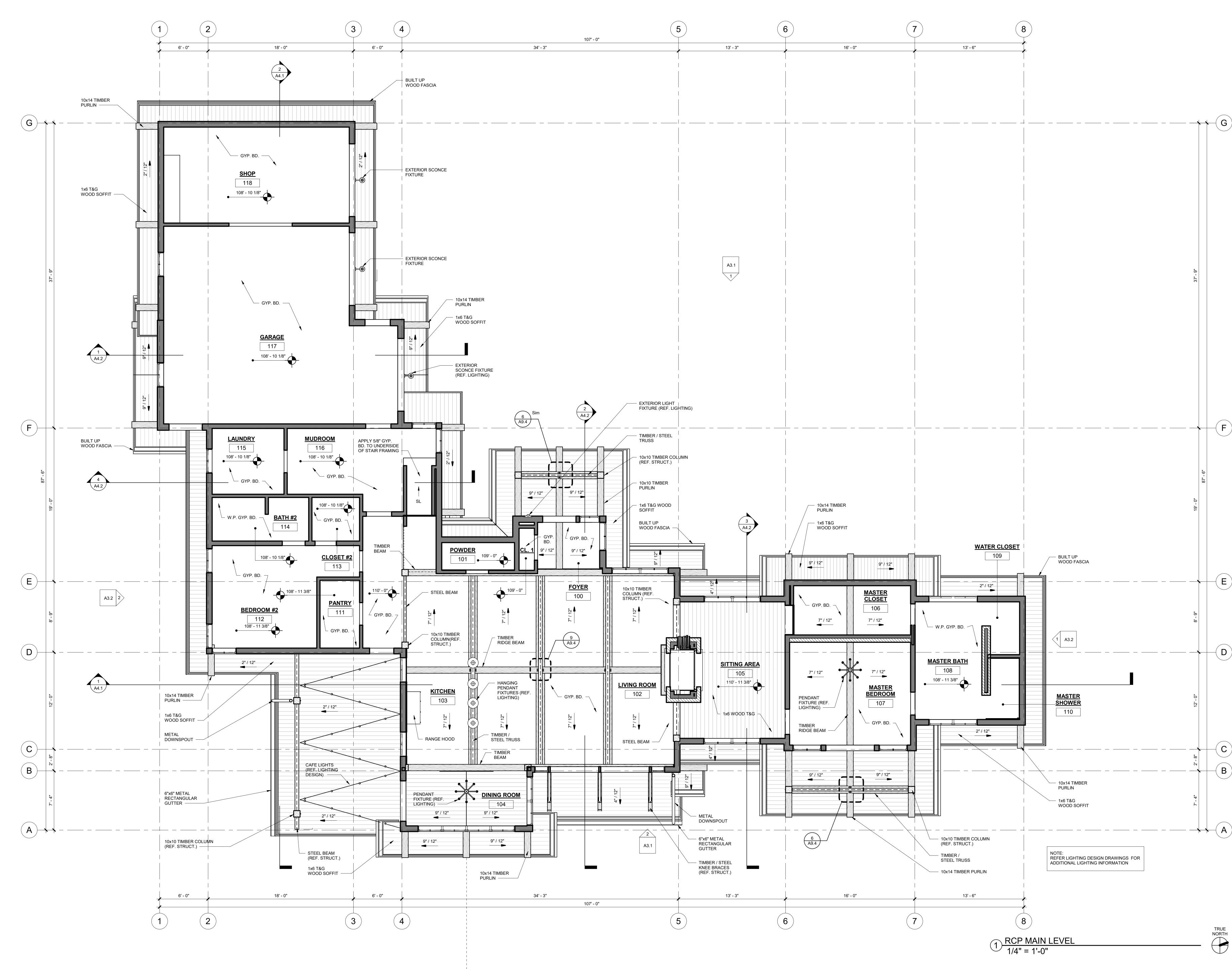
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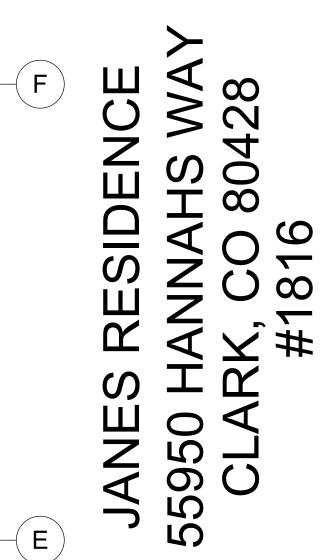
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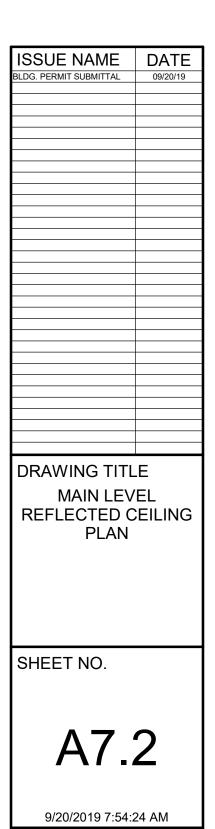














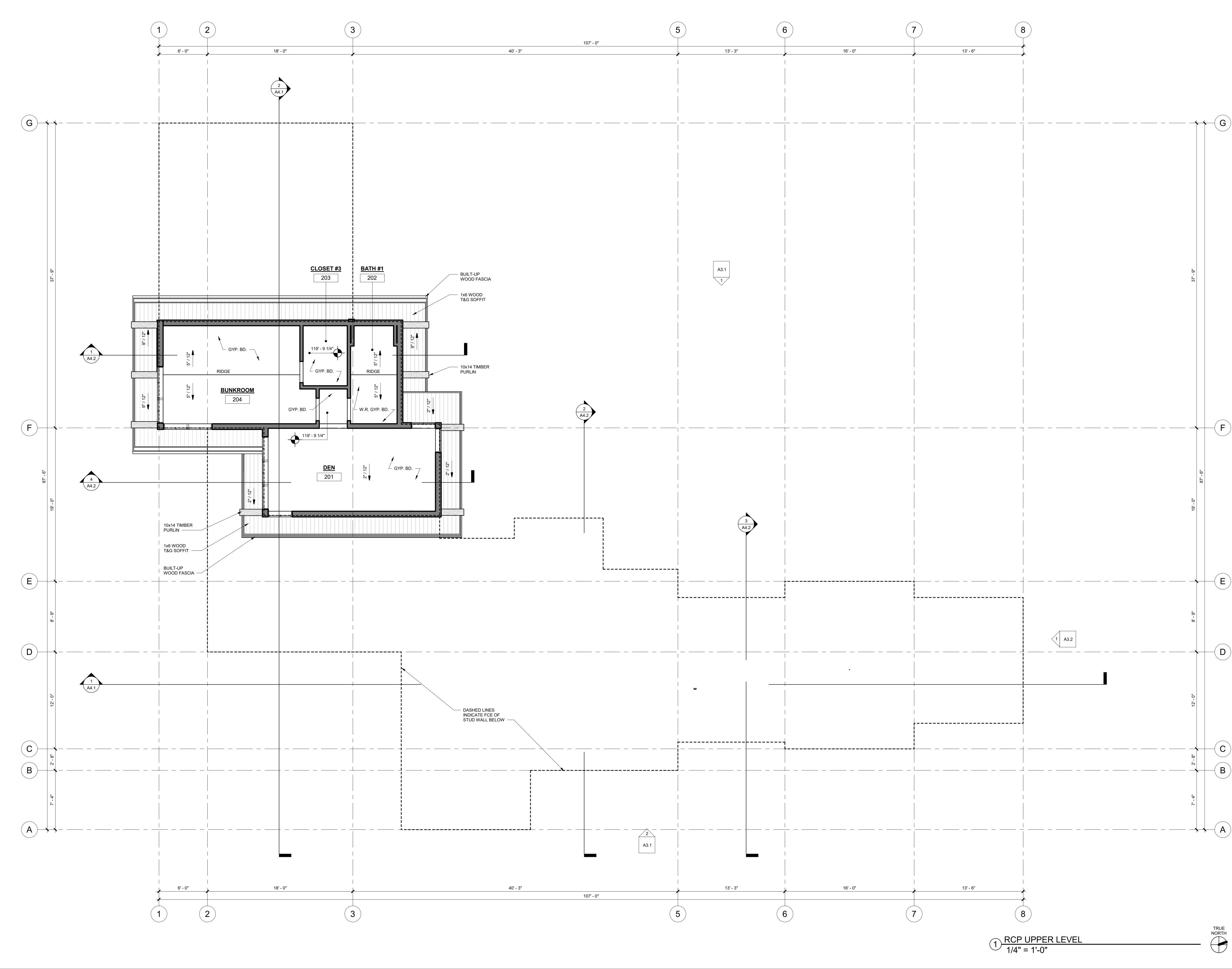
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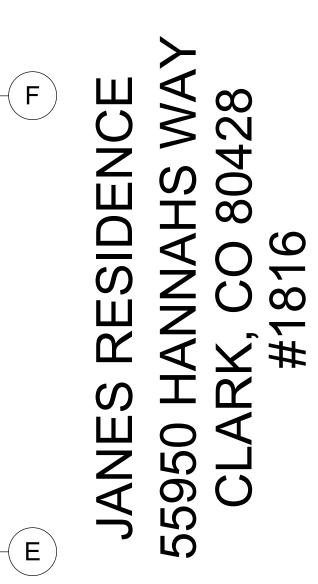
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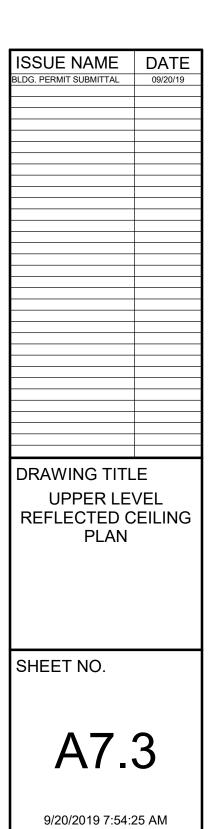
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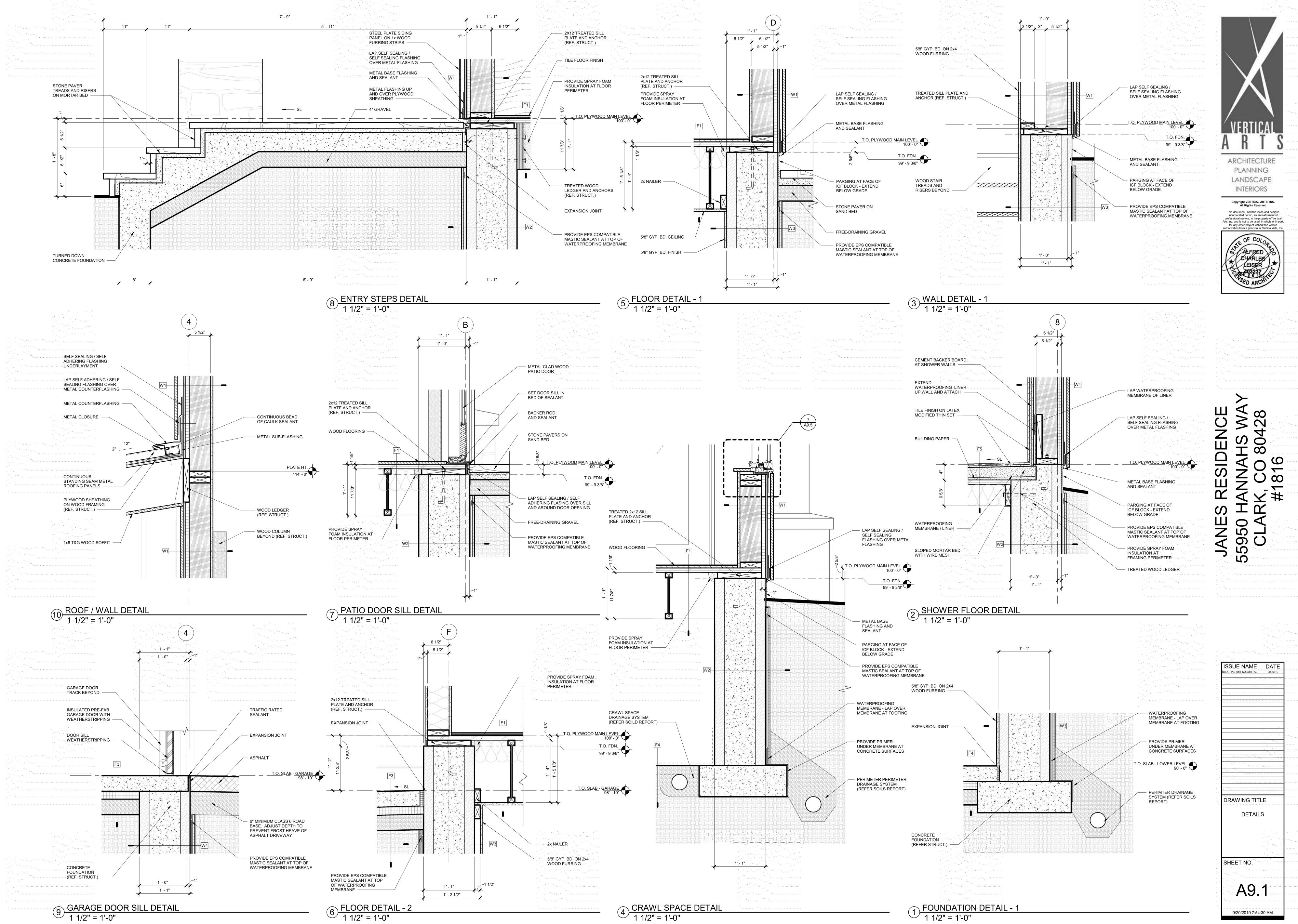


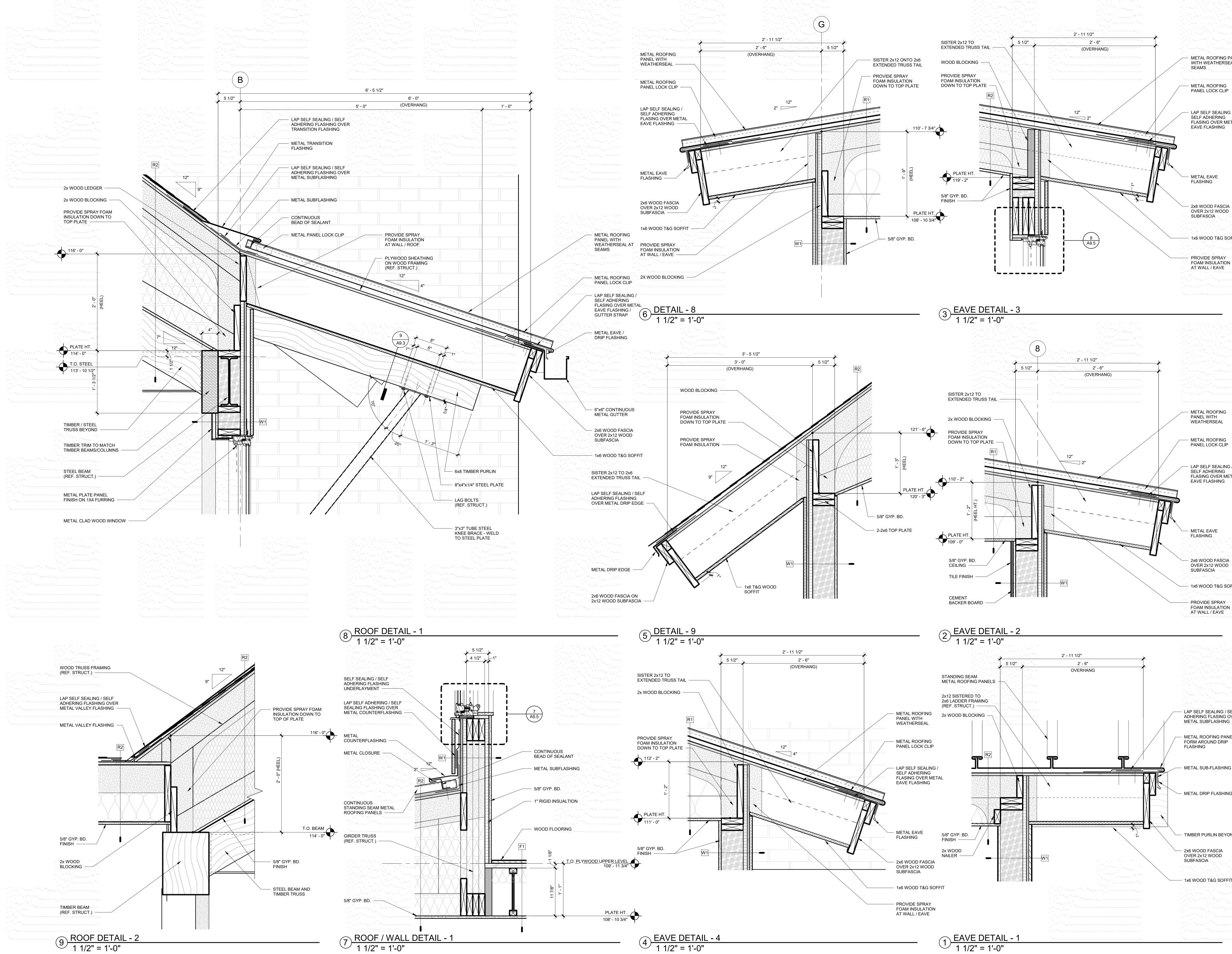
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METAL ROOFING PANEL WITH WEATHERSEAL AT

METAL ROOFING PANEL LOCK CLIP

LAP SELF SEALING / SELF ADHERING FLASING OVER METAL EAVE FLASHING

2x6 WOOD FASCIA OVER 2x12 WOOD 1x6 WOOD T&G SOFFIT

FOAM INSULATION

PANEL LOCK CLIP

 LAP SELF SEALING / SELF ADHERING FLASING OVER METAL EAVE FLASHING

2x6 WOOD FASCIA OVER 2x12 WOOD - 1x6 WOOD T&G SOFFIT

FOAM INSULATION AT WALL / EAVE

- LAP SELF SEALING / SELF ADHERING FLASING OVER

METAL ROOFING PANEL

METAL DRIP FLASHING

TIMBER PURLIN BEYOND

1x6 WOOD T&G SOFFIT

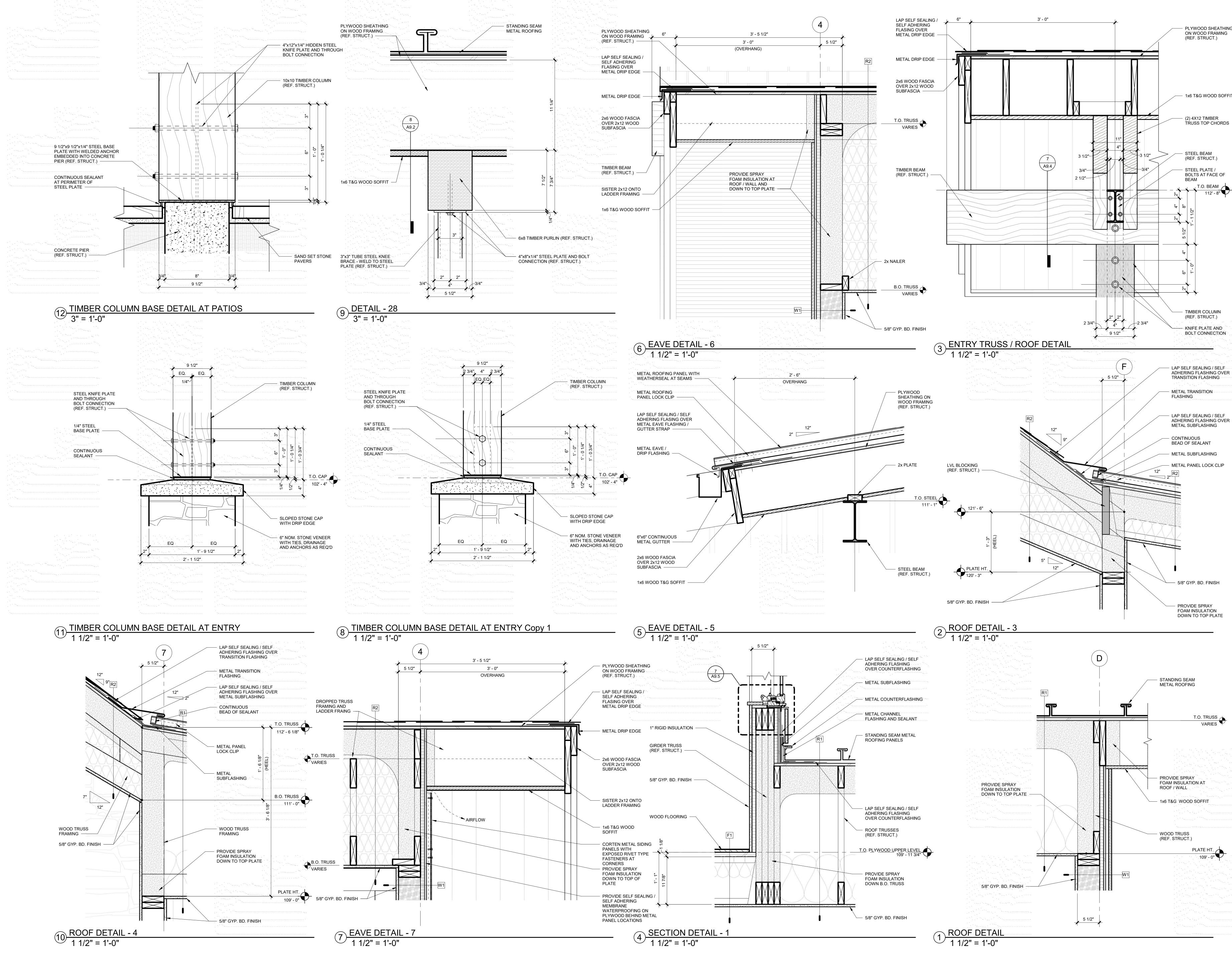


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- PLYWOOD SHEATHING ON WOOD FRAMING (REF. STRUCT.)

1x6 T&G WOOD SOFFIT

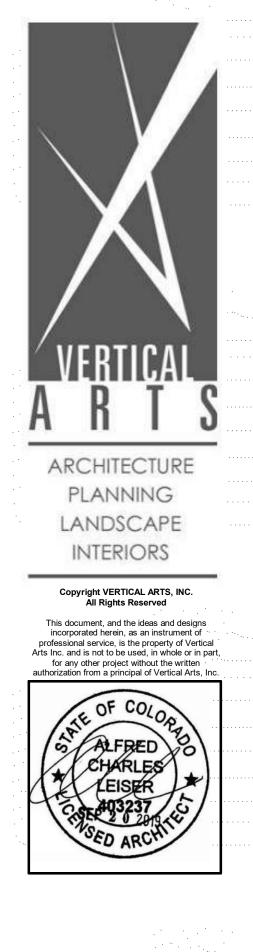
(2) 4X12 TIMBER TRUSS TOP CHORDS

BOLTS AT FACE OF T.O. BEAM 112' - 8" 🛡

TIMBER COLUMN (REF. STRUCT.) - KNIFE PLATE AND BOLT CONNECTION

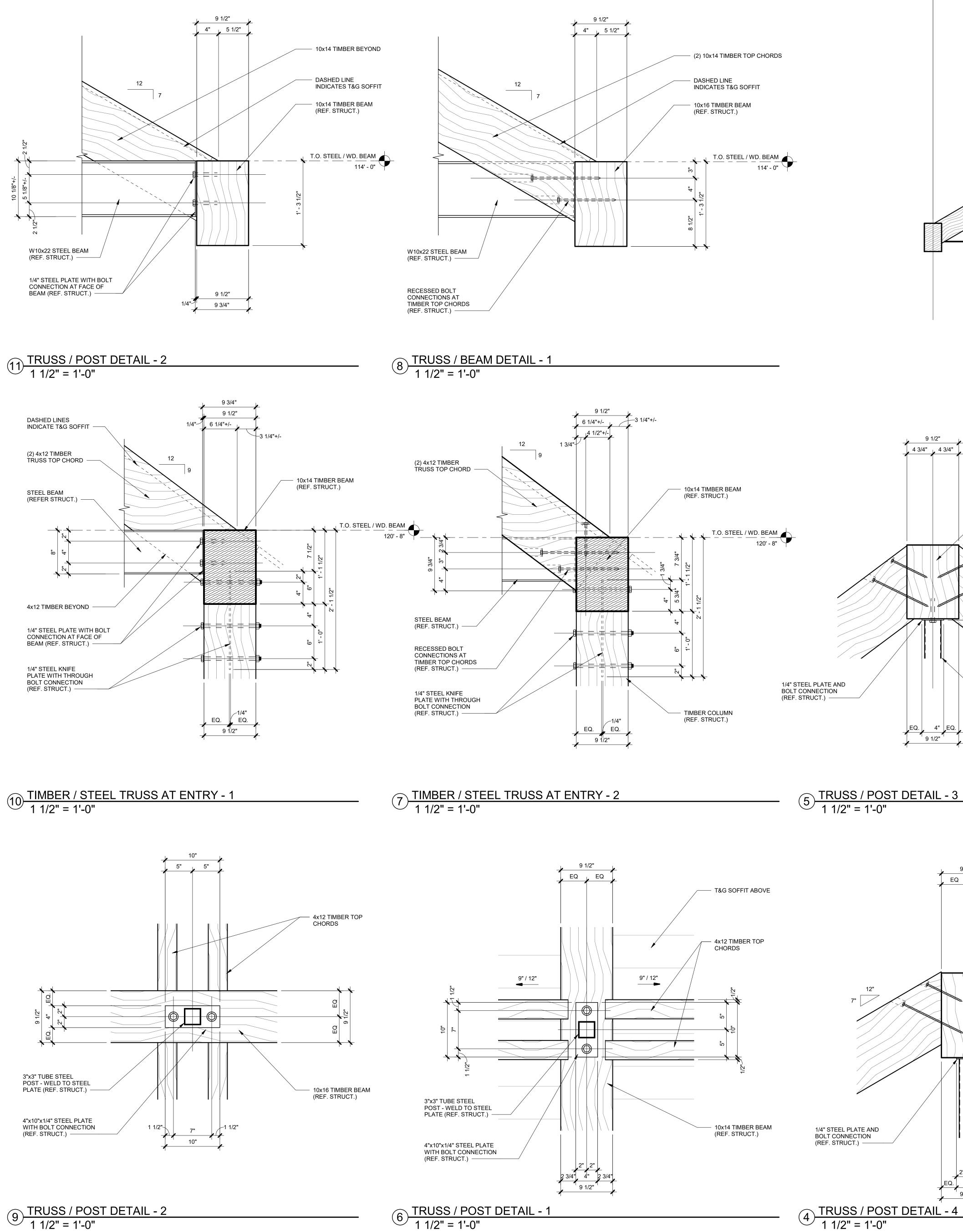
T.O. TRUSS VARIES

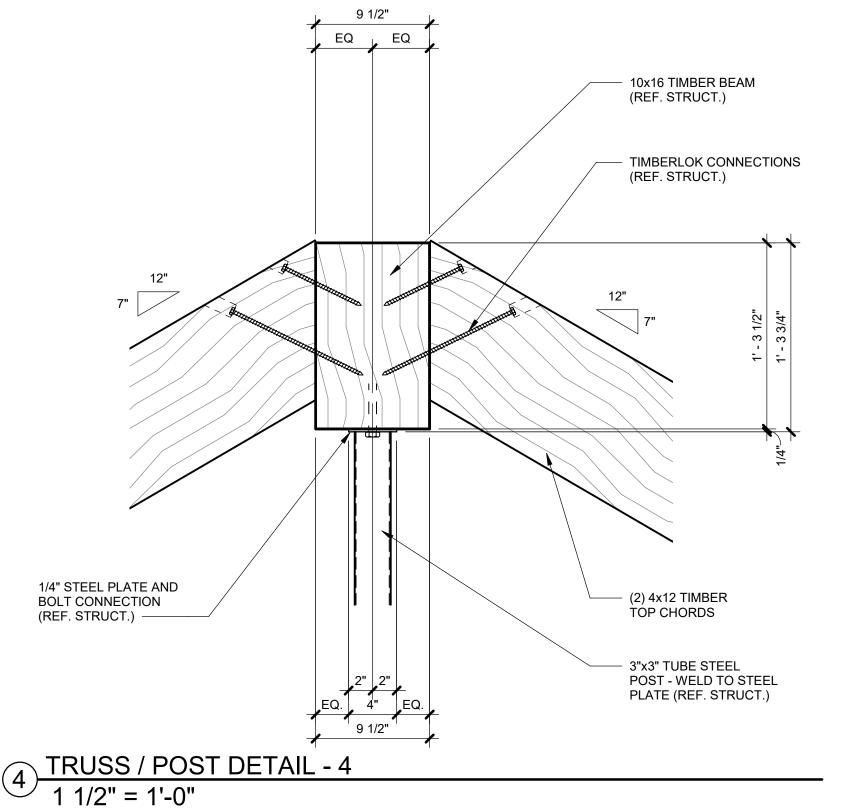
PLATE HT. 109' - 0"

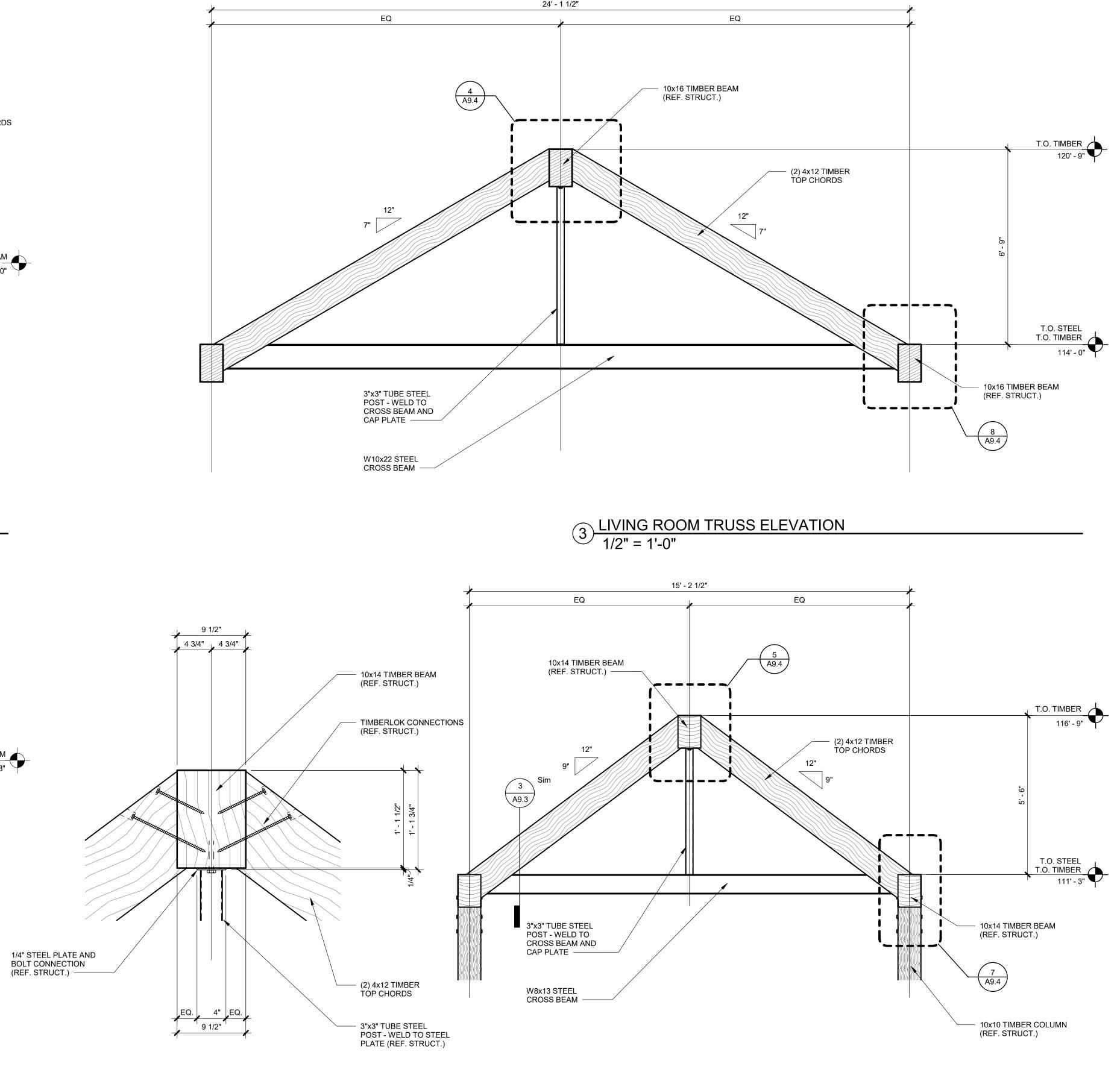


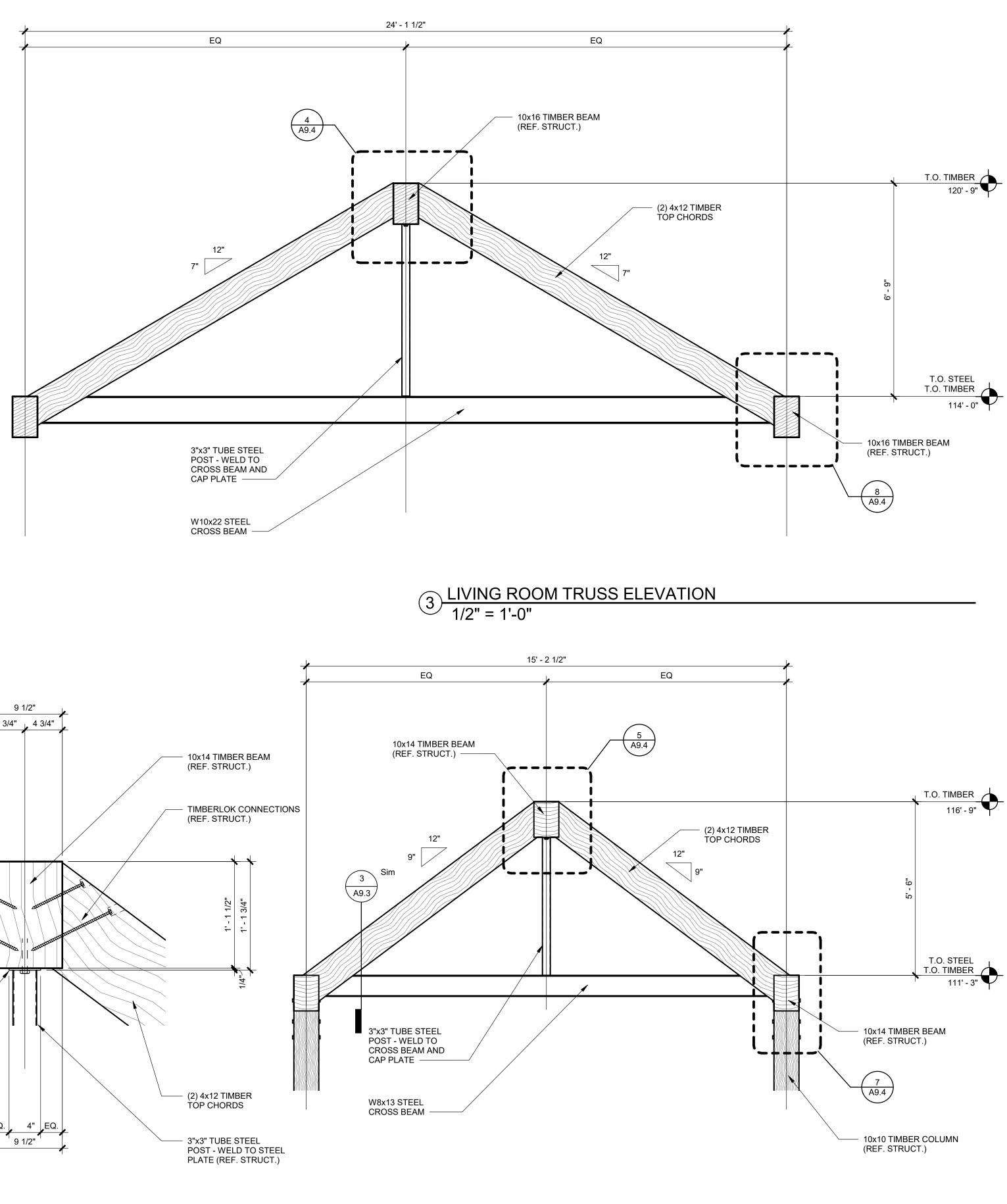
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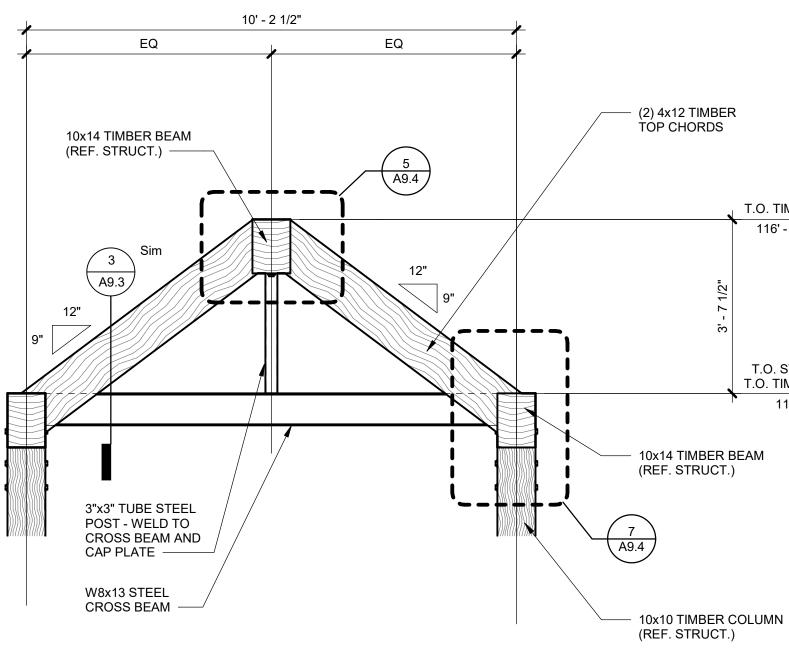








 $2 \frac{\text{MASTER PATIO TRUSS ELEVATION}}{1/2" = 1'-0"}$

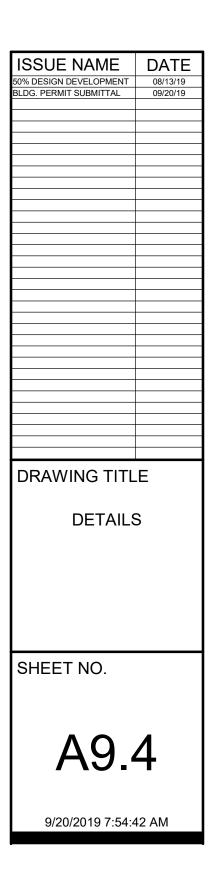


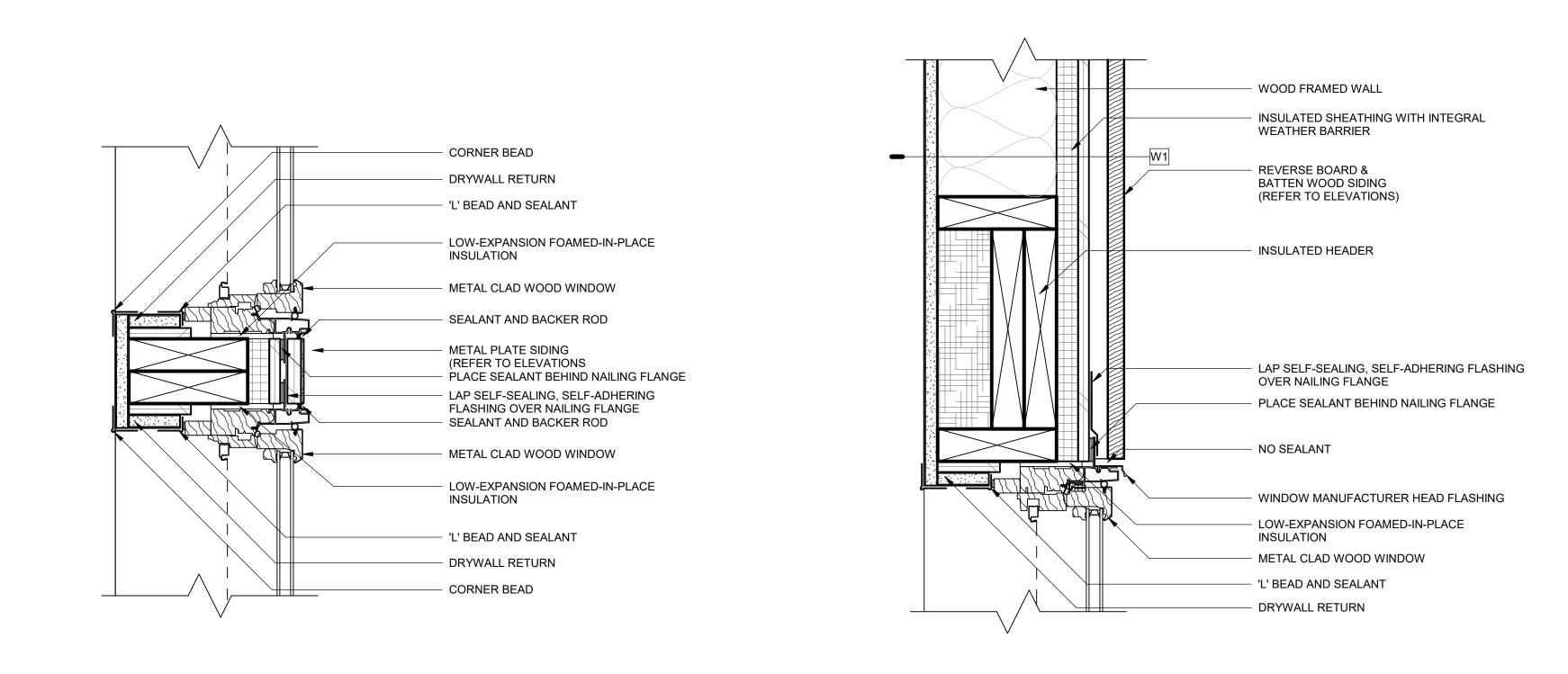


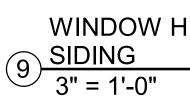
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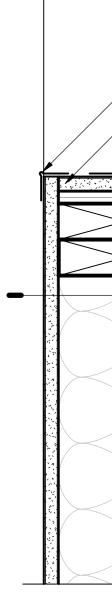
T.O. TIMBER 116' - 3 1/2"

T.O. STEEL T.O. TIMBER 112' - 8"

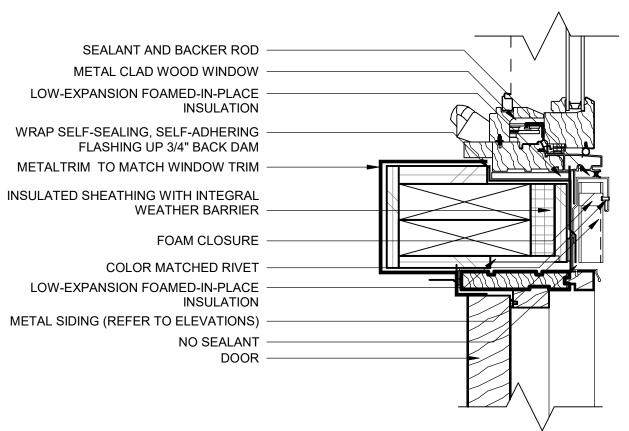




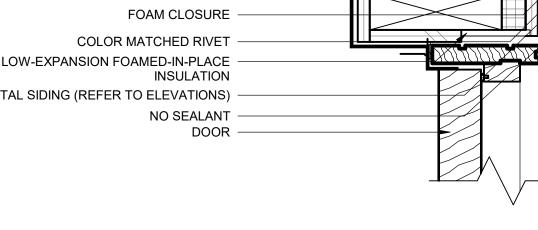




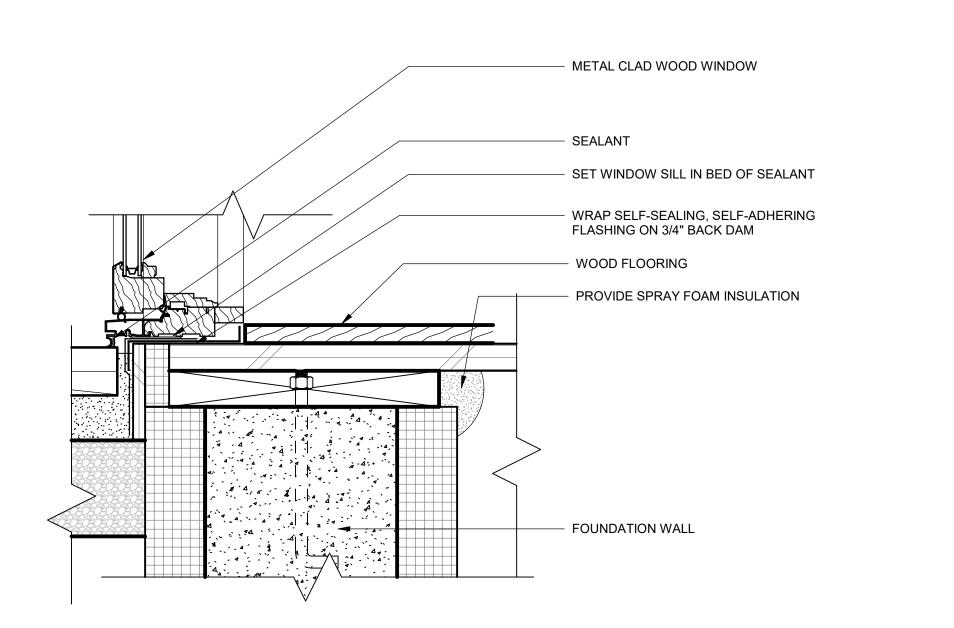
8 SIDING 3" = 1'-0"

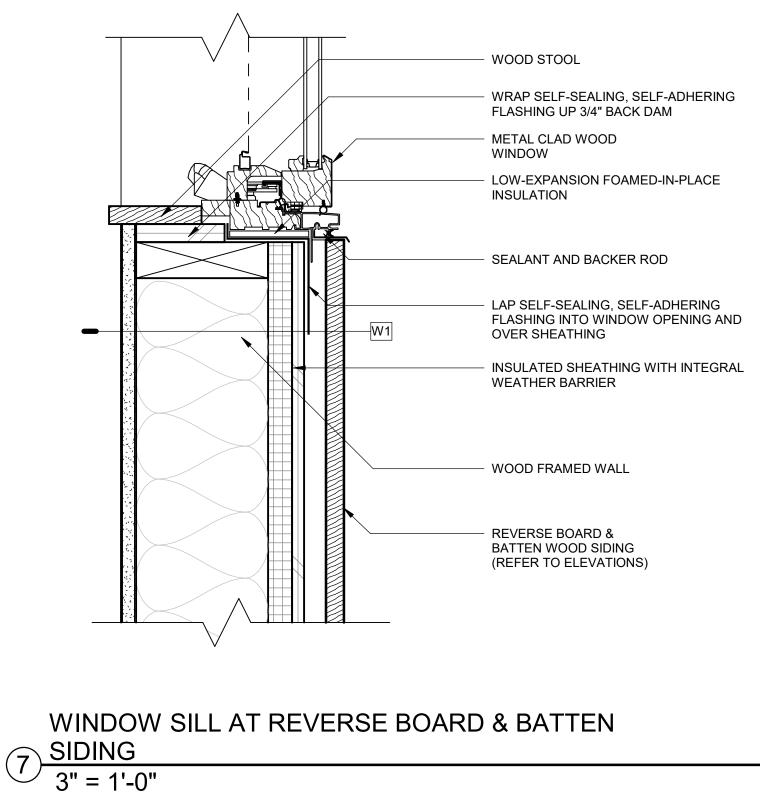


 $\textcircled{12} \underbrace{\text{WINDOW JAMB BLACK METAL}}_{3" = 1'-0"}$





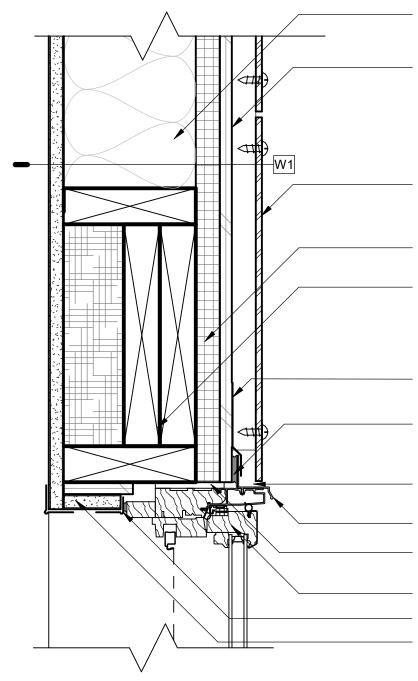




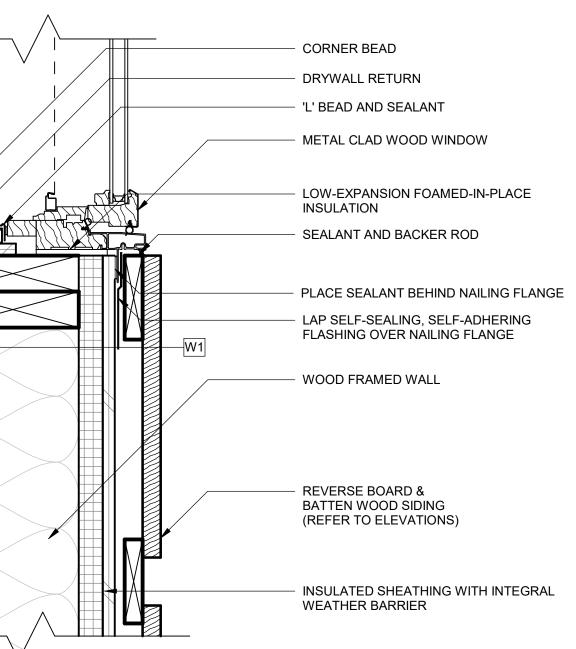
WINDOW JAMB AT REVERSE BOARD & BATTEN

 $\underbrace{10}_{3"} = 1'-0"$

WINDOW HEAD AT REVERSE BOARD & BATTEN



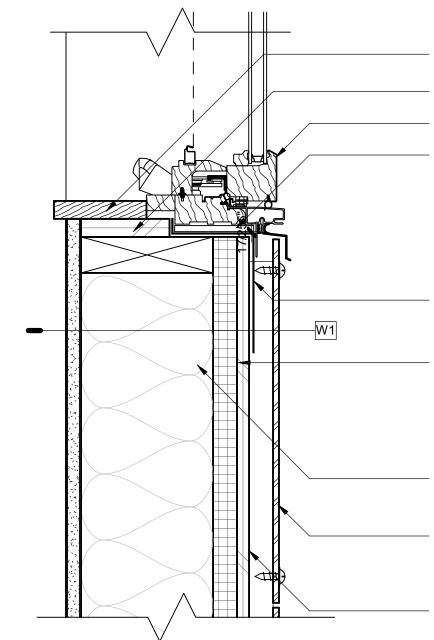
WINDOW HEAD AT CORTEN METAL SIDING 6 PANELS 3" = 1'-0"





______W1





 $(4) \frac{\text{WINDOW SILL AT CORTEN METAL SIDING PANELS}}{3" = 1'-0"}$

WOOD WALL FRAMING

PROVIDE SELF-SEALING-/SELF-ADHERING MEMBRANE WATERPROOFING ON PLYWOOD BEHIND METAL PANEL LOCATIONS

CORTEN METAL SIDING PANELS WITH EXPOSED RIVET TYPE FASTENERS AT CORNERS (REFER

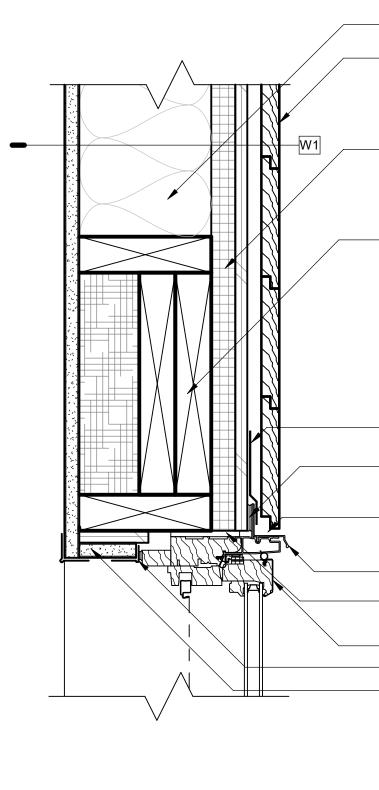
TO ELEVATIONS) INSULATED SHEATHING WITH INTEGRAL WEATHER BARRIER INSULATED HEADER

LAP SELF-SEALING, SELF-ADHERING FLASHING OVER NAILING FLANGE PLACE SEALANT BEHIND NAILING FLANGE

NO SEALANT

WINDOW MANUFACTURER HEAD FLASHING LOW-EXPANSION FOAMED-IN-PLACE INSULATION METAL CLAD WOOD WINDOW

- 'L' BEAD AND SEALANT DRYWALL RETURN



WOOD WALL FRAMING

HORIZONTAL SHIPLAP WOOD SIDING (REFER TO ELEVATIONS)

INSULATED SHEATHING WITH INTEGRAL WEATHER BARRIER

INSULATED HEADER

- LAP SELF-SEALING, SELF-ADHERING FLASHING OVER NAILING FLANGE PLACE SEALANT BEHIND NAILING FLANGE

NO SEALANT

WINDOW MANUFACTURER HEAD FLASHING LOW-EXPANSION FOAMED-IN-PLACE INSULATION METAL CLAD WOOD WINDOW - 'L' BEAD AND SEALANT DRYWALL RETURN

WINDOW HEAD AT HORIZONTAL SHIPLAP SIDING 3" = 1'-0"

- CORNER BEAD DRYWALL RETURN 'L' BEAD AND SEALANT
- LOW-EXPANSION FOAMED-IN-PLACE INSULATION
- METAL CLAD WOOD WINDOW SEALANT AND BACKER ROD
- JAMB FLASHING - PLACE SEALANT BEHIND NAILING FLANGE
- LAP SELF-SEALING, SELF-ADHERING FLASHING OVER NAILING FLANGE WOOD WALL FRAMING
- CORTEN METAL SIDING PANELS WITH EXPOSED RIVET TYPE FASTENERS AT CORNERS (REFER TO ELEVATIONS)
- PROVIDE SELF-SEALING-/SELF-ADHERING MEMBRANE WATERPROOFING ON PLYWOOD BEHIND METAL PANEL LOCATIONS INSULATED SHEATHING WITH INTEGRAL WEATHER BARRIER

WOOD STOOL

WRAP SELF-SEALING, SELF-ADHERING FLASHING UP 3/4" BACK DAM METAL CLAD WOOD WINDOW

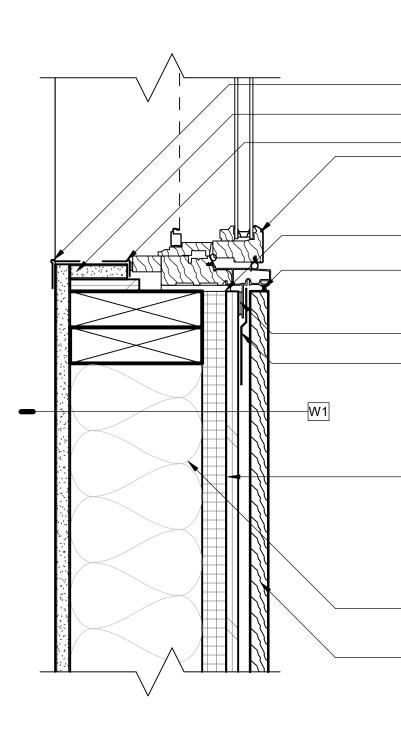
LOW-EXPANSION FOAMED-IN-PLACE INSULATION

LAP SELF-SEALING, SELF-ADHERING FLASHING INTO WINDOW OPENING AND OVER SHEATHING INSULATED SHEATHING WITH INTEGRAL WEATHER BARRIER

WOOD WALL FRAMING

CORTEN METAL SIDING PANELS WITH EXPOSED RIVET TYPE FASTENERS AT CORNERS (REFER TO ELEVATIONS)

PROVIDE SELF-SEALING-/SELF-ADHERING MEMBRANE WATERPROOFING ON PLYWOOD BEHIND METAL PANEL LOCATIONS



- CORNER BEAD - DRYWALL RETURN 'L' BEAD AND SEALANT METAL CLAD WOOD WINDOW

LOW-EXPANSION FOAMED-IN-PLACE INSULATION - SEALANT AND BACKER ROD

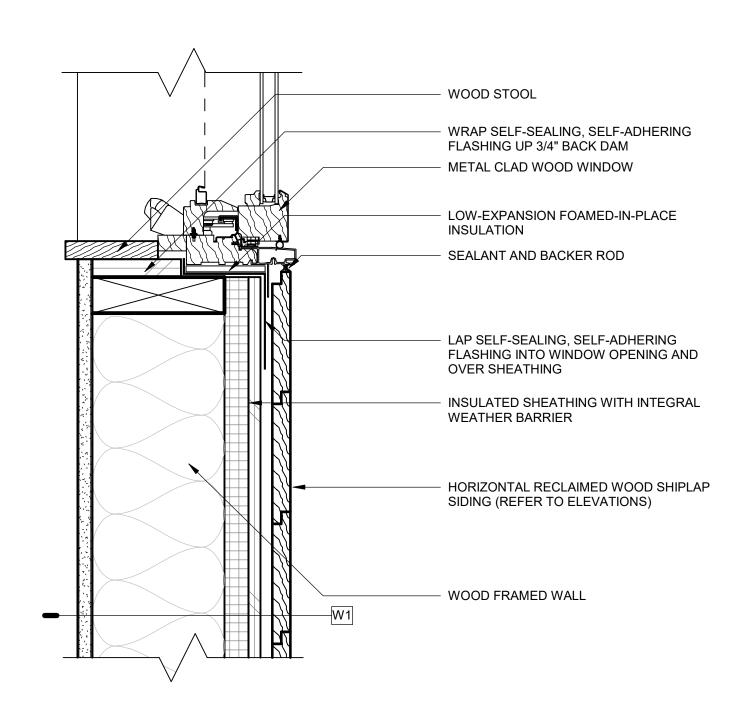
- PLACE SEALANT BEHIND NAILING FLANGE - LAP SELF-SEALING, SELF-ADHERING FLASHING OVER NAILING FLANGE

INSULATED SHEATHING WITH INTEGRAL WEATHER BARRIER

WOOD WALL FRAMING

HORIZONTAL WOOD SHIPLAP SIDING (REFER TO ELEVATIONS

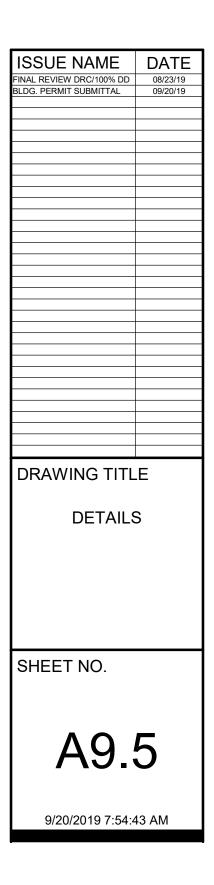
$\bigcirc \frac{\text{WINDOW JAMB AT HORIZONTAL SHIPLAP SIDING}}{3" = 1'-0"}$

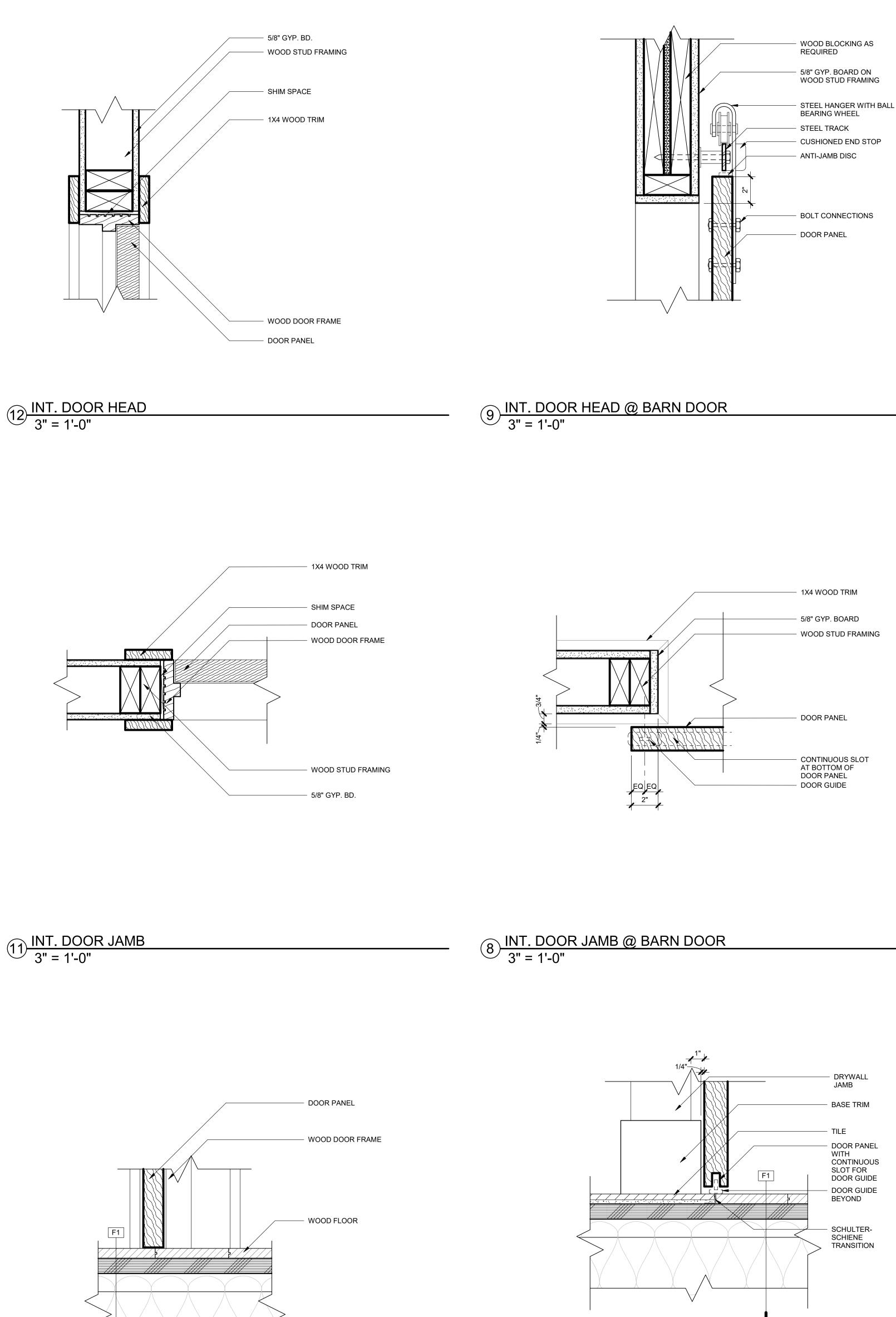


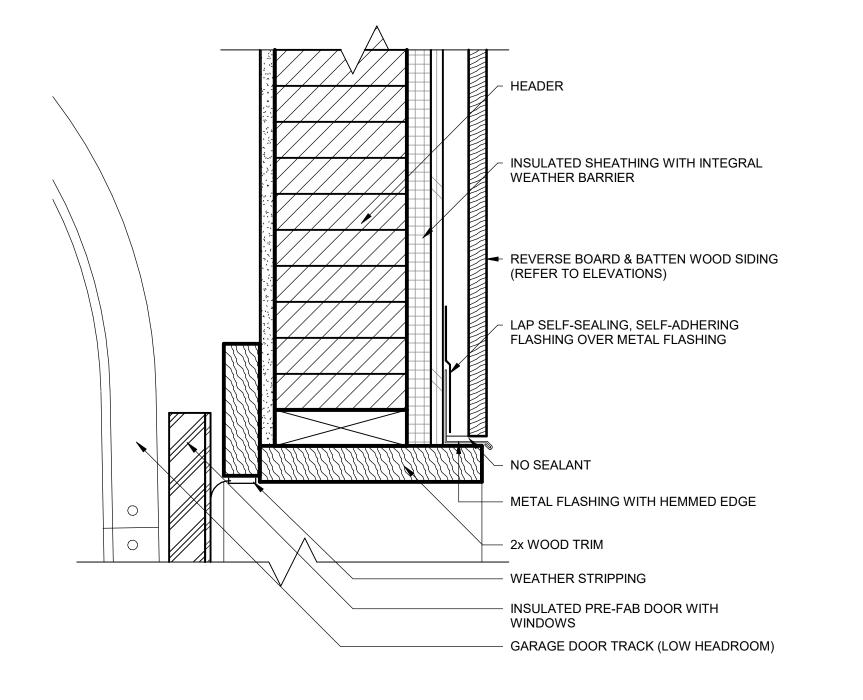
 $\underbrace{1}_{3"=1'-0"} WINDOW SILL AT HORIZONTAL SHIPLAP SIDING$



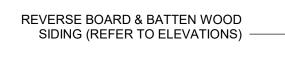










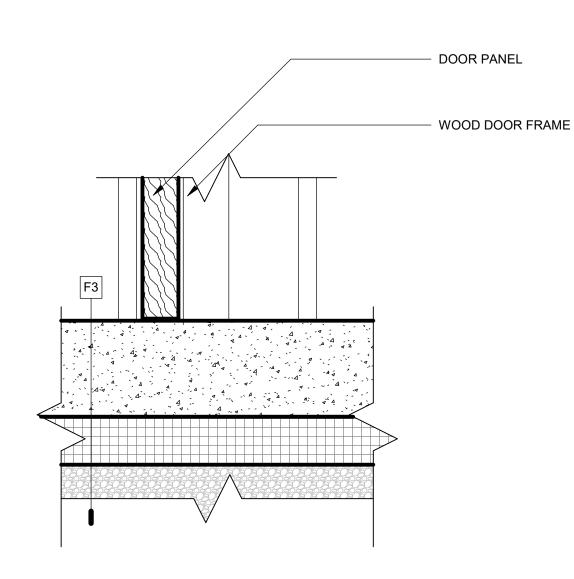


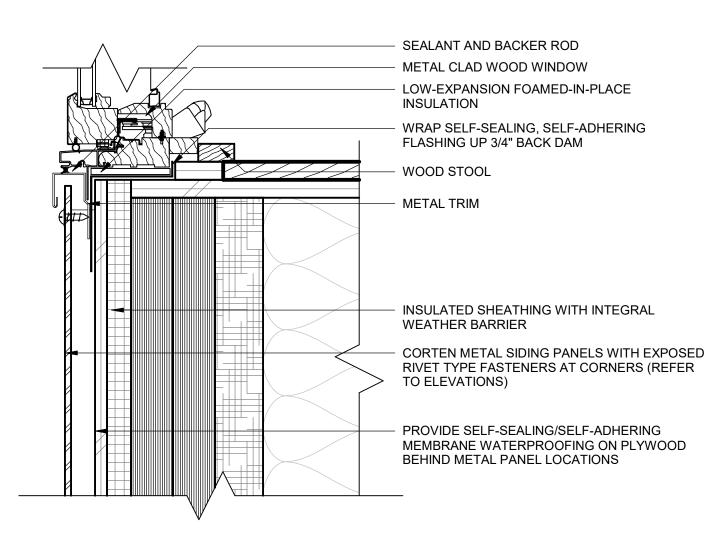


INSULATED PRE-FAB DOOR WITH WINDOWS

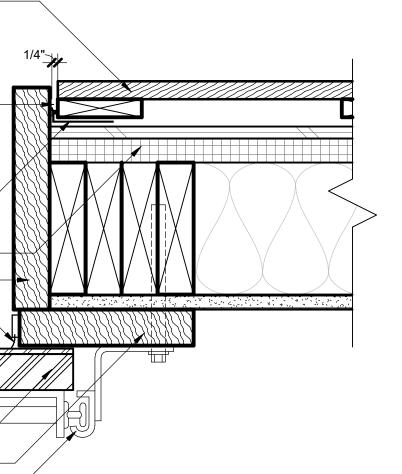
2x WOOD TRIM GARAGE DOOR TRACK

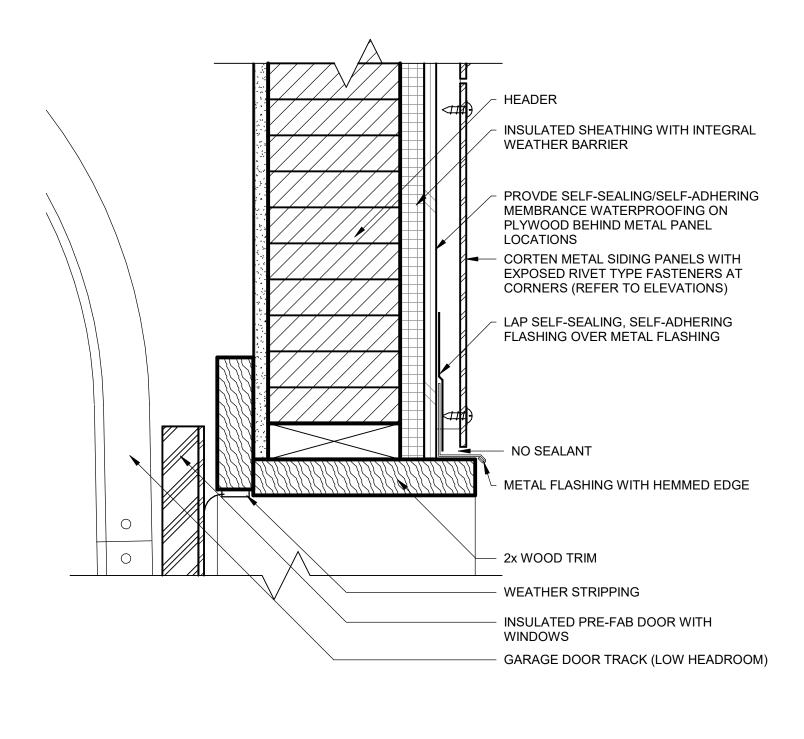






WINDOW SILL CORTEN METAL SIDING PANEL 2ND (3) FLOOR 3" = 1'-0"





GARAGE DOOR HEAD CORTEN METAL SIDING PANEL

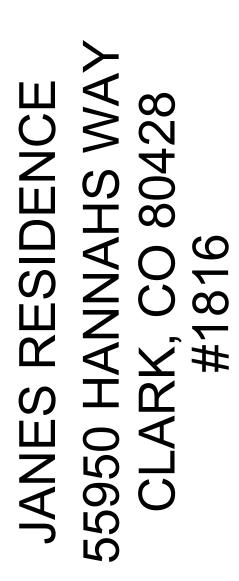
′ 3" = 1'-0"

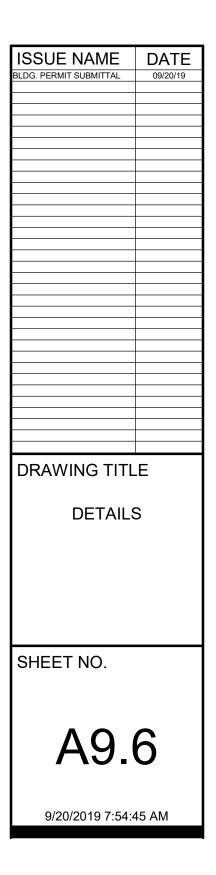
PROVIDE SELF-SEALING/SELF-ADHERING MEMBRANE WATERPROOFING ON PLYWOOD BEHIND METAL PANEL LOCATIONS CORTEN METAL SIDING PANELS WITH EXPOSED RIVET TYPE FASTENERS AT CORNERS (REFER TO ELEVATIONS) -JAMB FLASHING W1 SEALANT AND BACKER ROD PLACE SEALANT BEHIND NAILING FLANGE -LAP SELF-SEALING, SELF-ADHERING FLASHING ONTO WOOD TRIM INSULATED SHEATHING WITH INTEGRAL WEATHER BARRIER -2x WOOD TRIM WEATHER STRIPPING INSULATED PRE-FAB DOOR WITH WINDOWS 2x WOOD TRIM

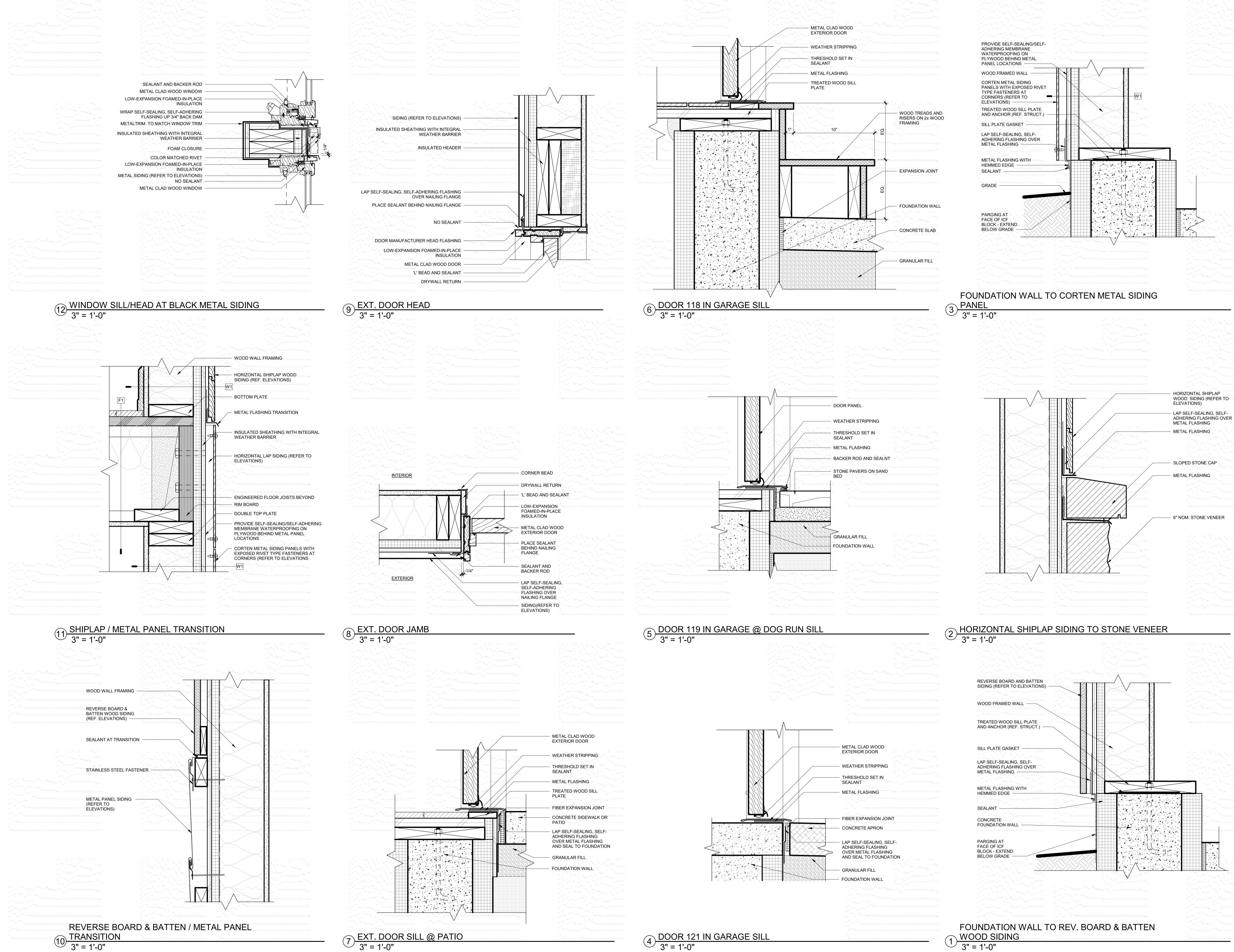
GARAGE DOOR TRACK

GARAGE DOOR JAMB CORTEN METAL SIDING PANELS 3" = 1'-0"









 $(4) \frac{\text{DOOR 121 IN GARAGE SILL}}{3" = 1'-0"}$

- WEATHER STRIPPING	
- THRESHOLD SET IN SEALANT	
- METAL FLASHING	
- BACKER ROD AND SEALN	The first second
	· · · · · · · · · · · · · · · · · · ·
GRANULAR FILL FOUNDATION WALL	

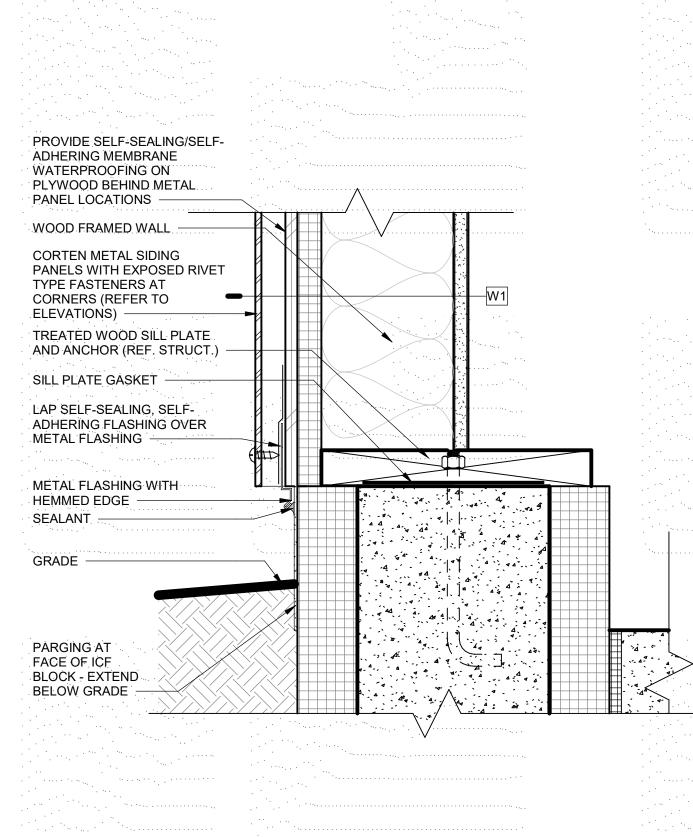
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- WEATHER STRIPPING	
- THRESHOLD SET IN SEALANT	
- METAL FLASHING	·····
	·······
- FIBER EXPANSION JO	INT

OVER METAL FLASHING AND SEAL TO FOUNDAT	
GRANULAR FILL	$(x,t,r)_{1}$

ONANOLANTILL	
FOUNDATION WALL	
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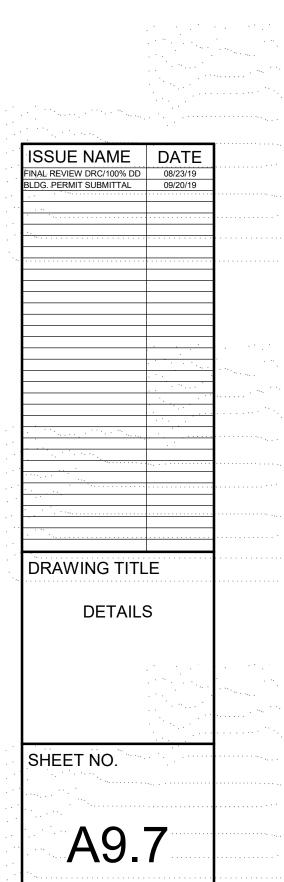




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Q $\odot \infty$ #



9/20/2019 7:54:49 AM

_____ 5/8" GYP. BD. FINISH -

> 5/8" GYP. BD. RETURN SEALANT WEATHERSTRIPPING

DOOR PANEL





DOOR PANEL

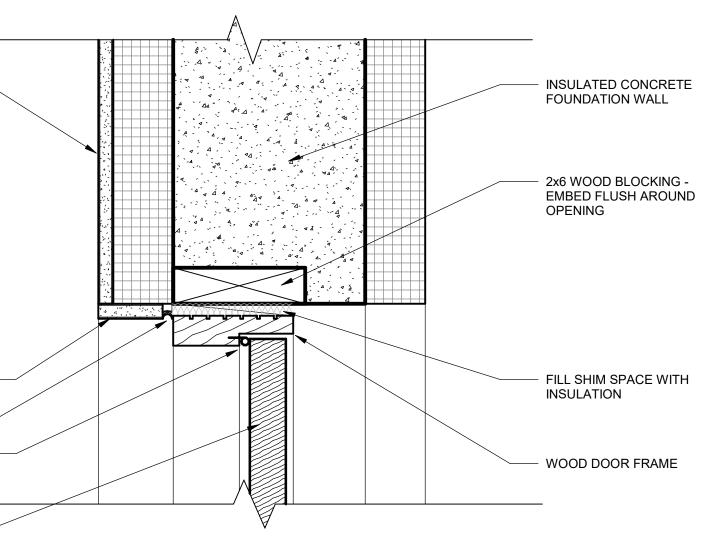
SEALANT -

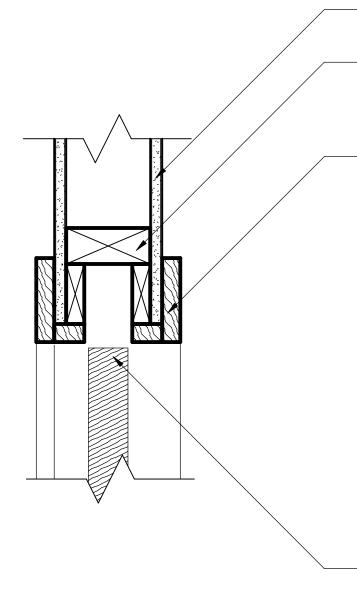
WOOD DOOR FRAME -

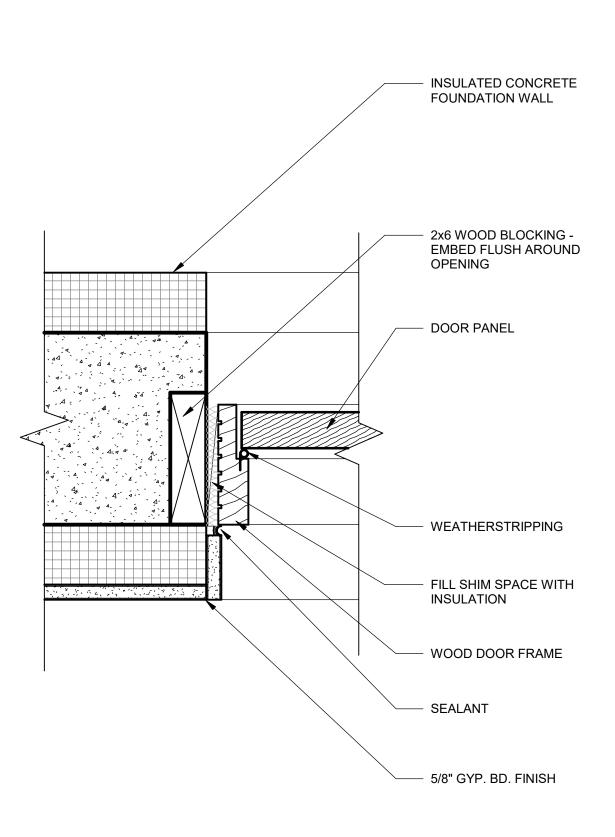
1x4 WOOD SILL

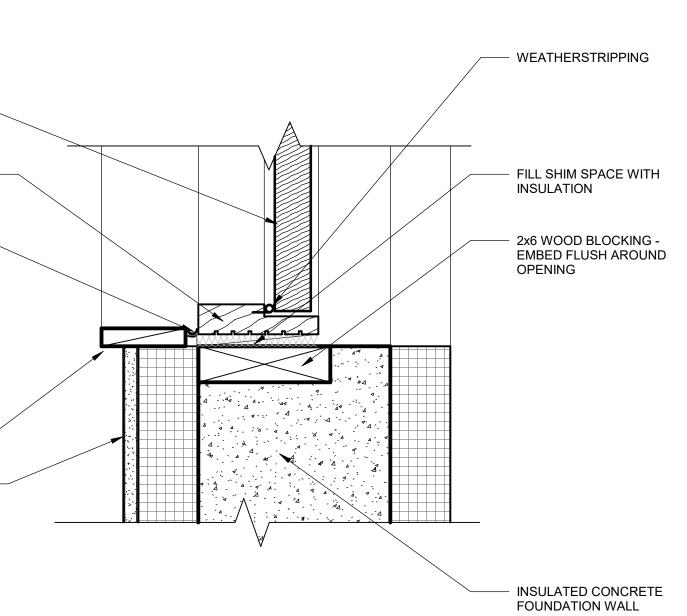
5/8" GYP. BD. FINISH -







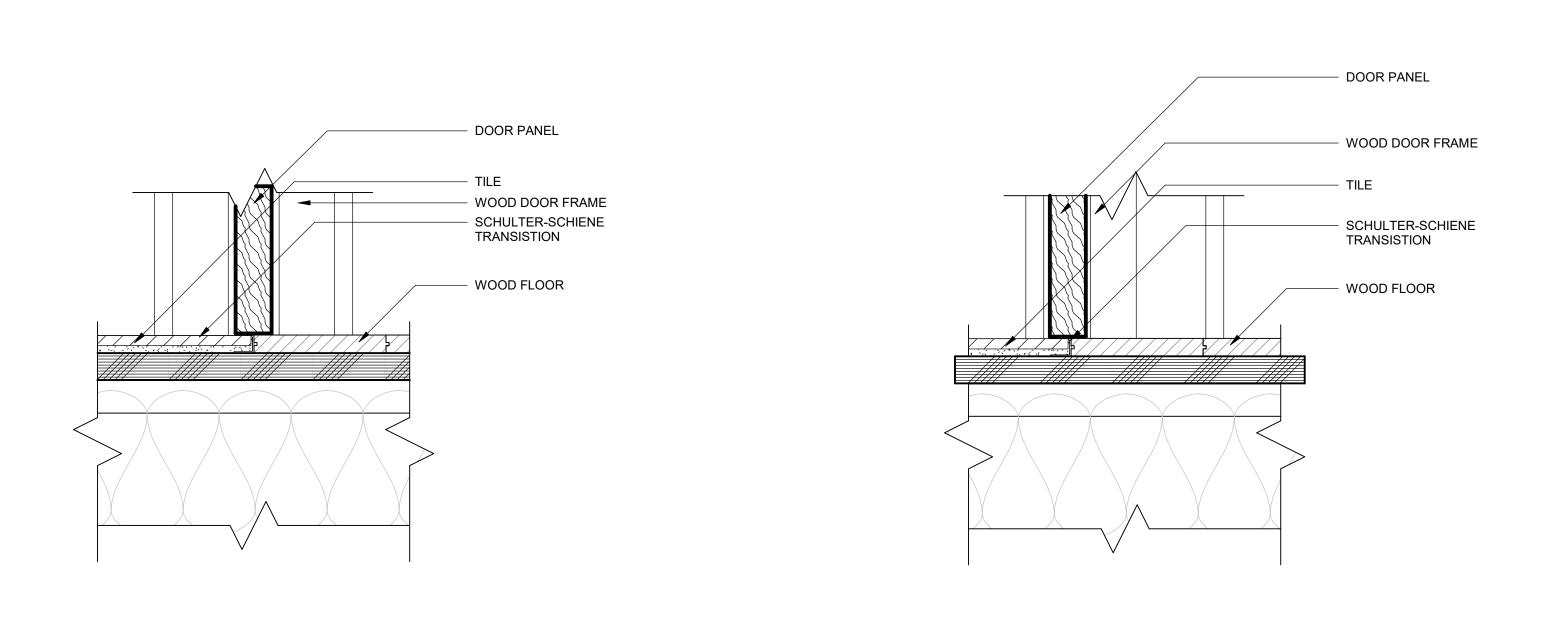




 $\bigcirc \frac{\text{INT. POCKET DOOR HEAD}}{3" = 1'-0"}$

1000000 $>\!\!<$ 10101010**1**010

 $(5) \frac{\text{INT. POCKET DOOR JAMB}}{3" = 1'-0"}$

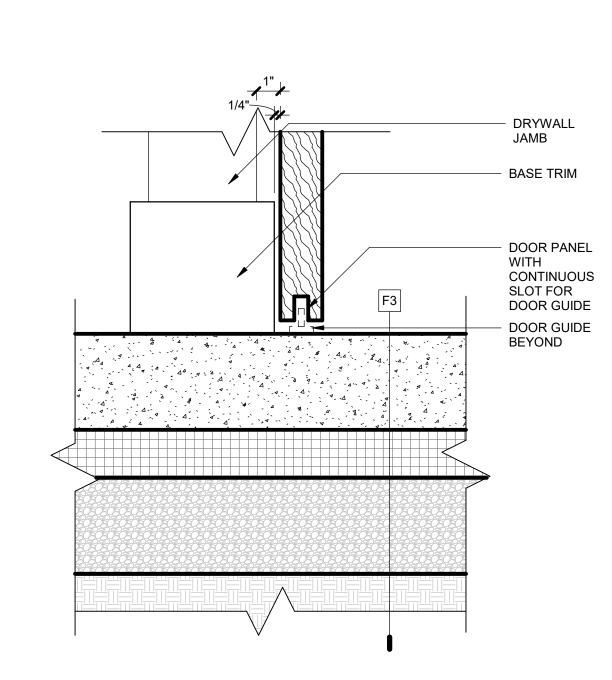


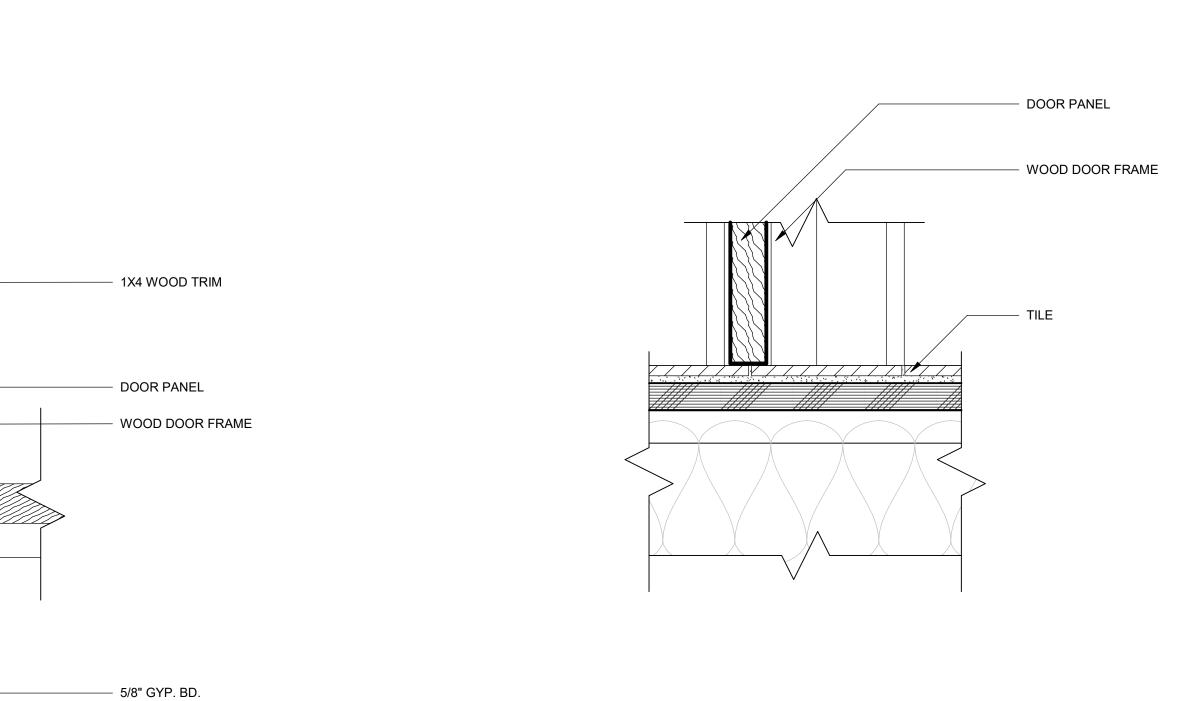
- 5/8" GYP. BD.

- WOOD STUD FRAMING

- 1X4 WOOD TRIM

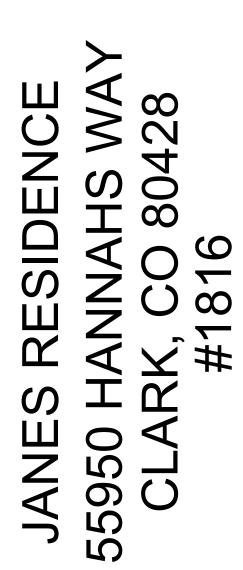
- DOOR PANEL

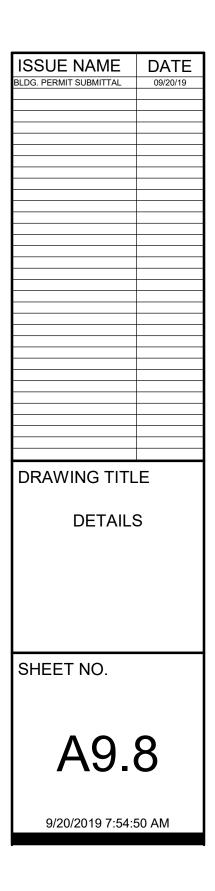


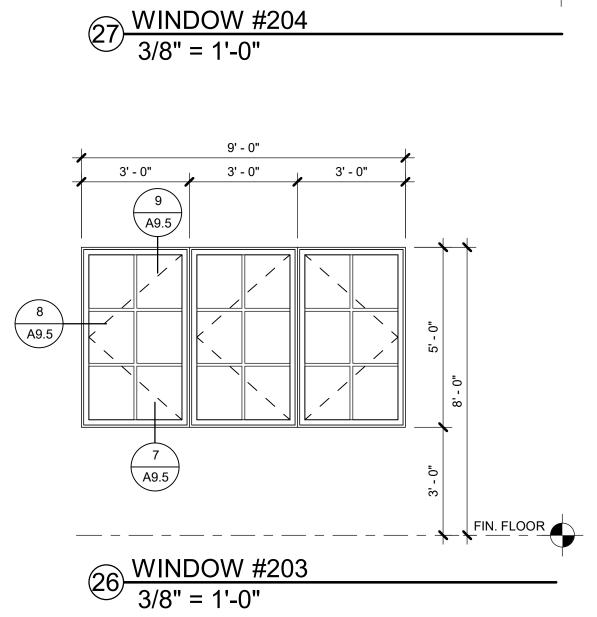


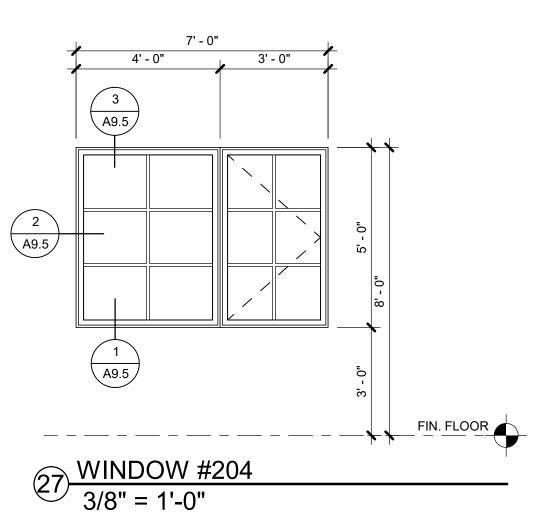
2 INT. DOOR SILL @ TILE 3" = 1'-0"

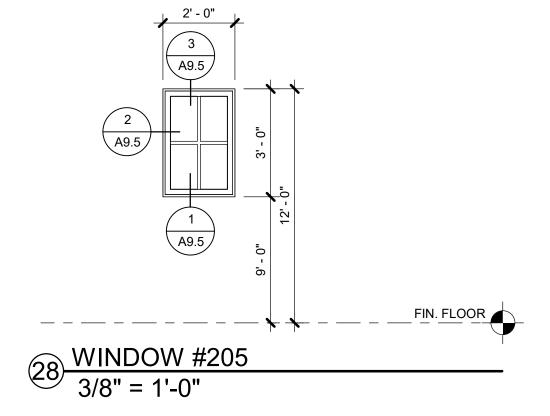


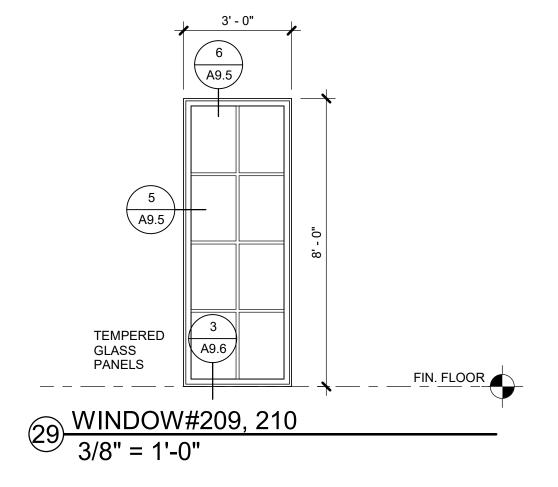


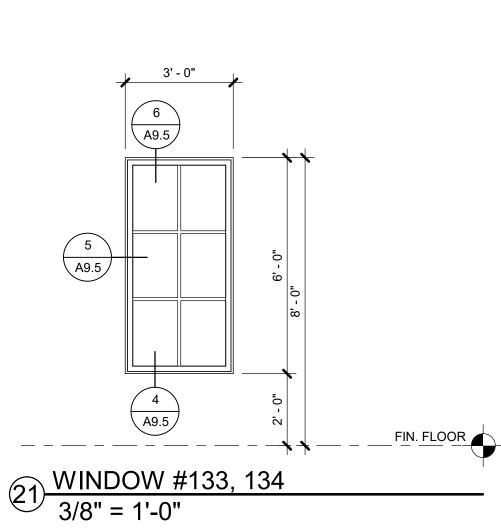


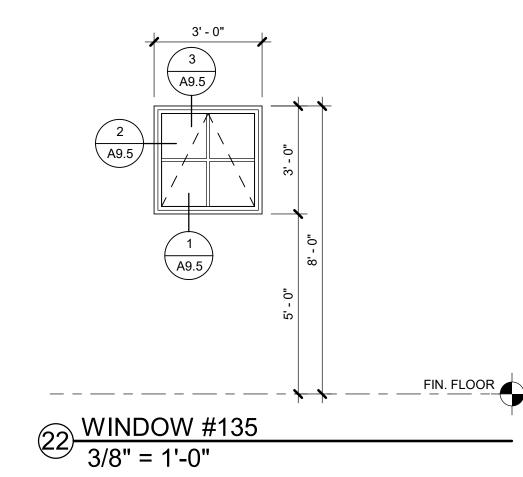


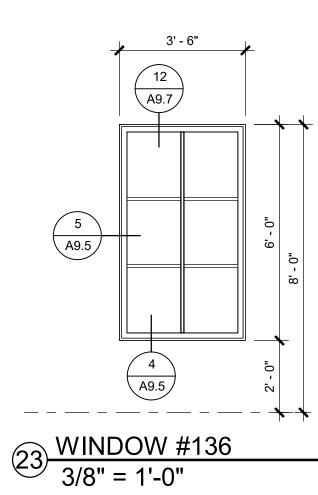


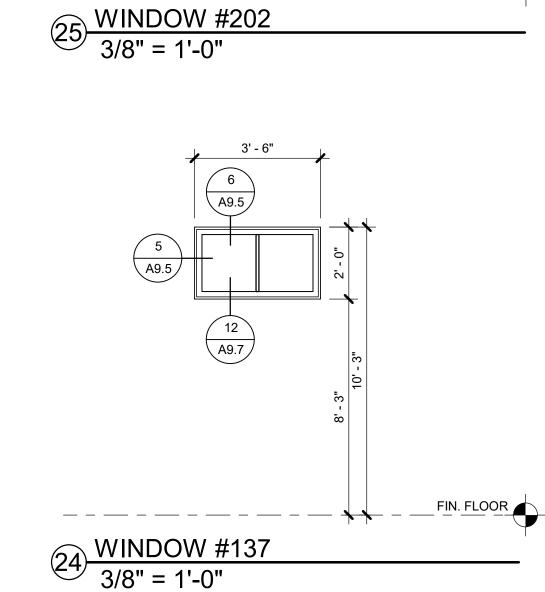


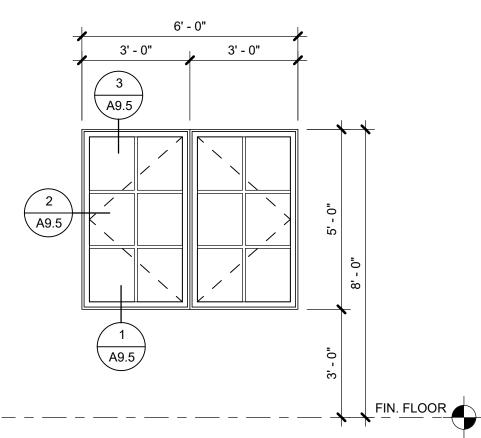


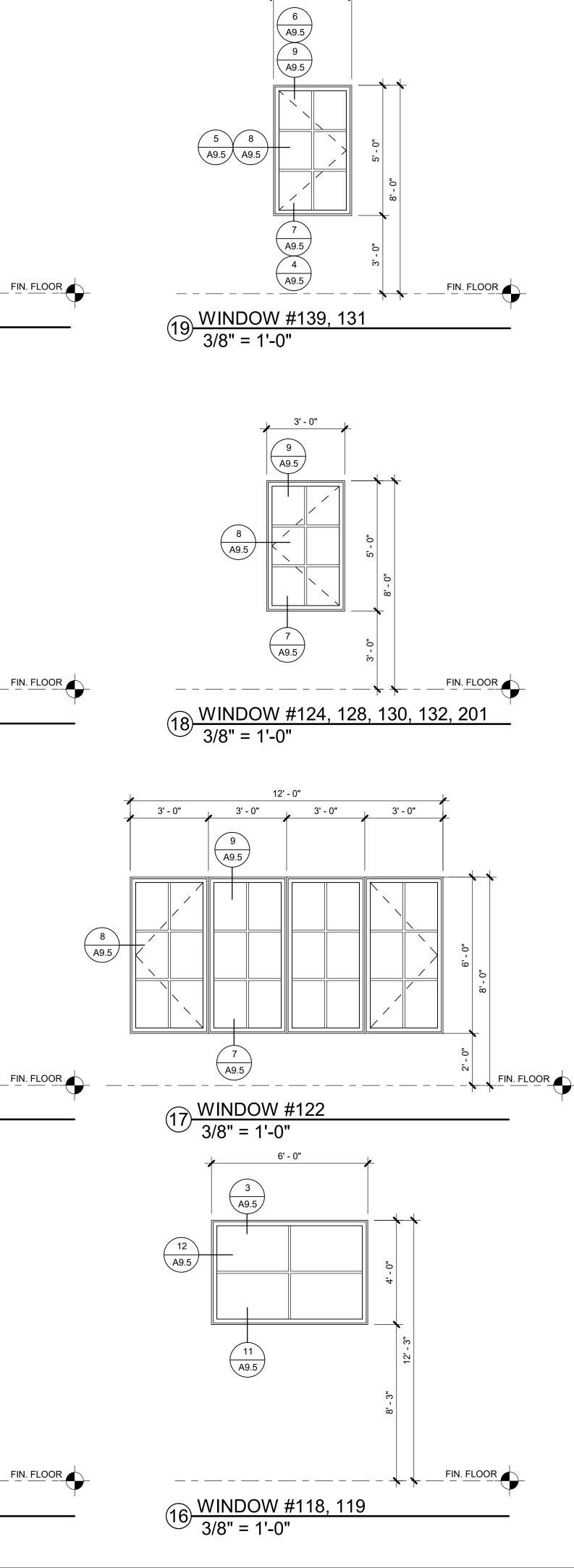












3' - 0"

9 A9.5 3 A9.5

A9.5

7 A9.5

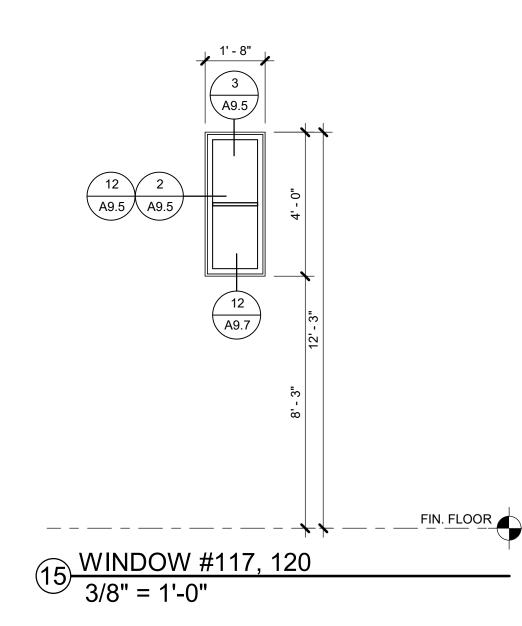
20 WINDOW #129, 206, 207, 208 3/8" = 1'-0"

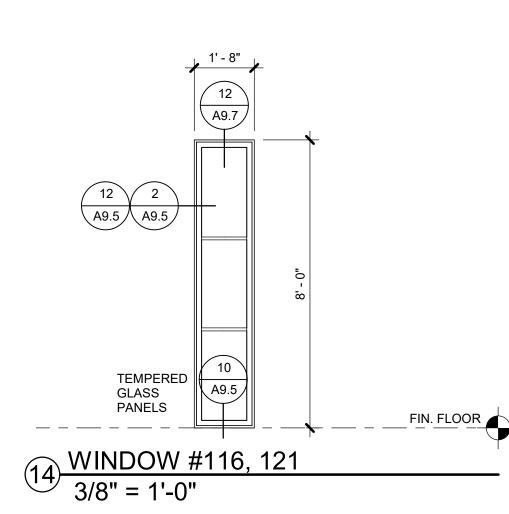
3' - 0"

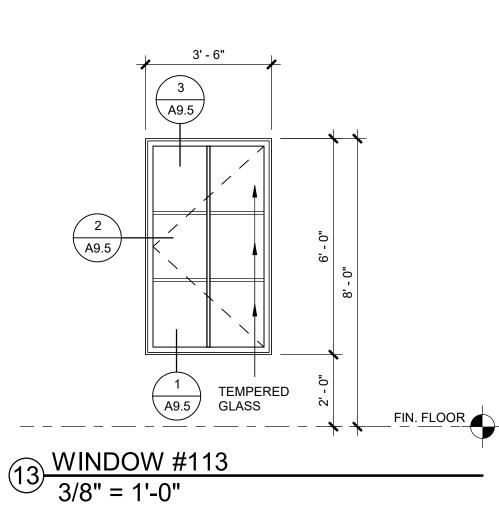
_____**-___**

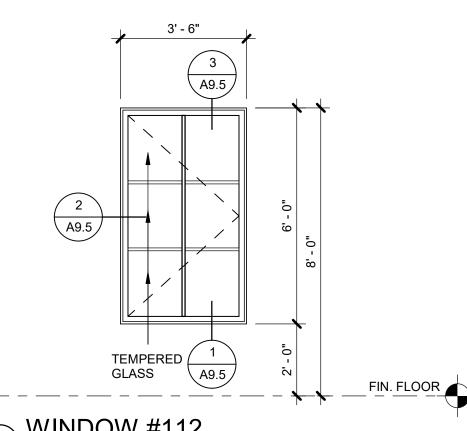
FIN. FLOOR

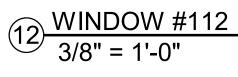
8 2 A9.5 A9.5

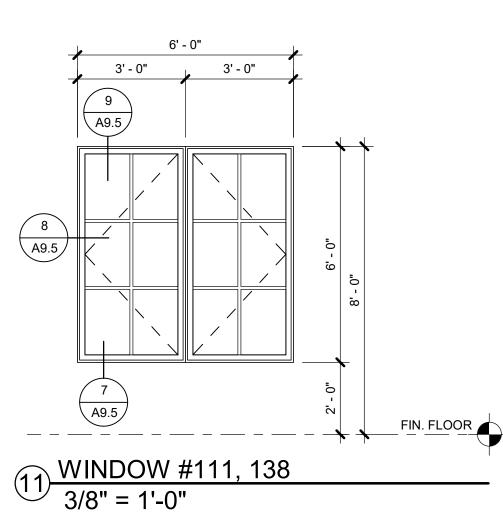


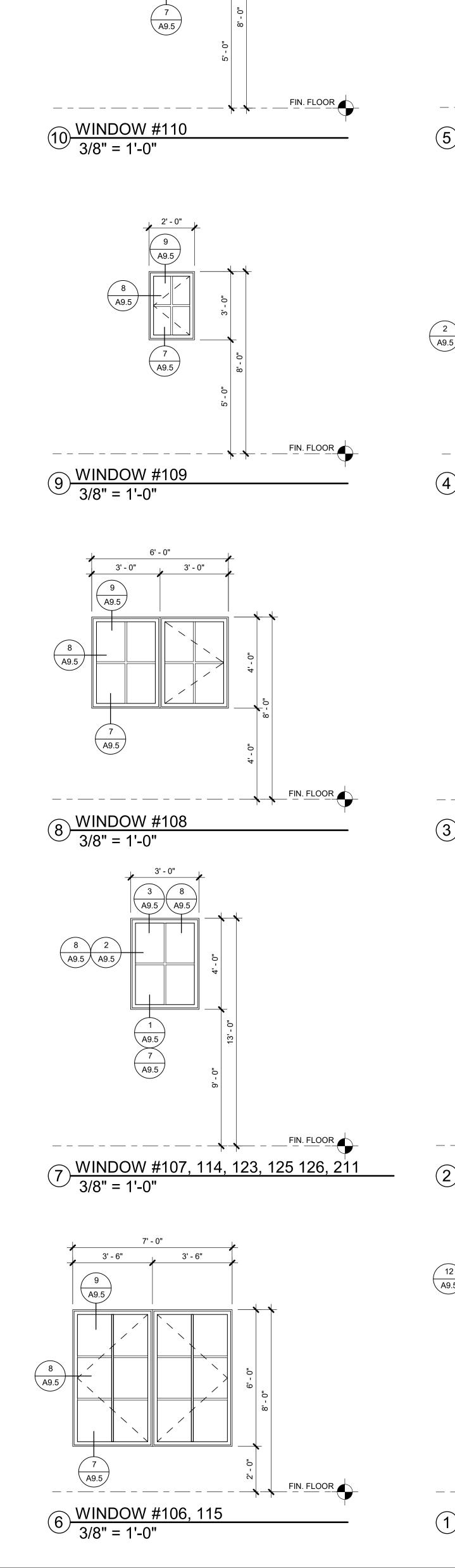








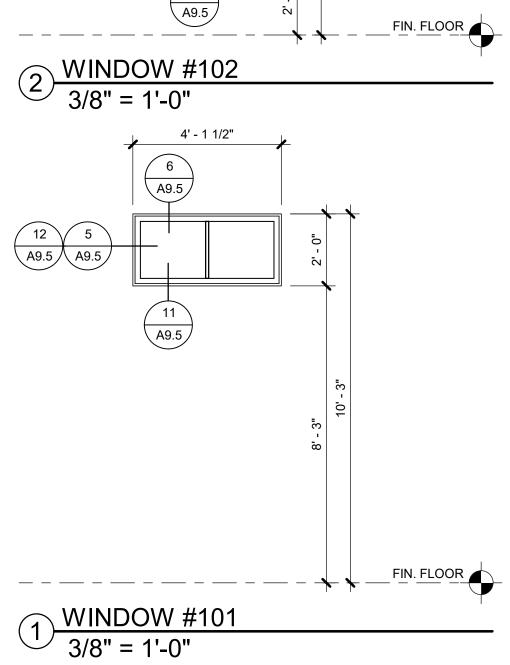


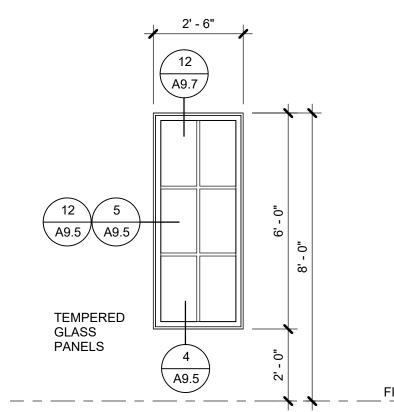


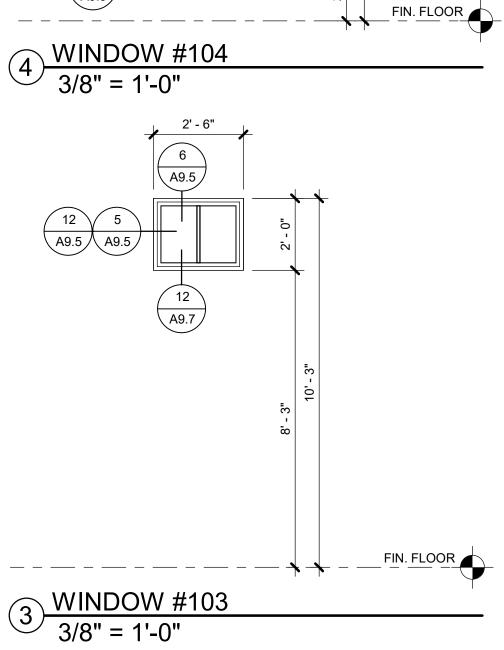
2' - 0"

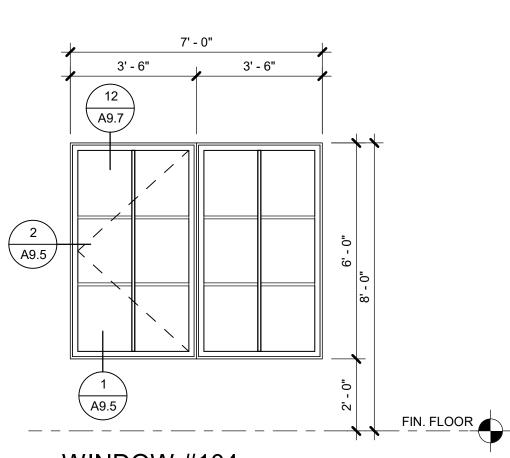
A9.5

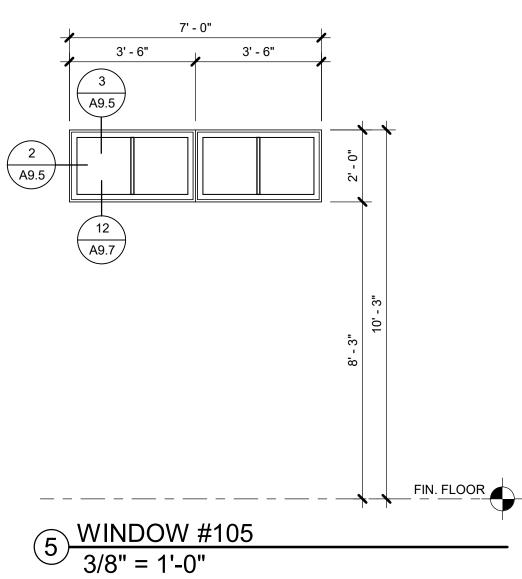
A9.5





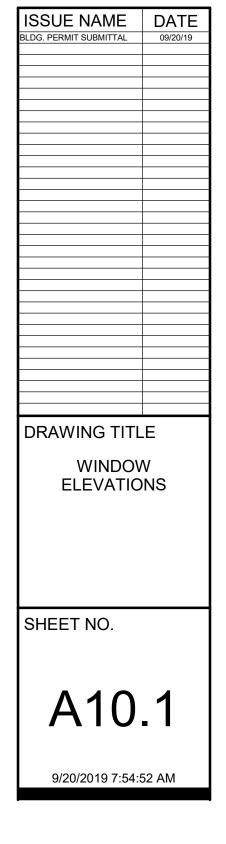


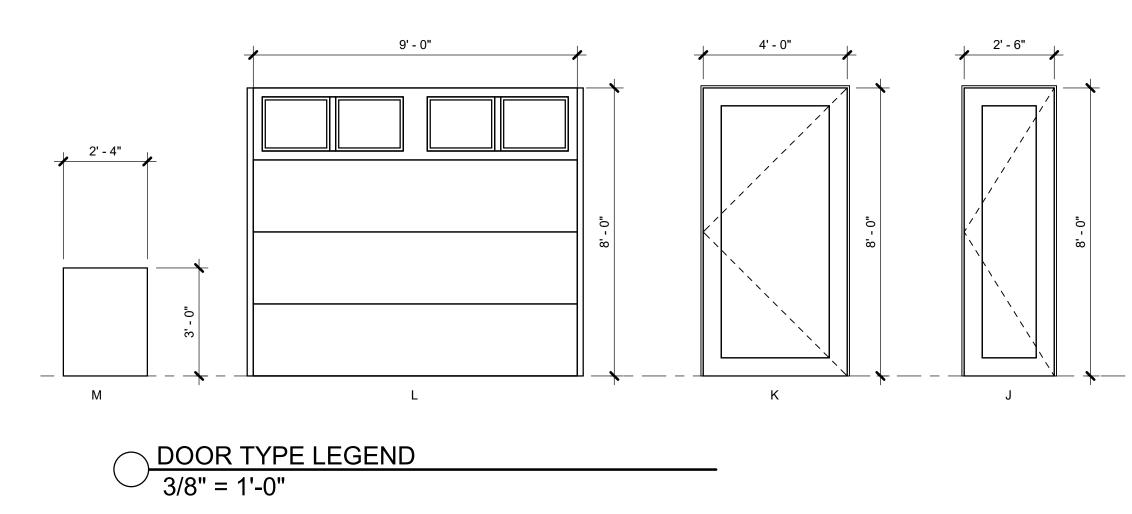


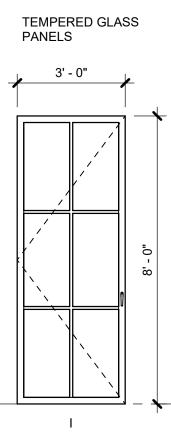


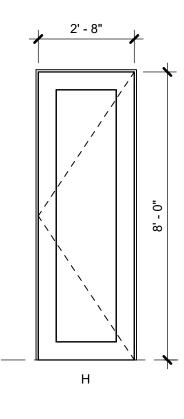


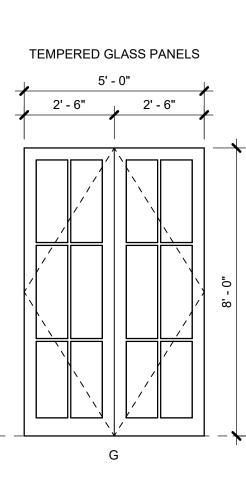


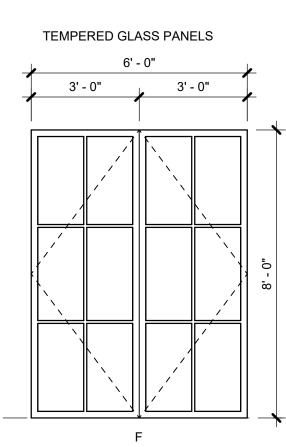


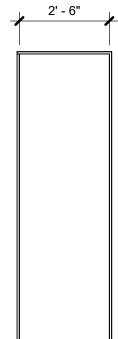




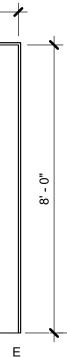


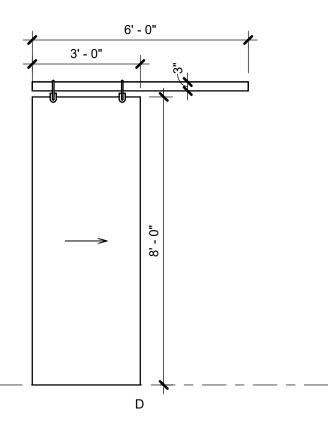


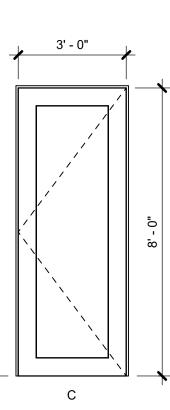


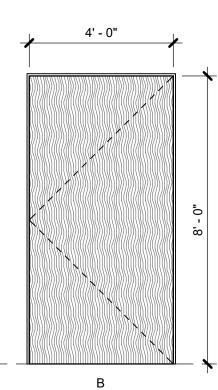


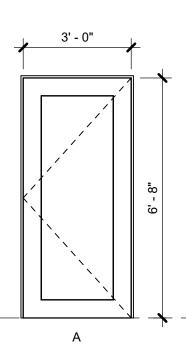
					DETAILS		
NUMBER	NOMINAL WIDTH	NOMINAL HEIGHT	TYPE	HEAD	JAMB	SILL	REMARKS
001	3' - 0"	6' - 8"	Α	12/A9.6	11/A9.6	4/A9.6	INTERIOR DOOR
002	2' - 4"	3' - 0"	R	9/A10.8	8/A10.8	7/A10.8	CRAWL SPACE ACCESS DOOR
101	4' - 0"	8' - 0"	В	9/A9.7	8/A9.7	8/A9.1	EXTERIOR CUSTOM WOOD AND METAL FRONT DOOR
102	3' - 0"	8' - 0"	С	12/A9.6	11/A9.6	1/A9.8	INTERIOR DOOR
103	3' - 0"	8' - 0"	С	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR, MASTER SUITE TBD
104	3' - 0"	8' - 0"	D	9/A9.6	8/A9.6	7/A9.6	BARN DOOR
105	2' - 6"	8' - 0"	E	6/A9.8	5/A9.8	4/A9.8	POCKET DOOR
106	6' - 0"	8' - 0"	F	9/A9.7	8/A9.7	7/A9.1	EXTERIOR DOUBLE FRENCH DOORS
107	6' - 0"	8' - 0"	F	9/A9.7	8/A9.7	7/A9.1	EXTERIOR DOUBLE FRENCH DOORS
108	6' - 0"	8' - 0"	F	9/A9.7	8/A9.7	7/A9.1	EXTERIOR DOUBLE FRENCH DOORS
109	5' - 0"	8' - 0"	G	9/A9.7	8/A9.7	7/A9.1	EXTERIOR DOUBLE FRENCH DOORS
110	2' - 8"	8' - 0"	Н	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR
111	3' - 0"	8' - 0"	I	9/A9.7	8/A9.7	7/A9.1	EXTERIOR SINGLE FRENCH DOOR
112	2' - 8"	8' - 0"	Н	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR
113	2' - 6"	8' - 0"	J	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR
114	2' - 6"	8' - 0"	J	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR
115	3' - 0"	8' - 0"	D	9/A9.6	8/A9.6	7/A9.6	BARN DOOR
116	3' - 0"	8' - 0"	С	12/A9.6	11/A9.6	1/A9.8	INTERIOR DOOR
117	2' - 8"	8' - 0"	Н	12/A9.6	11/A9.6	2/A9.8	INTERIOR DOOR
118	3' - 0"	8' - 0"	С	12/A9.6	11/A9.6	9/A9.8	INTERIOR DOOR (20 MINUTE RATED DOOR WITH WEATHERSTRIPPING
119	4' - 0"	8' - 0"	K	9/A9.7	8/A9.7	8/A9.8	EXTERIOR DOOR
120	9' - 0"	8' - 0"	L	2/A9.6	1/A9.6	9/A9.1	GARAGE DOOR
121	3' - 0"	8' - 0"	С	9/A9.7	8/A9.7	7/A9.8	EXTERIOR DOOR
122	9' - 0"	8' - 0"	L	6/A9.6	5/A9.6	9/A9.1	GARAGE DOOR
123	9' - 0"	8' - 0"	L	6/A9.6	5/A9.6	9/A9.1	GARAGE DOOR
124	4' - 0"	8' - 0"	0	9/A9.6	8/A9.6	3/A9.8	DOUBLE BARN DOOR
201	2' - 8"	8' - 0"	Н	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR
202	2' - 6"	8' - 0"	J	12/A9.6	11/A9.6	10/A9.6	INTERIOR DOOR
203	2' - 8"	8' - 0"	Н	12/A9.6	11/A9.6	1/A9.8	INTERIOR DOOR



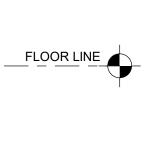




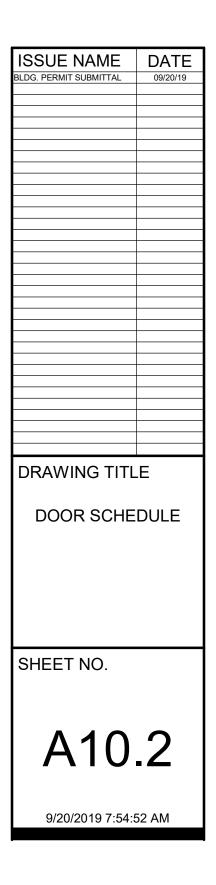












STRUCTURAL GENERAL NOTES

GOVERNING CODE: 2015 INTERNATIONAL RESIDENTIAL CODE (IRC) AND ALL LOCAL AMENDMENTS, EXCEPT AS NOTED: ANY STRUCTURAL ELEMENTS (IF ANY) NOT DESIGNED IN CONFORMANCE WITH THE IRC HAVE BEEN DESIGNED PER THE 2015 IBC PER IRC R301 1 3

D <u>ESIGN LOADS:</u> RISK CATEGORY:	II, Standard
ROOF LIVE LOADS:	
Ground Snow Load:	138 psf
Flat Roof Snow Load:	96.6+drift psf
LOOR LIVE LOADS:	
Residential:	40 psf
Exterior Decks:	Same as occupancy served
ROOF AND FLOOR DEAD LOADS:	
Roof - Asphalt Shingle:	20 psf
Floor - Carpet or Hardwood:	25 psf
VIND LOADS (ASCE 7 per IRC R301.2.1.1)	
Basic Wind Speed:	70 mph
Ultimate Wind Speed:	90 mph
Wind Exposure:	C
SEISMIC LOADS:	
Seismic Design Category:	Design not required per IRC R301.2.2 exception and IBC 1613.1 exception 1
OUNDATION DESIGN:	mmendations contained in soils investigation Report Number 19-1035 prepared by Western Slop
Geotech dated 9/4/2019.	
	chnical Engineer prior to placement of formwork or concrete. If different soil conditions exist the

Footings shall bear on the natural undisturbed soils or approved compacted structural fill. Exterior footings shall bear below frost depth; minimum frost depth shall be 4'-0" below adjacent exterior finished grade. Design of footings is based on:

Maximum allowable bearing pressure: 2500 psf Minimum dead load pressure: 750 psf.

EARTH RETAINING STRUCTURES:	
Earth equivalent fluid lateral pressure:	
Walls restrained at top (at rest):	70 pcf
Cantilevered walls (active):	60 pcf

Passive resisting:	250 pcf
Coefficient of sliding friction:	0.3
REINFORCED CONCRETE:	

<u>REINFORCED CONCRETE:</u>
Concrete design is based on the American Concrete Institute "Building Code Requirements for Reinforced Concrete" (ACI 318) and shall be
constructed in accordance with the "Standard Specifications for Structural Concrete" (ACI 301).
STRUCTURAL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES (normal weight concrete unless noted otherwise):
Minimum 28 day compressive strength (f ^r c) as follows:
Cement Type: I/II

Maximum Aggregate Size:	3/4"
Footings:	3,500 psi (Max W/C Ratio 0.52); Entrained Air 1.5% (± 1.5%); Slump 5 inches (± 1")
Walls:	4,000 psi (Max W/C Ratio 0.50); Entrained Air 3.0% (± 1.5%); Slump 4 inches (± 1")
Exterior Slabs-on-Grade (excludes flatwork):	3,500 psi (Max W/C Ratio 0.45); Entrained Air 6.0% (± 1.5%); Slump 4 inches (± 1")
Interior Slabs-on-Grade:	3,500 psi (Max W/C Ratio 0.50); Entrained Air 3.0% (± 1.5%); Slump 4 inches (± 1")
Reinforcing steel shall be fabricated and placed in	accordance with ACI 315 "Details and Detailing of Concrete Reinforcement."
When cold weather conditions exist, place and cu	re concrete in accordance with ACI 306.

Welded wire fabric shall conform to ASTM A185. Deformed reinforcement shall be domestic new billet steel conforming to ASTM A615, Grade 60 including stirrups and ties, except that reinforcing which is required to be welded shall conform to ASTM A706. Unless otherwise noted on the structural drawings, lap bars 50 diameters (50*Bar Diameter minimum).

Reinforcing at all abutting concrete (including footings) shall be continuous through or around all corners and intersections OR use matching corner bars of equal size and spacing to reinforcing in the abutting members. Install 2-#5 bars (minimum) around all sides of all openings in concrete and extend 2'-0" past edges of openings, unless otherwise noted. In continuous members, splice top bars at mid-span between supports and splice bottom bars over supports.

Form intermittent shear keys at all construction joints and as shown on the structural drawings. Unless otherwise noted on the drawings, minimum concrete cover over reinforcing shall be as follows: Unformed surface cast against and permanently exposed to earth: 3"

Formed surface exposed to earth or weather: 1-1/2" Formed surface not exposed to weather or in contact with ground: 3/4"

Install chairs, bolsters, additional reinforcement, and accessories necessary to support reinforcement at position shown on drawings. Support of reinforcement on wood, brick, or other unacceptable materials shall not be permitted.

Keep reinforcement clean and free of dirt and oil. Oil forms prior to placing reinforcement. Fiber admixture shall be 100% virgin polypropylene, fibrillated fibers, type 111 4.1.3, performance level one, per ASTM C1116. Properly place, accurately position and maintain securely in place all embedded items prior to and during concrete placement. Anchor bolts and rods for beam and column-bearing plates shall be placed with setting templates.

Unless otherwise shown in the architectural drawings, provide 3/4" chamfers at all exposed concrete edges.

STRUCTURAL STEEL:

Structural steel shall be detailed, fabricated and erected in accordance with the "Specification for Structural Steel Buildings" (AISC 360) and the "Code of Standard Practice for Steel Buildings and Bridges" (AISC303) by the American Institute of Steel Construction (AISC). All structural steel shall conform to the ASTM Standards and grades indicated below, unless noted otherwise on the drawings or details. Structural steel wide flange beams and WTs: ASTM A992, 50 ksi yield Other rolled shapes, including plates, channels, and angles: ASTM A36, 36 ksi yield. Hollow structural section (HSS) rectangular shapes: ASTM A500, Grade B, 46 ksi vield

ASTM A500, Grade B, 42 ksi yield HSS round shapes: Unless otherwise noted, framed beam connections shall be bearing-type with 3/4" diameter, snug tight, ASTM A325 bolts, detailed in conformance with the structural drawings and the "Steel Construction Manual" by the AISC. Install bolts in accordance with AISC's "Specification for Structural Joints Using ASTM A325 or A490 bolts."

All beams shall have full depth web stiffeners each side of webs above and below columns (1/4" plate or as noted). Anchor rods shall conform to ASTM F1554, Grade (36, 55, and/or 105) as noted on the structural drawings with weldability supplement S1. Headed anchor studs (HAS) shall conform to ASTM A108 and shall be connected to structural steel with equipment approved by the stud manufacturer according to the stud manufacturer's recommendations.

Welding shall be done by a certified welder in accordance with the AISC documents listed above, the American Welding Society (AWS) D1.1: 2006 Structural Welding Code, and the recommendations for use of E70XX electrodes. Where not specifically noted, minimum weld shall be 3/16" fillet by length of contact edge.

All post-installed anchors shall have current International Code Council Evaluation Service (ICC-ES) reports and shall be installed in accordance with the manufacturer's requirements. Expansion anchors shall be approved "wedge" type unless specifically noted to be "sleeve" type as noted on the structural drawings. Chemical anchors shall be approved epoxy or similar adhesive type as appropriate for installation in solid and non-solid base materials.

Grout beneath column base and beam bearing plates shall have a minimum 28-day, compressive strength of 5,000 psi and shall be nonshrink, non-metallic, and tested in accordance with ASTM C1107.

STRUCTURAL WOOD & TIMBER:

Design is based on ANSI/AF&PA NDS "National Design Specification for Wood Construction with Supplement: Design Values for Wood Construction" and ANSI/AF&PA SDPWS "Special Design Provisions for Wind and Seismic." 2x framing lumber shall be S4S Doug-Fir No. 2 and better unless noted otherwise.

All lumber shall be 19% or less maximum moisture content, unless noted otherwise. Solid timber beams and posts shall be Kiln Dried Douglas Fir-Larch No. 1.

2x stud bearing walls shall be 2x6 @ 16" (UNO) Doug-Fir Stud Grade or better. 2x top and bottom plates shall be Doug-Fir No. 2 or better.

Use of wood bearing walls shown on drawings with laterally unsupported heights in excess of that shown in IBC Table 2308.9.1 have been justified by Anthem's analysis. Fasteners for use with treated wood shall comply with 2015 IRC.

Wood in contact with concrete shall be pressure-treated Douglas Fir-Larch or Southern Yellow Pine. Preservative treated wood shall be treated in accordance with AWPA U1 and AWPA M4.

Conventional light framing shall comply with IRC Sections R502, R602, and R802.

Minimum nailing shall be provided as specified in IRC Table R602.3(1) "Fastener Schedule for Structural Members." Metal framing anchors shown or required, shall be Simpson Strong-Tie or equal code approved connectors and installed with the number and type of nails recommended by the manufacturer to develop the maximum rated capacity. Note that heavy-duty hangers and skewed hangers may not be stocked locally and require special order from the factory. Glue wood nailer plates to steel beams and attach with either 1/2"Ø bolts @ 32" o.c., staggered or 0.145"Ø powder actuated drive pins @ 16"

o.c. staggered. Width of nailer plate shall match beam width + 1/8" min (1/4" max) overhang each side. Lead holes for lag screws shall be 40%-70% of the shank diameter at the threaded section and equal to the shank diameter at the unthreaded section per NDS Section 11.1.3. Connector bolts and Lag screws shall conform to ANSI/ASME B18.2.1 and ASTM SAE J429 Grade 1.

Nails and Spikes shall conform to ASTM F1667. Wood Screws shall conform to ANSI/ASME B18.6.1.

WOOD FRAMING NOTES:

Install solid blocking between joists under jamb studs of openings. Columns must have a continuous load path to foundation.

Unless noted otherwise, install two lengths of solid blocking x joist depth x 12 inches long in floor framing under column loads. Built-up stud columns shall consist of 2x4, 2x6, or 2x8 studs with number of laminations noted on plan and each lamination shall be nailed together with (2) rows of 12d gun nails (0.131"Ø x 3 1/4") @ 6" full height of column. Do not splice laminations.

All beams and trusses shall be braced against rotation at points of bearing. Unless noted otherwise, lower chord of gable end trusses shall be anchored to wall plate with framing anchors at 4'-0" spacing and laterally braced to roof framing at 8'-0" spacing. Provide continuous wall studs each side of openings equal to one-half or greater the number of studs interrupted by opening unless noted

otherwise. All wall studs shall be continuous from floor to floor or from floor to roof.

Provide solid blocking or rim joists at all joist supports and joist ends. Sole plate at all perimeter walls and at designated shear walls shall be nailed with (4) 0.131"Øx3" nails at 16" minimum.

WOOD SHEATHING:

Plywood and Oriented Strand Board (OSB) floor, roof, and wall sheathing shall be APA rated with stamp including APA trademark and panel span rating Minimum Floor Sheathing: 23/32" APA Sturd-I-Floor rated 24 inch o.c. tongue & groove, glued and nailed.

Minimum Roof Sheathing: 5/8" OSB or CDX plywood, APA 40/20, nailed. Minimum Wall Sheathing: 7/16" OSB or CDX plywood, APA 24/16, blocked and nailed.

All roof rafters, joists, trusses, beams shall be anchored to supports with metal framing anchors.

Nail wall sheathing with minimum 8d gun or sinker nail @ 4" at panel edges, and @ 8" at intermediate framing except as noted. Nail ZIP sheathing with minimum 10d gun or sinker nail @ 4" at panel edges, and @ 8" at intermediate framing except as noted. Block and nail ALL edges between studs. Minimum (3) 8d nails per stud. Nail all plates using panel edge nail spacing indicated.

Sheathe all exterior walls. Sheathe interior walls as shown on the drawings. Sheathing shall be continuous from bottom plate to top plate. cut in "L" and "T" shapes around openings. Lap sheathing over rim joists a minimum 4" at all floors to tie upper and lower stud walls together. Minimum height of sheathing panels shall be 16" to ensure that plates are tied to studs.

Machine Applied Nailing (i.e. Gun Nailing): The use of machine applied nailing is subject to satisfactory jobsite demonstration and the approval by the project structural engineer. The approval is subject to continued satisfactory performance. If nail heads penetrate the outer ply more than would be normal for a hand hammer or if minimum allowable edge distances are not maintained the performance will be deemed unsatisfactory.

PLANT FABRICATED / PRE-ENGINEERED WOOD FRAMING:

I-series roof and floor joists shall be manufactured by iLevel Trus Joist with structural wood flanges and webs designed for structural capacities and design provisions according to ASTM D 5055. Substitution of equivalent series by other manufacturer is acceptable with engineer approval I-series roof and floor joists shall be installed per the manufacturer's recommendations. Do not cut or notch chords in any manner. Holes in

webs shall not exceed manufacturer's published limit criteria. Members noted as LVL (Laminated Veneer Lumber) on plan shall be 1 3/4" wide x depth indicated, plant-fabricated, and have the following minimum allowable design values:

 $F_b = 2600 \text{ psi}$ $F_v = 285 \text{ psi}$ $F_{c\parallel} = 2510 \text{ psi}$ $F_{c\perp} = 750 \text{ psi}$ E = 2000 ksiMembers noted as LSL (Laminated Strand Lumber) on plan shall be plant-fabricated and have the following minimum allowable design values: 1 1/2" $F_b = 1700 \text{ psi}$ $F_v = 400 \text{ psi}$ $F_{c||} = 1400 \text{ psi}$ $F_{c\perp} = 680 \text{ psi}$ E = 1300 ksi1 3/4" F_b = 2325 psi F_v = 310 psi $F_{c\parallel}$ = 2325 psi $F_{c\perp}$ = 800 psi E = 1550 ksi

Bridging and blocking shall be installed according to the fabricator's requirements.

OPEN-WEB WOOD TRUSSES: Manufacture and installation of metal plated wood trusses shall comply with ANSI/TPI 1 "National Design Standard for Metal-Plate-Connected Wood Truss Construction," BCSI (Building Component Safety Information) "Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses," and DSB-89 "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses."

Pre-engineered, prefabricated trusses shall be designed by an engineer registered in the State of Colorado, and shall comply with code requirements. Trusses shall be designed to support the full dead loads and the superimposed design loads noted above or on the drawings. Stresses shall not exceed those listed in the National Design Specification for Wood Construction with Supplement (AF&PA NDS).

The fabricator shall determine truss web arrangements and member forces. Truss to truss connections specified shall be by truss supplier, unless specifically noted on the drawings. Unless otherwise indicated, trusses shall be designed for perpendicular to grain bearing on Hem Fir plates (405 psi).

End grain bearing is not allowed unless accepted in writing by ANTHEM.

Design truss for bearing blocks or truss bearing enhancers as required to compensate for overstresses. specify size, species, and nailing for

bearing blocks.

Truss fabricator shall specify all floor and roof truss bracing and bridging. Calculations and shop drawings, including member sizes, lumber species and grades, and substantiating data for connector capacities, shall be submitted to the architect and engineer for review prior to fabrication.

SHOP DRAWINGS:

The structural drawings are copyrighted and shall not be copied for use as erection the basis for shop drawings requires prior approval by Anthem, a signed releas subcontractors, and deletion of Anthem's name and logo from all sheets so used.

The general contractor shall submit in writing any requests to modify the structural drawings or project specifications. All shop and erection drawings shall be checked and stamped (after having been checked) by the general contractor prior to submission for structural engineer's review; shop drawing submittals not checked by the general contractor prior to submission to the structural engineer will be returned without review.

- Furnish two (2) prints of shop and erection drawings to the structural engineer for review prior to fabrication for: reinforcing steel, structural steel, plant fabricated wood joists,
- pre-engineered wood trusses.

Submit in a timely manner to permit 10 working days for review by the structural engineer. Shop drawings submitted for review do not constitute "request for change in writing" unless specific suggested changes are clearly marked, in any event, changes made by means of the shop drawing submittal process become the responsibility of the one initiating the change.

STRUCTURAL ERECTION AND BRACING REQUIREMENTS: The structural drawings illustrate and describe the completed structure with elements in their final positions, properly supported, connected,

and/or braced.

- conditions unless otherwise indicated. Although due diligence has been applied to make the drawings as complete as possible, not every detail is illustrated and not every exceptional condition is addressed. All proprietary connections and elements shall be installed in accordance with the manufacturers' recommendations. All work shall be accomplished in a workmanlike manner and in accordance with the applicable codes and local ordinances.
- The general contractor is responsible for coordination of all work, including layout and dimension verification, materials coordination, shop drawing review, and the work of subcontractors. Any discrepancies or omissions discovered in the course of the work shall be immediately reported to the architect and structural engineer for resolution. Continuation of work without notification of discrepancies relieves the architect and structural engineer from all consequences.
- Unless otherwise specifically indicated, the structural drawings do not describe methods of construction. The general contractor, in the proper sequence, shall perform or supervise all work necessary to achieve the final completed structure, and to protect the structure, workmen, and others during construction. Such work shall include, but not be limited to temporary bracing, shoring for construction equipment, shoring for excavation, formwork, scaffolding, safety devices and programs of all kinds, support and bracing for cranes and other erection equipment.
- Do not backfill against basement or retaining walls until supporting slabs and floor framing are in place and securely anchored, unless adequate temporary bracing is installed. Temporary bracing shall remain in place until all floors, walls, roofs and any other supporting elements are in place.
- The architect and structural engineer bear no responsibility for the above items, and observation visits to the site do not in any way include inspections of these items. These plans have been engineered for construction at one specific building site. Builder assumes ALL responsibility for use of these plans at
- PRECAUTIONARY NOTES ON STRUCTURAL BEHAVIOR: Interior architectural finish detailing must accommodate the relative differential movements of supporting structural elements.
- Where the roof framing element spans are long, applied loading will naturally cause substantial deflection. Interior elements hung from the roof structure will deflect with the roof. The floor is a floating concrete slab-on-grade and may experience movements independent of the structural foundations. Interior elements
- supported on the slab-on-grade floor will move with the floor. Interior elements supported on foundations and columns will not experience similar or measurable movements.
- Exterior/perimeter wall assemblies hung from the edge of the building structure will be directly affected (to some degree) by changes in external temperature and floor deflection.
- Exterior/perimeter and interior architectural finish details should allow for relative movements between elements with different support conditions.
- The foundation design shown assumes that the owner/builder is aware of the presence of expansive soils, and that he has read the previously referenced soils report. Use of these plans is indication that the owner/builder accepts the risks associated with building on this site, especially those related to slab on grade construction in finished areas. Anthem, LLC will not be held liable for damages caused by slab movement.

LETTERS OF CONSTRUCTION COMPLIANCE:

The general contractor shall determine from the local building authority, at the time the building permit is obtained, whether any letters of construction compliance will be requested from the structural engineer. The contractor shall notify the structural engineer of all such requirements in writing prior to the start of construction. Two day advance notice shall be given when requesting site visits necessary as the basis for the compliance letter. The general contractor shall provide copies of all third-party testing and inspection reports to the architect and structural engineer a minimum of one week prior to the date that the compliance letter is needed.

INSPECTIONS: Inspections and Testing shall be performed by a qualified Inspector in accordance with IRC section R109. The Inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official, for inspection of the particular type of construction or operation requiring inspection. Except as noted, the inspections outlined in the IRC are in addition to, and beyond the scope of, periodic Structural Observations. Structural Observations are included in the structural engineering design and construction administration services provided by the structural engineer.

PENNYWEIGHT	TYPE	DIAMETER	LEN
6d	COOLER	0.092"	1-
	COMMON	0.131"	2-
60	BOX	0.113"	2-
8d	SINKER	0.113"	2-
	GUN	0.113"	2-
	COMMON	0.148"	
10d	BOX	0.128"	
100	SINKER	0.120"	2-
	GUN	0.131"	
	COMMON	0.148"	3-
104	BOX	0.128"	3-
12d	SINKER	0.135"	3-
	GUN	0.131"	3-
	COMMON	0.162"	3-
16d	BOX	0.135"	3-
	SINKER	0.148"	3-



n plans or shop details. Use of Anthem's electronic files as	
se of liability by the general contractor and/or his	
ed	

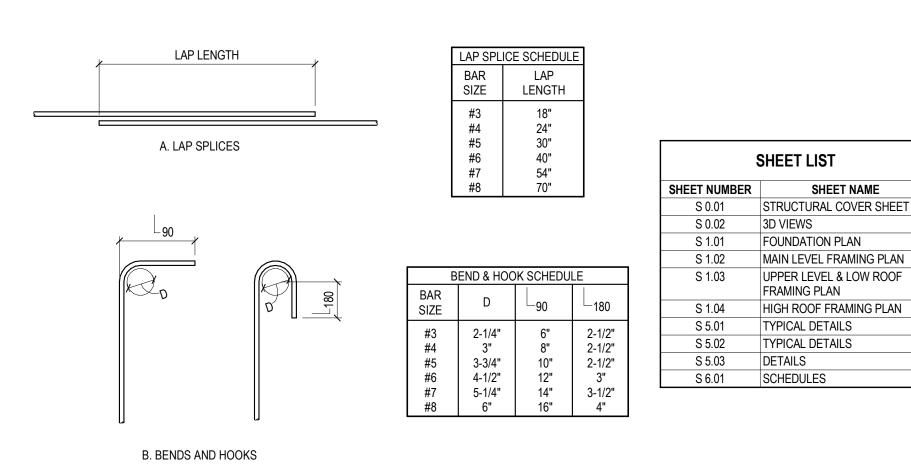
The structural drawings illustrate typical and representative details to assist the general contractor. Details shown apply at all similar

ANY OTHER building site. Plans shall not be used for construction at any other building site without specific review by the engineer.

ENGTH 1-7/8" 2-1/2" 2-1/2" 2-3/8" 2-3/8" 3" 2-7/8" 3-1/4" 3-1/4" 3-1/8" 3-1/4" 3-1/2" 3-1/2" 3-1/4"

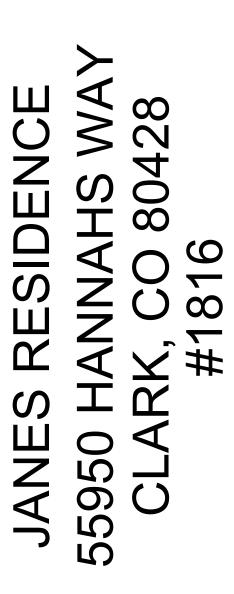
AB	Anchor Rod (Bolt)	EF	Each Face	LVL	Laminated Veneer Lumber (generic)	RO	Rough Openin
ADDL	Additional	EJ	Expansion Joint	LW	Light Weight	SC	Slip Critical
AFF	Above Finished Floor	EL	Elevation	MASY	Masonry	SCH	Schedule
ALT	Alternative	EN	Edge Nailing	MATL	Material	SDST	Self Drilling Se
AMT	Amount	ENGR	Engineer	MAX	Maximum	SECT	Section
APPROX	Approximate	EQ	Equal	MECH	Mechanical	SF	Square Feet
ARCH	Architect, Architectural	EQUIP	Equipment	MEZZ	Mezzanine	SHT	Sheet
ASD	Allowable Stress Design	EQUIV	Equivalent	MFR	Manufacture, -er, -rd	SHTG	Sheathing
AVG	Average	ES	Each Side	MIN	Minimum	SIM	Similar
BC	Bottom of Concrete	EST	Estimate	MTL	Metal	SL	Sloped
BL	Brick Ledge	E-W	East to West	NIC	Not In Contact	SOG	Slab On Grade
BLK	Block	EXC	Excavate	N-S	North to South	SP	Space,-s
BLKG	Blocking	EXP	Expansion	NTS	Not to Scale	SPEC	Specifications
BM	Beam	EXT	Exterior	OD	Outside Diameter	SQ	Square
BOT	Bottom	FDN	Foundation	OF	Outside Face	STD	Standard
BRG	Bearing	FF	Finished Floor	OH	Opposite Hand	STL	Steel
CANT	Cantilever	FIG	Figure	OPNG	Opening	STIFF	Stiffener
CF	Cubic Foot	FL	Flush	OPP	Opposite	STRUCT	Structure (Stru
CIP	Cast In Place	FLR	Floor	OSB	Oriented Strand Board	SY	Square Yard
CJ	Construction Joint (Control Joint)	FP	Full Penetration	PAF	Powder Actuated Fastener	SYM	Symmetrical
CLG	Ceiling	FTG	Footing	PC	Precast	T&B	Top and Botto
CLR	Clear	GA	Gage (Gauge)	PE	Pre-engineered (trusses)	T&G	Tongue and G
CMU	Concrete Masonry Unit	GALV	Galvanized	PEN	Penetration	ТВ	Top of Beam
COL	Column	GC	General Contractor	PERP	Perpendicular	TC	Top of Concre
СОМ	Common	GEN	General	PKT	Pocket	TJ	Top of Joist
CONC	Concrete	GL	Glue Laminated (Glu-lam)	PL	Property Line	TL	Total Load, To
CONN	Connection	GR	Grade	PLF	Pounds per Linear Foot	ТМ	Top of Masonr
CONT	Continue (Continuous)	GT	Girder Truss	PSF	Pounds per Square Foot	Т.О	Top of
CONSTR	Construction	GYP BD	Gypsum Board	PSI	Pounds per Square Inch	TRANS	Transverse
COORD	Coordinate, Coordination	HAS	Headed Anchor Stud	PSL	Parallel Strand Lumber (generic)	TYP	Typical
CS	Countersink	HNGR	Hanger	PT	Pressure Treated	ULT	Ultimate
CTR	Center	HORIZ	Horizontal	P.T	Post Tensioned	UNO	Unless Noted
CY	Cubic Yard	HT	Height or Heavy Timber	PV	Photovoltaic	VERT	Vertical
DAB	Deformed Anchor Bar	ID	Inside Diameter	QTY	Quantity	VIF	Verify In Field
DIAG	Diagonal	INT	Interior	R	Radius	WA	Wedge Anchor
DIM	Dimension	K	Kip (1,000 lbs)	RE	Reference (refer to)	WF	Wide Flange
DL	Dead Load	LGS	Light Gage Stud	RECT	Rectangle	WP	Work Point
DN	Down	LL	Live Load	REINF	Reinforcement	WT	Weight
DP	Drilled Pier	LLH	Long Leg Horizontal	REQ	Required	WWF	Welded Wire F
DWG	Drawing	LLV	Long Leg Vertical	REQMT	Requirement	XS	Extra Strong
EA	Each	LSH	Long Side Horizontal	RET	Retaining Wall	XSECT	Cross Section
ECC	Eccentric	LSV	Long Side Vertical	RM	Room	XXS	Double Extra S
E-E	End to End	LT	Light	RMO	Rough Masonry Opening		

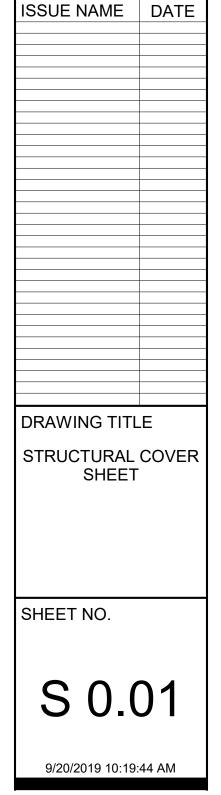
						303-8 anthei	mstructural.com Job #19-
			ABBREV	ATIONS	KEY		
3 DDL	Anchor Rod (Bolt) Additional	EF EJ	Each Face	LVL LW	Laminated Veneer Lumber (generic) RO SC	Rough Opening
F	Additional Above Finished Floor	EL	Expansion Joint Elevation	MASY	Light Weight Masonry	SCH	Slip Critical Schedule
_T MT	Alternative Amount	EN ENGR	Edge Nailing Engineer	MATL MAX	Material Maximum	SDST SECT	Self Drilling Self Tapping Section
PROX	Approximate	ENGR	Engineer	MAX	Mechanical	SF	Square Feet
RCH SD	Architect, Architectural Allowable Stress Design	EQUIP EQUIV	Equipment Equivalent	MEZZ MFR	Mezzanine Manufacture, -er, -rd	SHT SHTG	Sheet Sheathing
VG	Average	EQUIV	Each Side	MIN	Minimum	SIM	Similar
C L	Bottom of Concrete Brick Ledge	EST E-W	Estimate East to West	MTL NIC	Metal Not In Contact	SL SOG	Sloped Slab On Grade
- _K	Block	EXC	Excavate	N-S	North to South	SP	Space,-s
LKG M	Blocking Beam	EXP EXT	Expansion Exterior	NTS OD	Not to Scale Outside Diameter	SPEC SQ	Specifications Square
OT	Bottom	FDN	Foundation	OF	Outside Face	STD	Standard
RG	Bearing	FF FIG	Finished Floor	OH OPNG	Opposite Hand	STL STIFF	Steel Stiffener
ANT F	Cantilever Cubic Foot	FIG	Figure Flush	OPNG OPP	Opening Opposite	STRUCT	Structure (Structural)
IP J	Cast In Place	FLR	Floor	OSB PAF	Oriented Strand Board	SY SYM	Square Yard
_G	Construction Joint (Control Joint) Ceiling	FP FTG	Full Penetration Footing	PAF PC	Powder Actuated Fastener Precast	T&B	Symmetrical Top and Bottom
LR	Clear	GA	Gage (Gauge)	PE	Pre-engineered (trusses)	T&G TB	Tongue and Groove
MU OL	Concrete Masonry Unit Column	GALV GC	Galvanized General Contractor	PEN PERP	Penetration Perpendicular	TC	Top of Beam Top of Concrete
OM	Common	GEN	General	PKT	Pocket	TJ	Top of Joist
ONC ONN	Concrete Connection	GL GR	Glue Laminated (Glu-lam) Grade	PL PLF	Property Line Pounds per Linear Foot	TL	Total Load, Top of Ledge Top of Masonry
	Continue (Continuous)	GT	Girder Truss	PSF	Pounds per Square Foot	T.O	Top of
ONSTR OORD	Construction Coordinate, Coordination	GYP BD HAS	Gypsum Board Headed Anchor Stud	PSI PSL	Pounds per Square Inch Parallel Strand Lumber (generic)	TRANS TYP	Transverse Typical
S	Countersink	HNGR	Hanger	PT	Pressure Treated	ULT	Ultimate
TR Y	Center Cubic Yard	HORIZ HT	Horizontal Height or Heavy Timber	P.T PV	Post Tensioned Photovoltaic	UNO VERT	Unless Noted Otherwise Vertical
AB	Deformed Anchor Bar	ID	Inside Diameter	QTY	Quantity	VIF	Verify In Field
IAG IM	Diagonal Dimension	INT K	Interior Kip (1,000 lbs)	R RE	Radius Reference (refer to)	WA WF	Wedge Anchor Wide Flange
-	Dead Load	LGS	Light Gage Stud	RECT	Rectangle	WP	Work Point
N P	Down Drilled Pier	LL	Live Load Long Leg Horizontal	REINF	Reinforcement Required	WT WWF	Weight Welded Wire Fabric
WG	Drawing	LLV	Long Leg Vertical	REQMT	Requirement	XS	Extra Strong
A CC	Each Eccentric	LSH LSV	Long Side Horizontal	RET RM	Retaining Wall Room	XSECT XXS	Cross Section Double Extra Strong
E	End to End	LT	Light		Rough Masonry Opening		
_	XK VT "X" King studs "V" Trimme	r etude etude			$\overline{\mathbf{A}}$		
	XK, YT "X" King studs, "Y" Trimmer wall thickness	r studs, studs			СМИ		
	XK, YT "X" King studs, "Y" Trimmer wall thickness C Indicates column continuou		to match				
	wall thickness C Indicates column continuou	s through leve	to match el shown		CMU Concrete		
	A A Indicates column continuou Indicates column above lev level framing plan for size; i in floor cavity of equal size	s through leve el shown, see install squash and equal colu	to match el shown e next blocking umn size				
	A A Mail thickness Mail thickness Indicates column continuou Indicates column above lev level framing plan for size; i	s through leve el shown, see install squash and equal colu s noted otherv	to match el shown e next blocking umn size vise				
	 Wall thickness C Indicates column continuou A Indicates column above lev level framing plan for size; i in floor cavity of equal size below to foundation - unless 	is through leve el shown, see install squash and equal colu s noted otherv w level shown	to match el shown e next blocking umn size vise		Concrete Concrete Earth fill Porous fill (i.e. gravel)	all	
	wall thickness C Indicates column continuou A Indicates column above level framing plan for size; i in floor cavity of equal size below to foundation - unless XXXX Indicates column type below	s through leve el shown, see install squash and equal colu s noted otherv w level shown or beam	to match el shown e next blocking umn size vise		Concrete Concrete Earth fill Porous fill (i.e. gravel)	all	
	wall thickness wall thickness Indicates column continuou A Indicates column above lev level framing plan for size; i in floor cavity of equal size below to foundation - unless XXXX Indicates column type below — Indicates dropped header o Beam, Joist, or Truss bears Beam, Joist, or Truss connect hanger	s through leve el shown, see install squash and equal colu s noted otherv w level shown or beam s on wall or be ected to suppo	to match el shown e next blocking umn size vise eam below ort with metal		Concrete Concrete Earth fill Porous fill (i.e. gravel) Interior wood bearing w	all	
	wall thickness C Indicates column continuou A Indicates column above lev level framing plan for size; i in floor cavity of equal size below to foundation - unless XXXX Indicates column type below — Indicates column type below — Indicates dropped header o Beam, Joist, or Truss bears Beam, Joist, or Truss connel hanger Beam, Joist, or Truss connel concealed hanger	s through leve el shown, see install squash and equal colu s noted otherv w level shown or beam s on wall or be ected to suppo	to match el shown e next blocking umn size vise eam below ort with metal		Concrete Concrete Earth fill Porous fill (i.e. gravel) Interior wood bearing w Wood shear wall Indicates 'existing' Indicates 'new'		
	wall thickness C Indicates column continuou A Indicates column above lev level framing plan for size; i in floor cavity of equal size below to foundation - unless XXXX Indicates column type below — Indicates column type below — Indicates dropped header o Beam, Joist, or Truss bears Beam, Joist, or Truss connect hanger Beam, Joist, or Truss connect concealed hanger Indicates span direction	s through leve el shown, see install squash and equal colu s noted otherv w level shown or beam s on wall or be ected to suppo	to match el shown e next blocking umn size vise eam below ort with metal		Concrete Con	d'	
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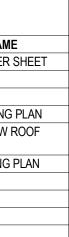


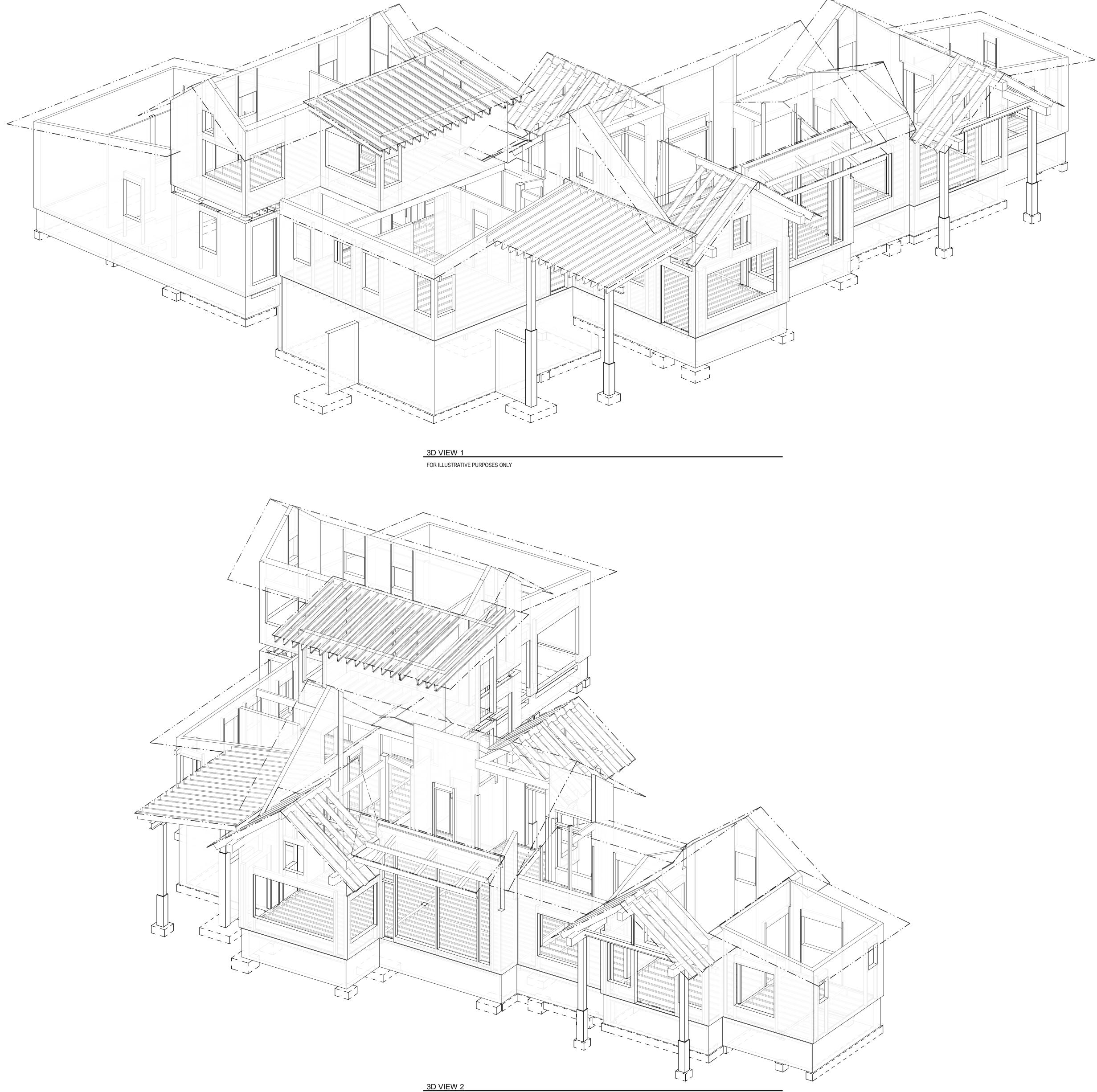
TYPICAL REINFORCING FOR CONCRETE

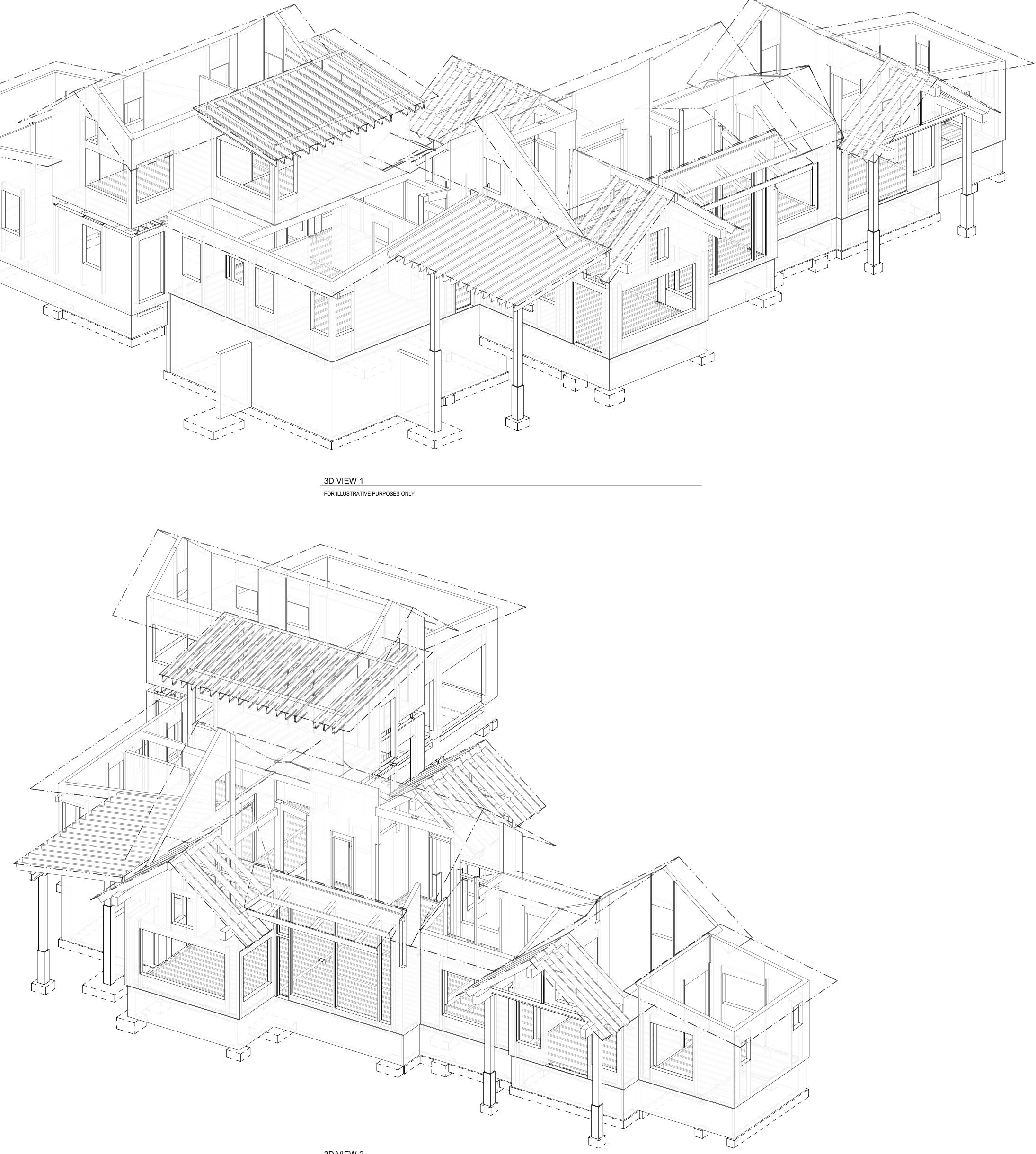








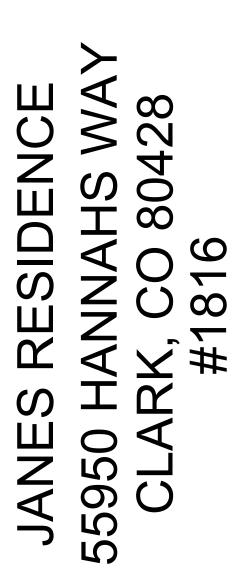


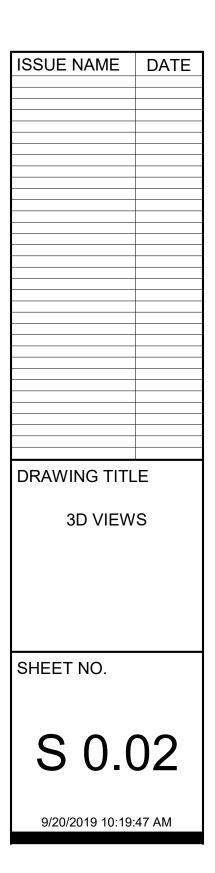


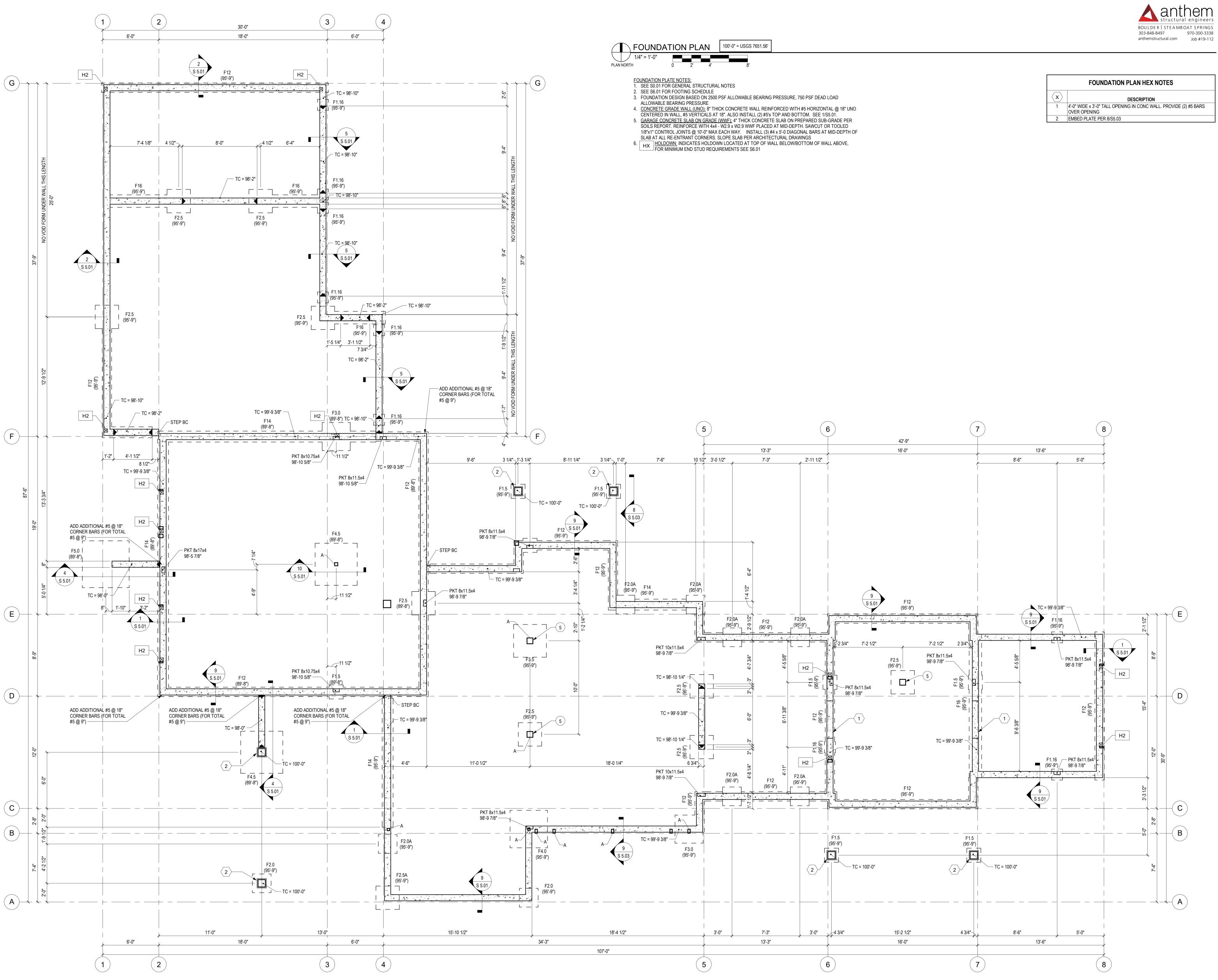
FOR ILLUSTRATIVE PURPOSES ONLY





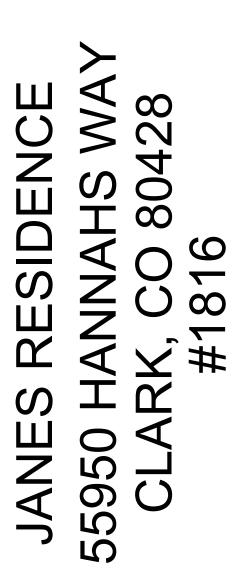


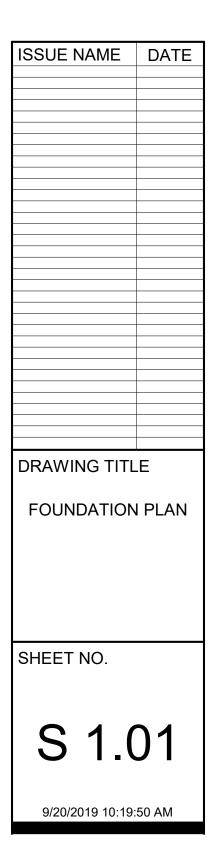


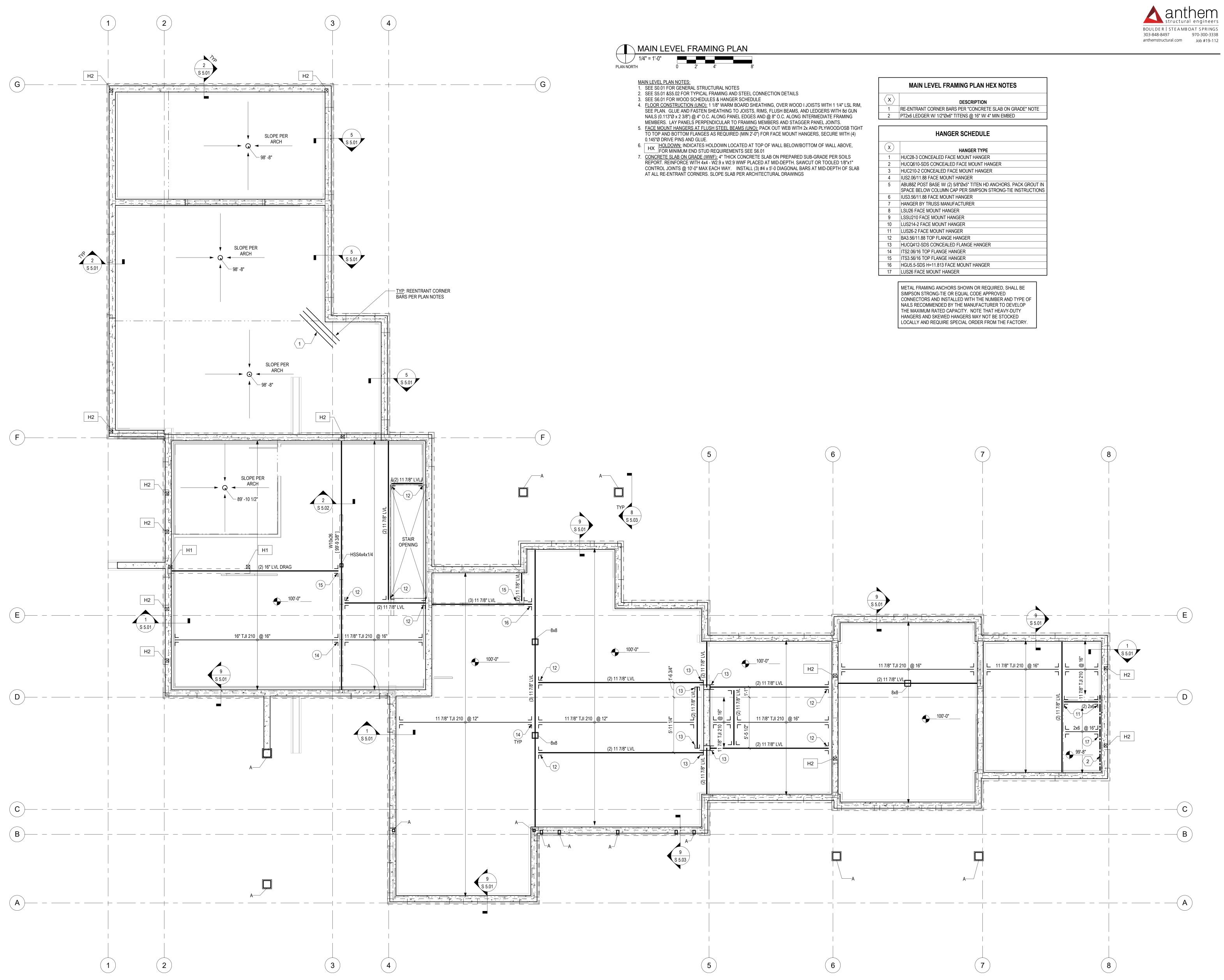


FOUNDATION PLAN HEX NOTES				
$\langle \mathbf{x} \rangle$	DESCRIPTION			
1	4'-0" WIDE x 3'-0" TALL OPENING IN CONC WALL. PROVIDE OVER OPENING			
2	EMBED PLATE PER 8/S5.03			

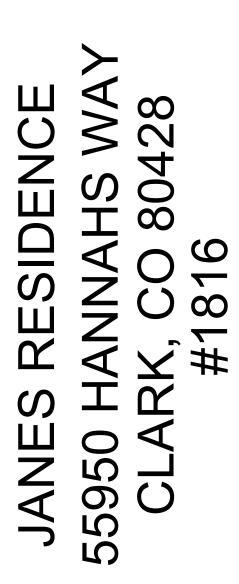


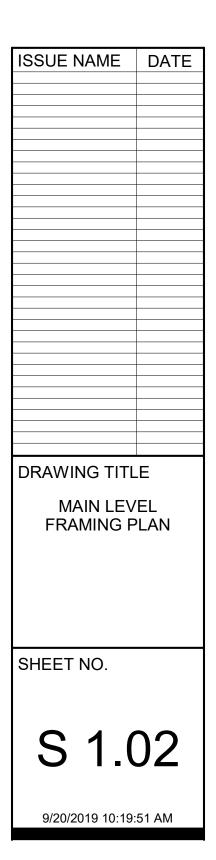


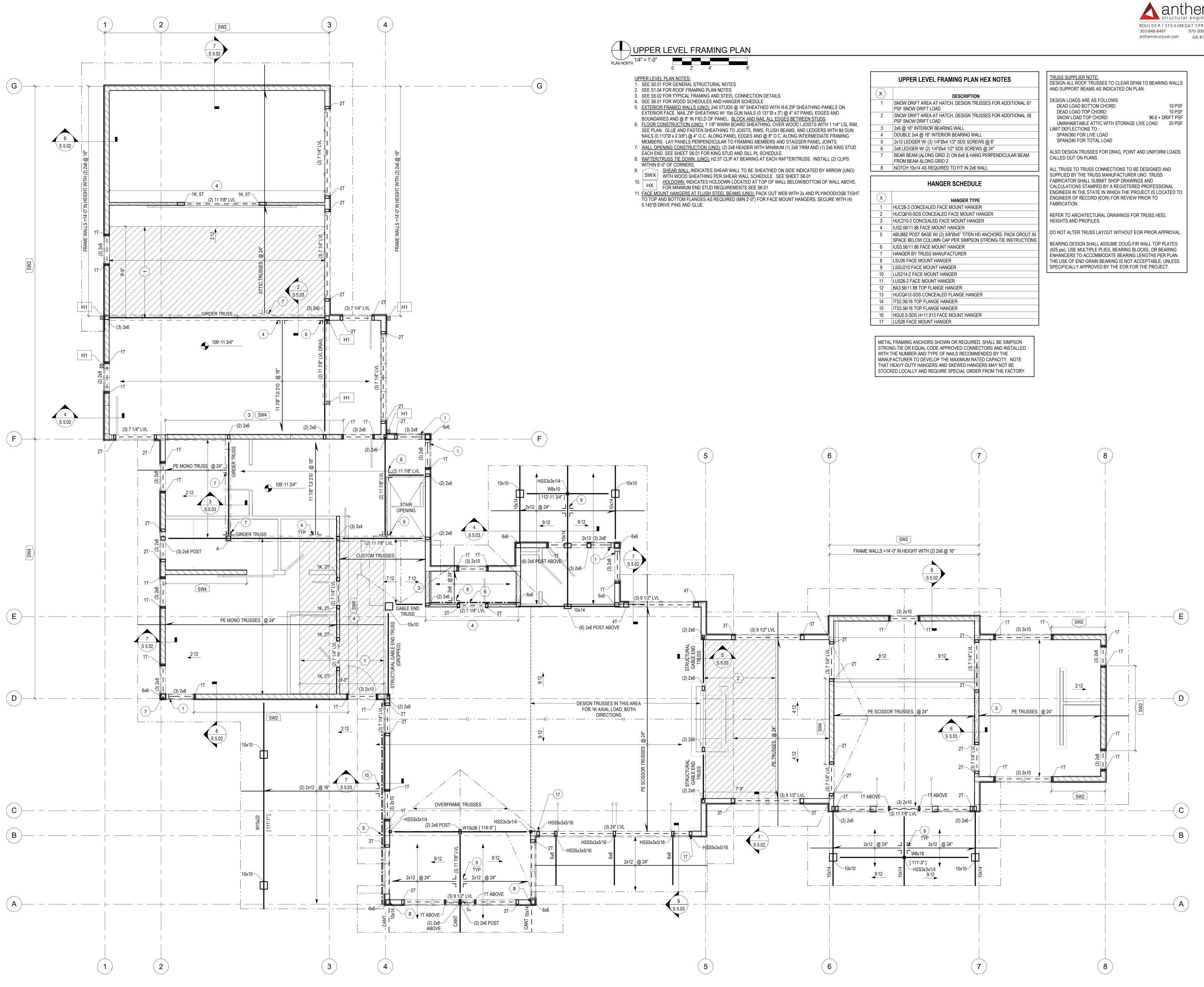














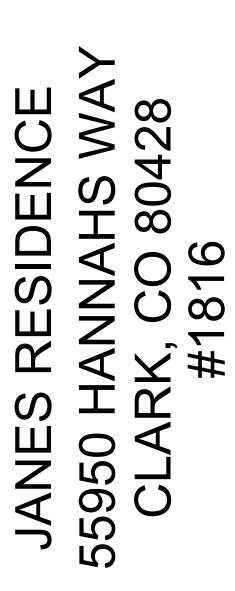
UPPER LEVEL FRAMING PLAN HEX NOTES

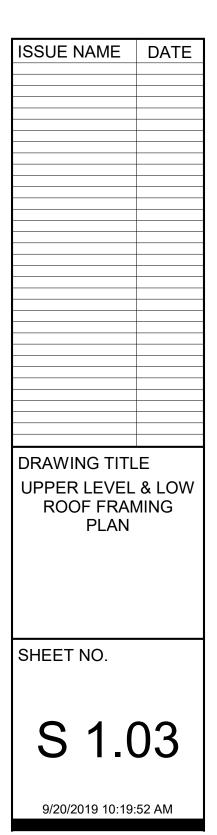
- 10 PSF 10 PSF 96.6 + DRIFT PSF UNINHABITABLE ATTIC WITH STORAGE LIVE LOAD: 20 PSF ALSO DESIGN TRUSSES FOR DRAG, POINT AND UNIFORM LOADS ALL TRUSS TO TRUSS CONNECTIONS TO BE DESIGNED AND CALCULATIONS STAMPED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED TO DO NOT ALTER TRUSS LAYOUT WITHOUT EOR PRIOR APPROVAL.

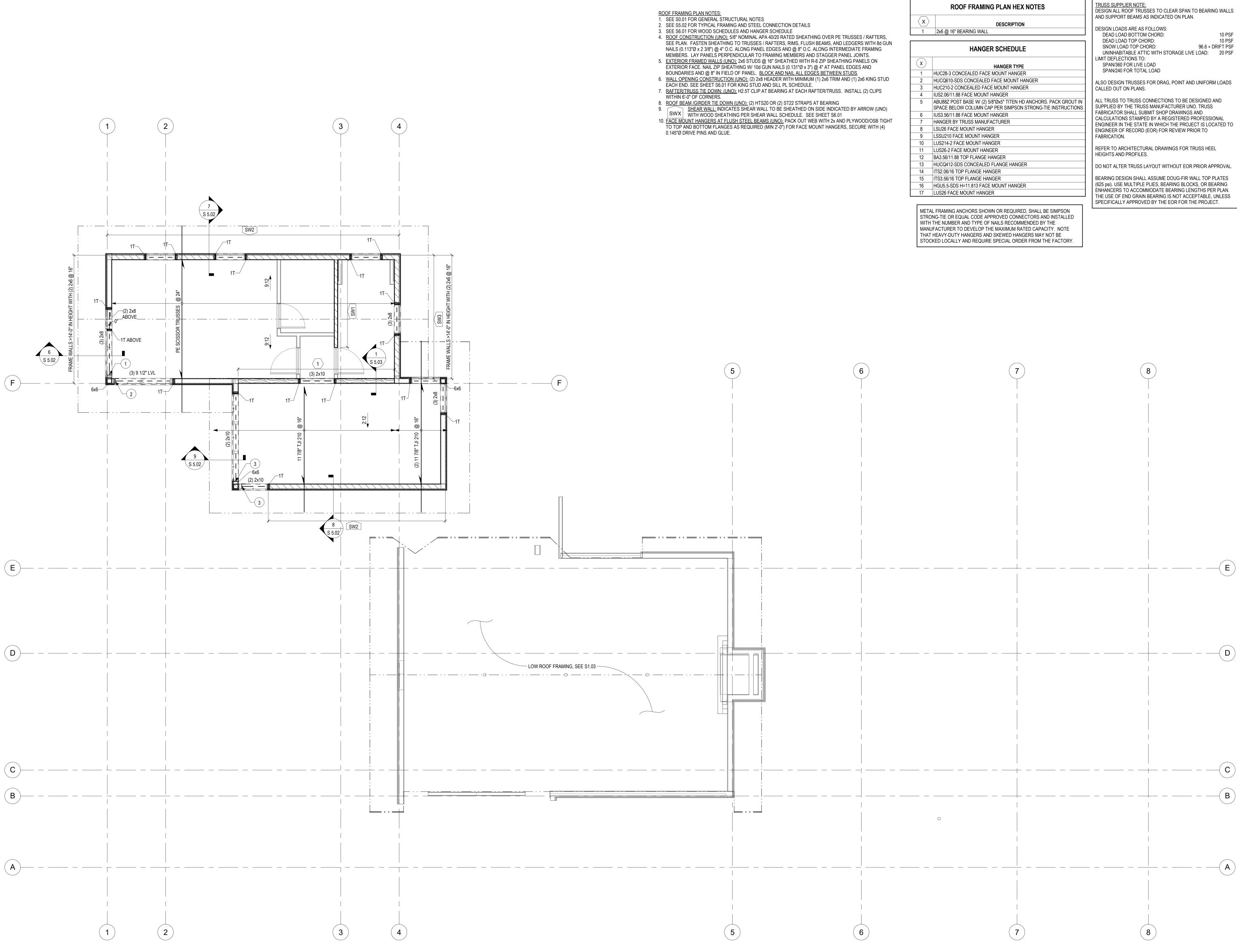
ENHANCERS TO ACCOMMODATE BEARING LENGTHS PER PLAN. THE USE OF END GRAIN BEARING IS NOT ACCEPTABLE, UNLESS SPECIFICALLY APPROVED BY THE EOR FOR THE PROJECT.













ROOF FRAMING PLAN

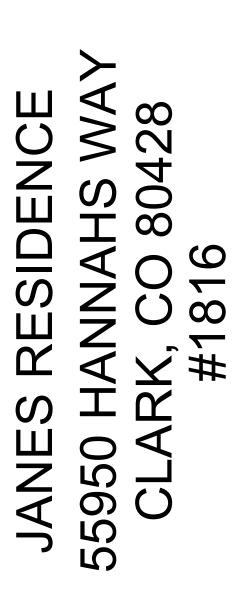
/ 1/4" = 1'-0"

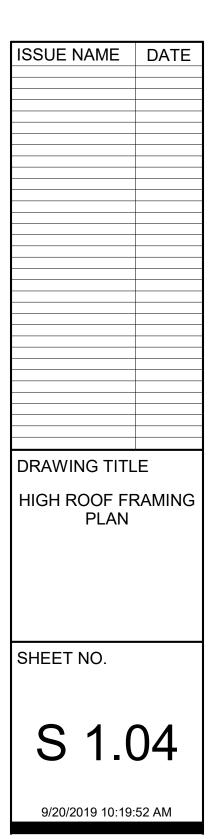
PLAN NORTH

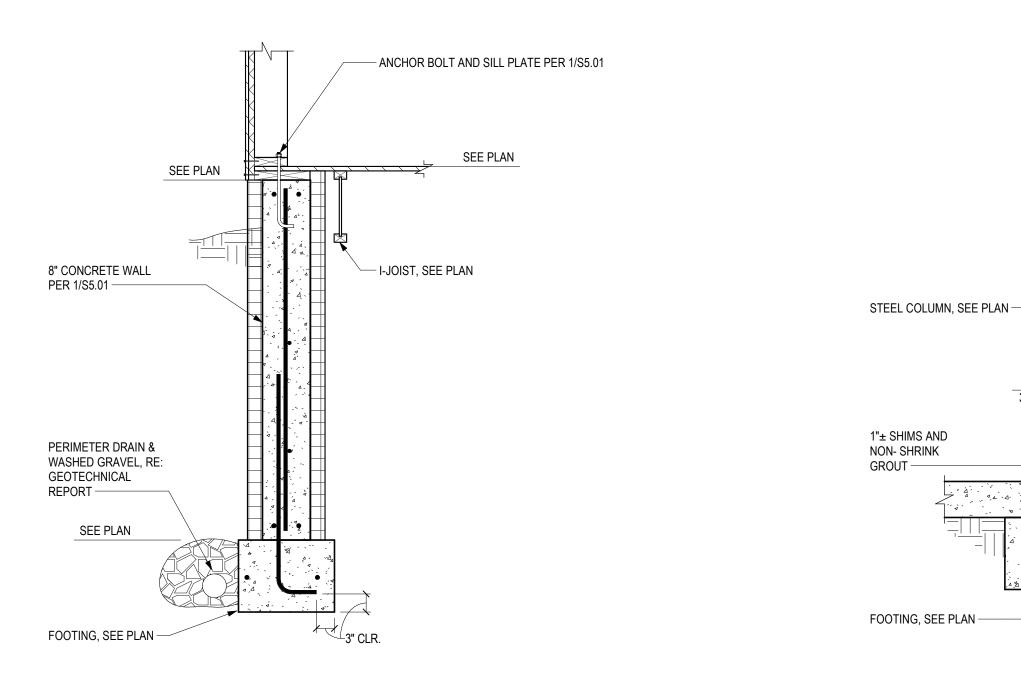
	ROOF FRAMING PLAN HEX NOTE
\frown	

TRUSS SUPPLIER NOTE:
DESIGN ALL ROOF TRUSSES TO CLEAR SPAN TO BEARING W
TRUSS SUPPLIER NOTE: DESIGN ALL ROOF TRUSSES TO CLEAR SPAN TO BEARING W. AND SUPPORT BEAMS AS INDICATED ON PLAN.

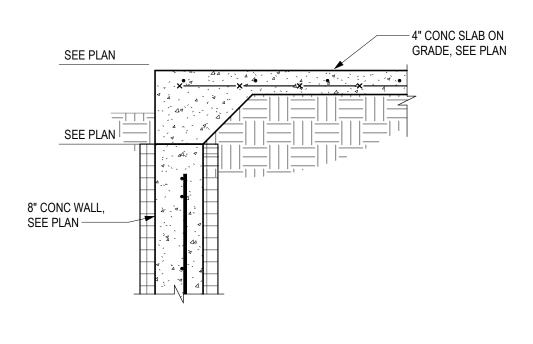


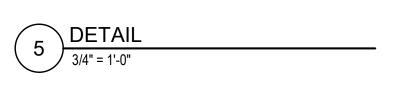


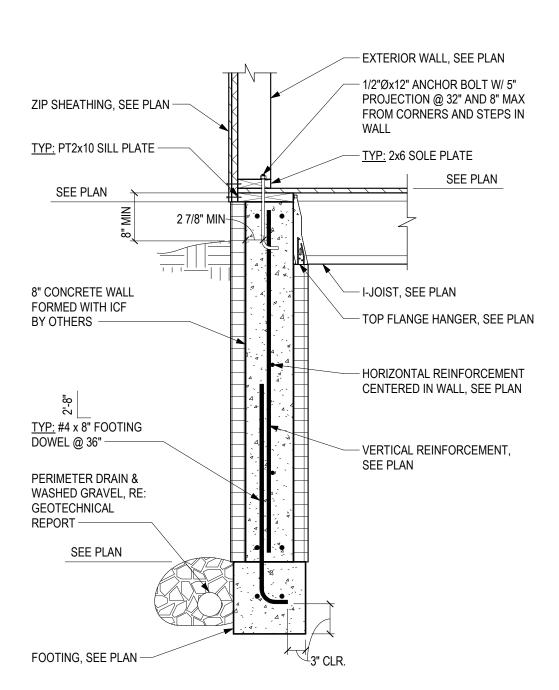




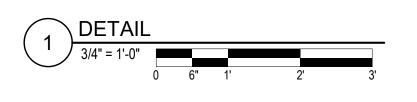




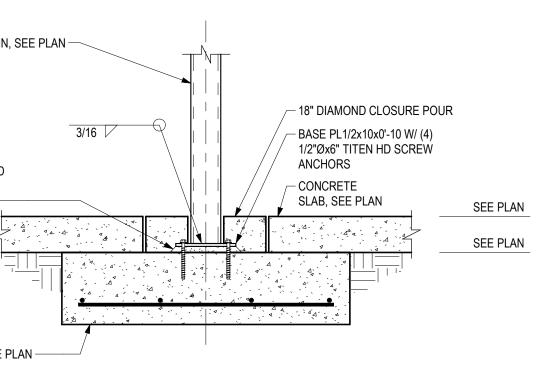




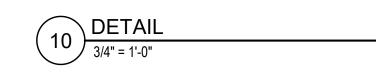
TYPICAL CRAWLSPACE FOUNDATION WALL

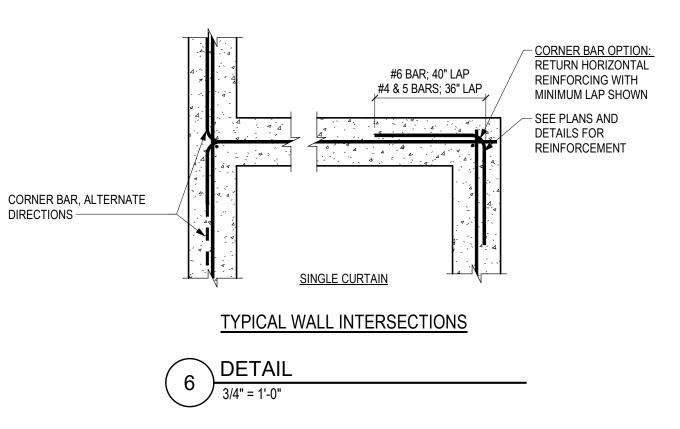


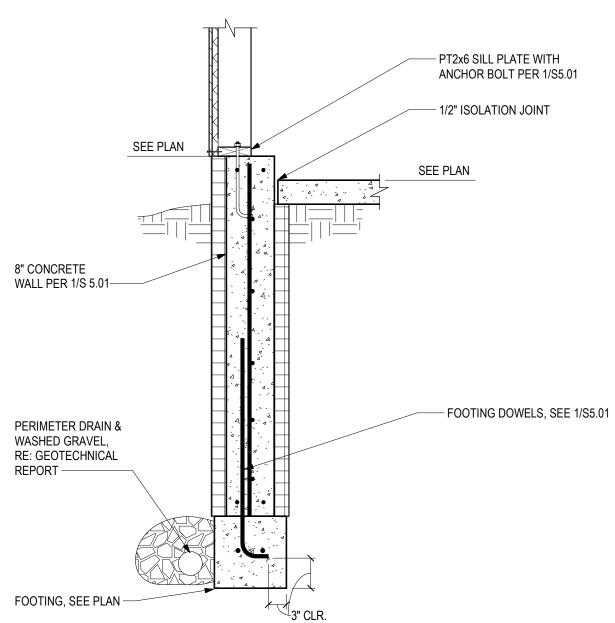
DIRECTIONS -



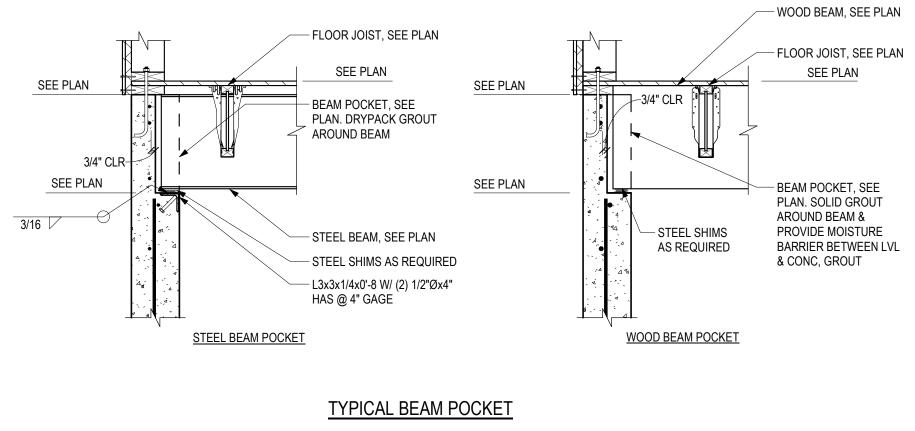
TYPICAL STEEL COLUMN AT INTERIOR FOOTING

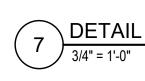


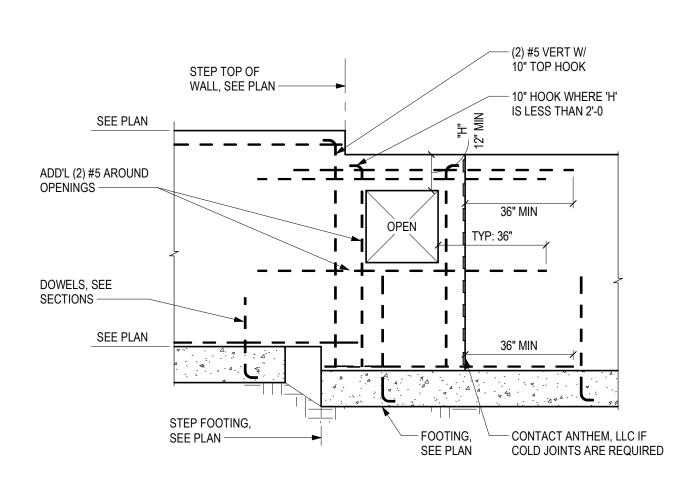




TYPICAL GARAGE FOUNDATION WALL



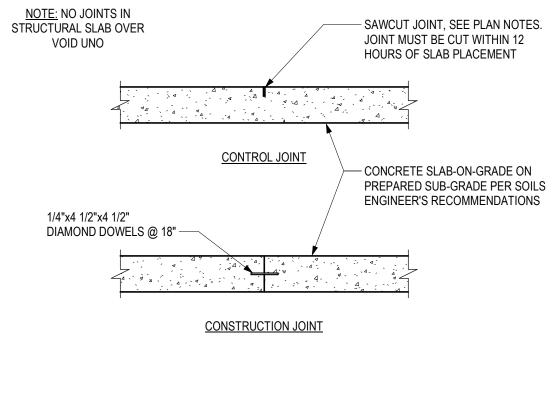




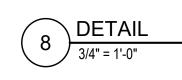


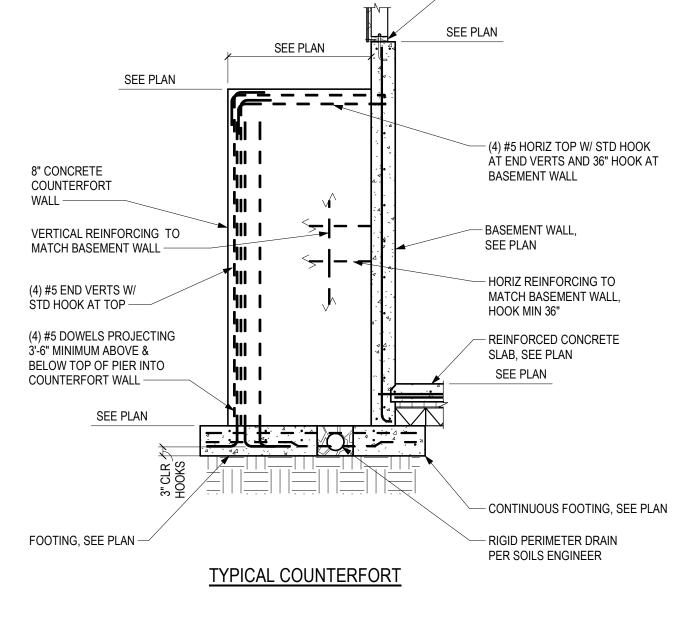






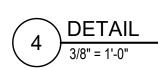
TYPICAL JOINTS AT CONCRETE SLAB-ON-GRADE





TYPICAL REINFORCING AT WALL OPENINGS AND STEPS





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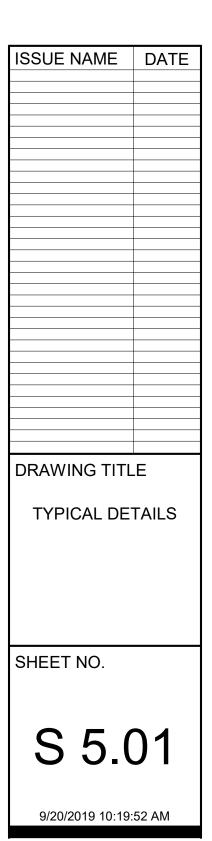
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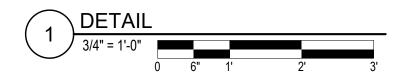
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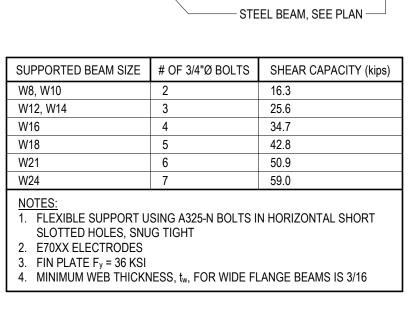
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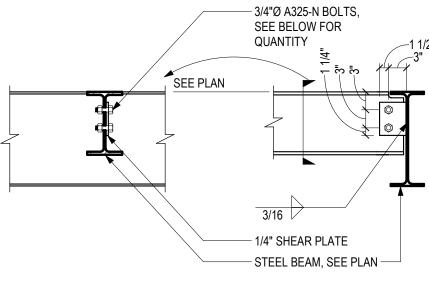
SEE1/S 5.01

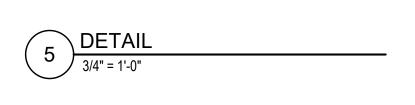


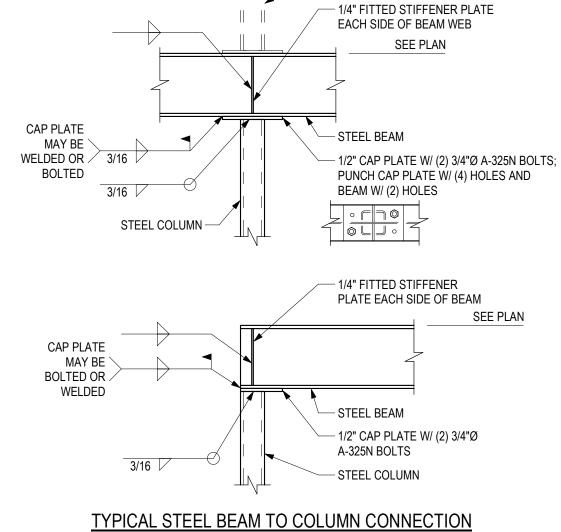




TYPICAL SINGLE PLATE SHEAR CONNECTION



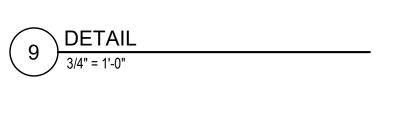




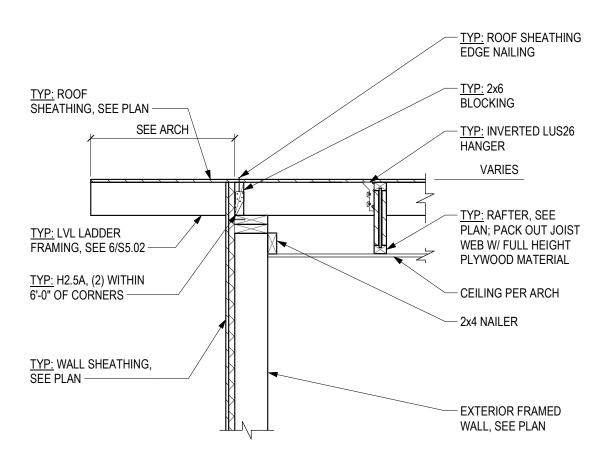
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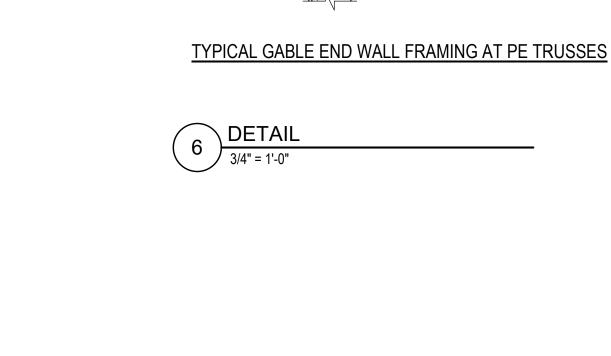
STEEL COLUMN ABOVE

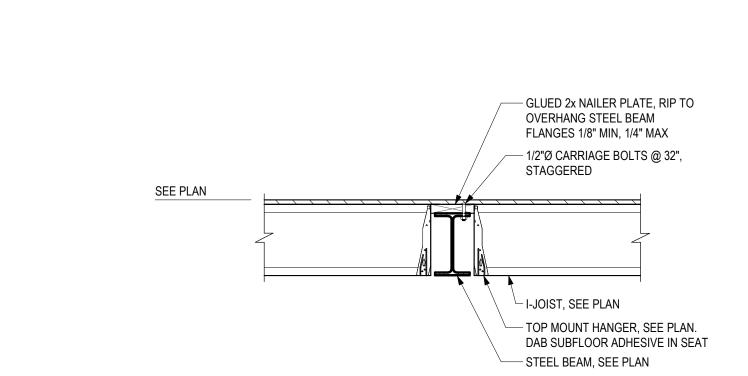
WHERE OCCURS ON PLAN



TYPICAL GABLE END WALL FRAMING AT RAFTERS







TYP: INVERTED LUS26 HANGER ~

<u>TYP:</u> 7 1/4" LVL @ 24" LADDER FRAMING ——

MEMBERS, SEE PLAN -

<u>TYP:</u> (2) 2x TOP PLATE -

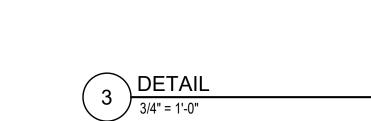
2x6 BLOCKING -

ROOF SHEATHING,

SEE PLAN -

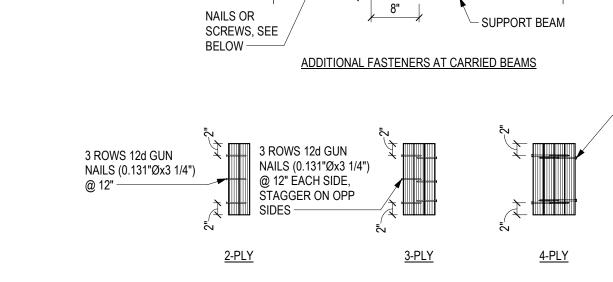
2 DETAIL 3/4" = 1'-0"

TYPICAL I-JOIST TO FLUSH STEEL BEAM



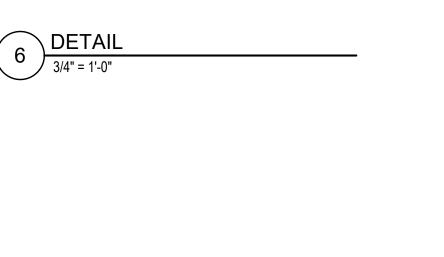
PLY-TO-PLY CONNECTION

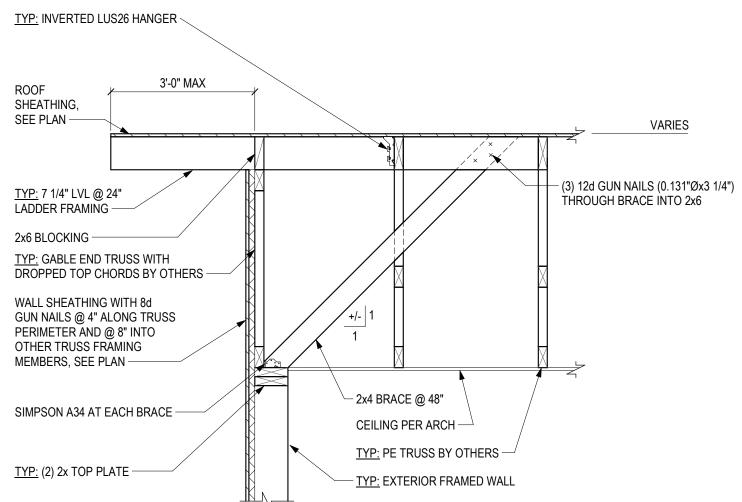
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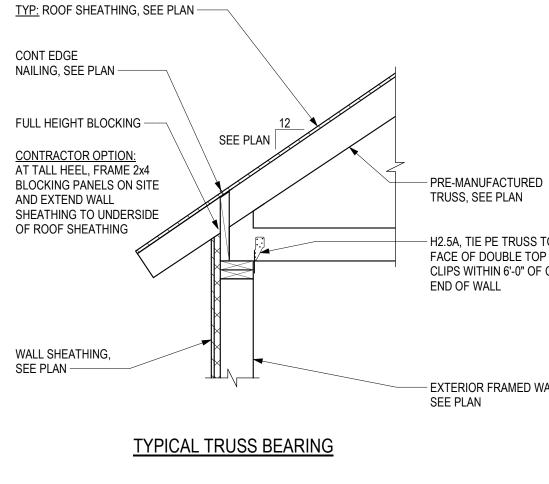


(4) 1/4"Ø x 3" SDS SCREWS EACH SIDE

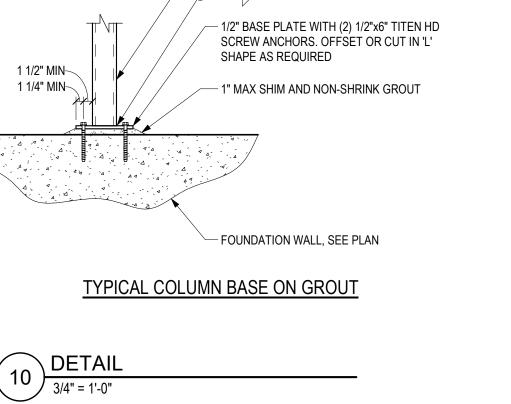
OF CARRIED BEAM -



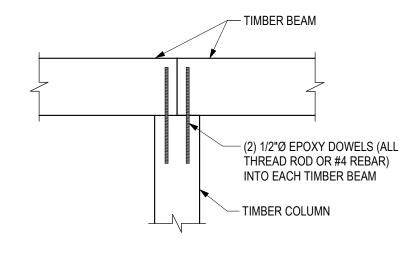




7 DETAIL 3/4" = 1'-0"



— STEEL COLUMN, SEE PLAN



TYPICAL TIMBER BEAM TO COLUMN CONNECTION

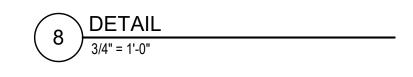
(11) DETAIL 3/4" = 1'-0"

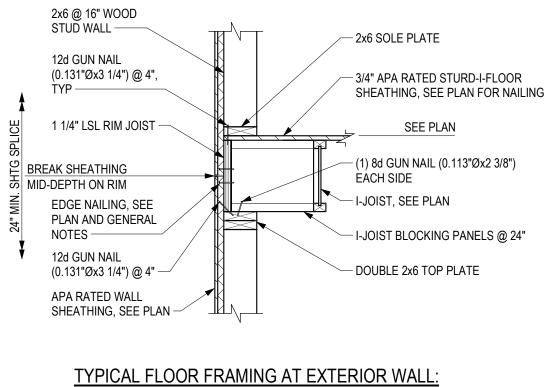


LSL BLOCKING & VENTILATION HOLES: L/3 L/3 L/3 $\neg \rightarrow$ MAX ALLOWABLE V-CUT FASTEN 2x4 TAILS TO SIDE OF I-JOIST W/ 10d GUN NAILS @ 16" ----SEE PLAN ROOF SHEATHING, SEE PLAN -----LSL SHEAR BLOCKING -2x6 @ 24" MAX RAFTER TAILS — TYPICAL SEE ARCH FOR PLATE - I-JOIST RAFTER W/ SEAT CUT & HEIGHT BEVELED WEB STIFFENERS ON BOTH SIDES AT BEARING 3'-0" MAX - WEB PACKING WALL SHEATHING, - BEVELED 2x4 BLOCK SEE PLAN ----- H2.5A, (2) CLIPS WITHIN 4'-0" OF CORNERS

TYPICAL I-JOIST RAFTER BEARING

- EXTERIOR FRAMED WALL, SEE PLAN





JOIST PARALLEL TO WALL





- CARRIED BEAM

• •

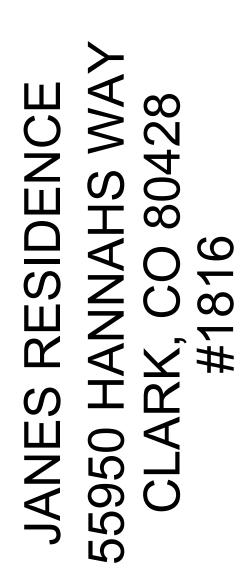
/--- 2 ROWS 1/4"Øx6" SDS SCREWS @ 16" EACH SIDE, STAGGER ON OPP SIDES

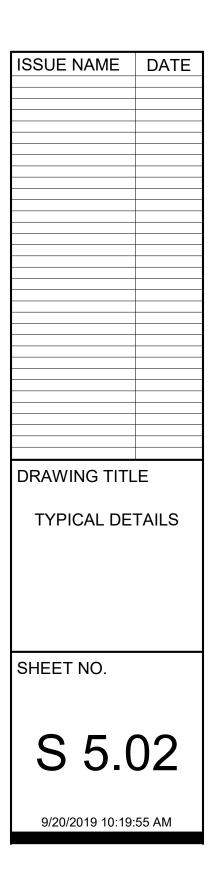
TYPICAL MULTI-MEMBER CONNECTIONS FOR BUILT UP BEAMS

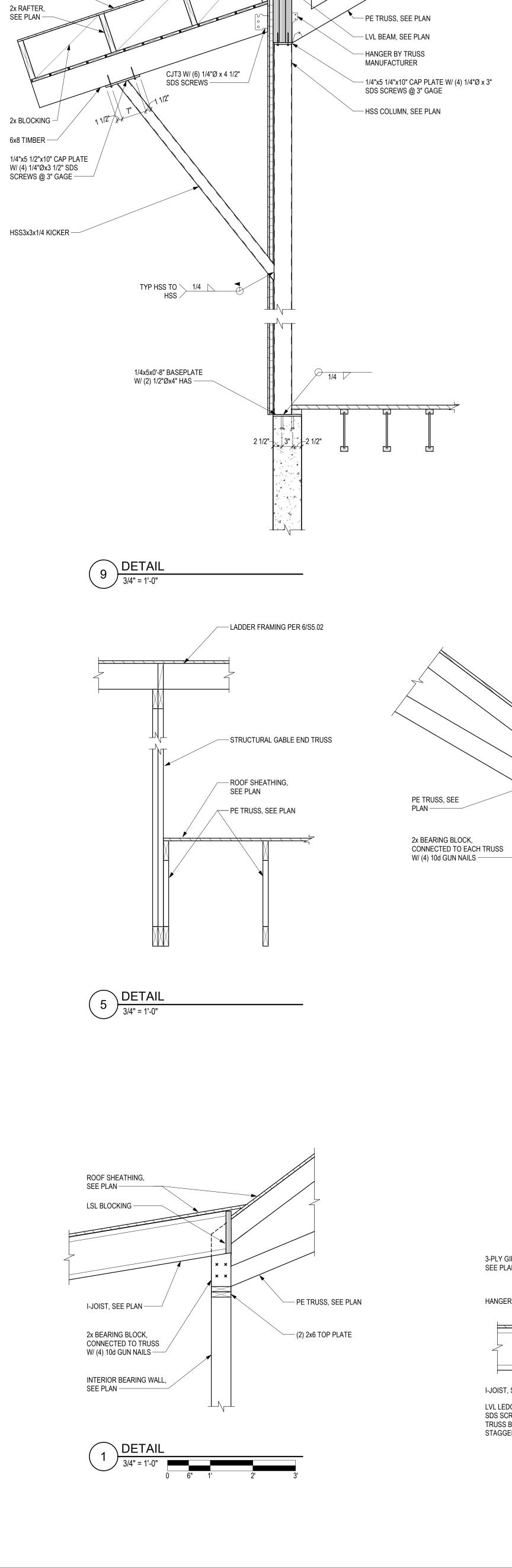
— H2.5A, TIE PE TRUSS TO OUTSIDE FACE OF DOUBLE TOP PLATE. (2) CLIPS WITHIN 6'-0" OF CORNERS AND END OF WALL

— EXTERIOR FRAMED WALL, SEE PLAN

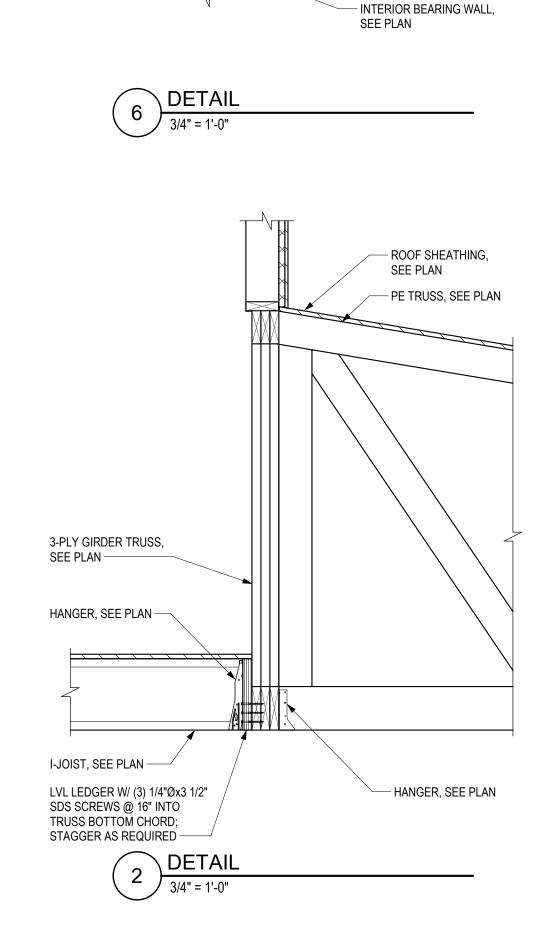








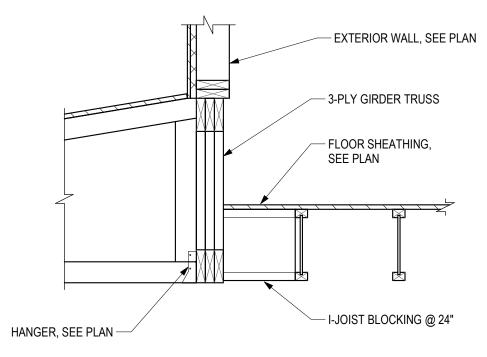
ROOF SHEATHING, SEE PLAN —



- ROOF SHEATHING, SEE PLAN

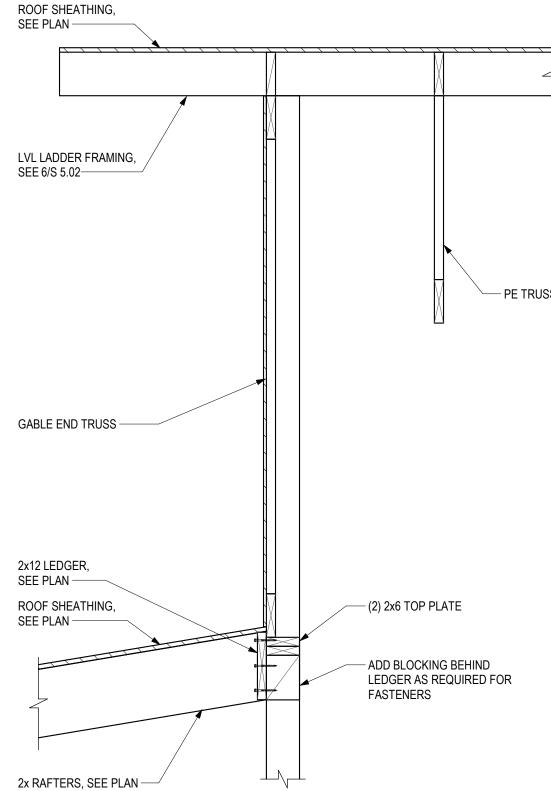
- PE TRUSS, SEE PLAN

— (2) 2x6 TOP PLATE



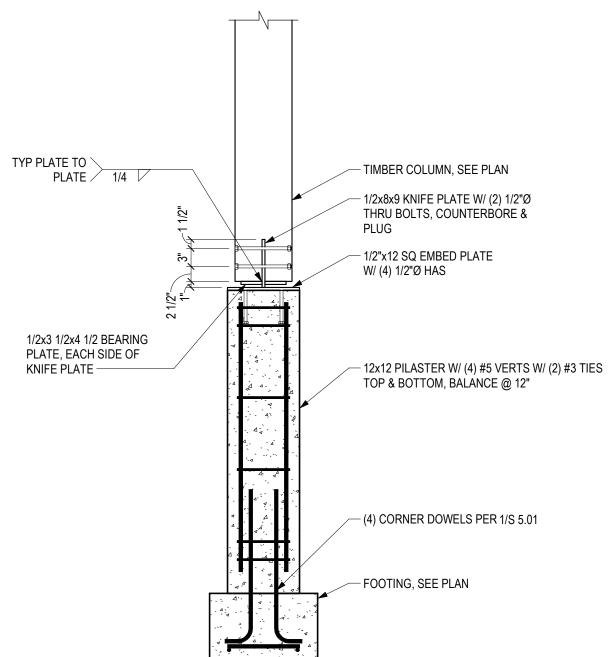
3 DETAIL 3/4" = 1'-0"





8 DETAIL 3/4" = 1'-0" H2.5A CLIP @ EA RAFTER — A35 CLIP @ 24", EA SIDE (3) 2x6 BEARING BLOCK /--- 2x RAFTER, SEE PLAN ____ ROOF SHEATHING -— TIMBER BEAM, SEE PLAN A35 CLIP @ 24" -2x BLOCKING — 2x RAFTER, SEE PLAN 2x6 BEARING BLOCK — H2.5A CLIP @ EA RAFTER — TIMBER BEAM, SEE PLAN 4 DETAIL 3/4" = 1'-0"

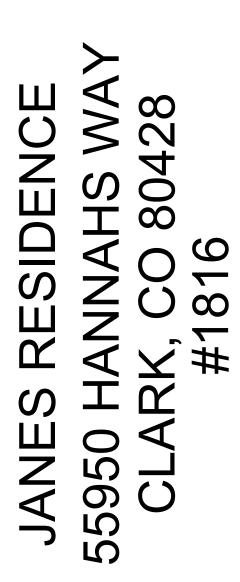
PE TRUSS, SEE PLAN

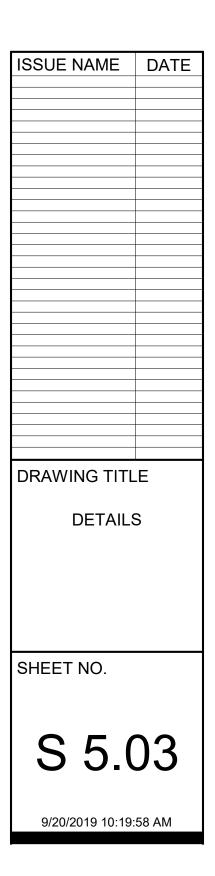






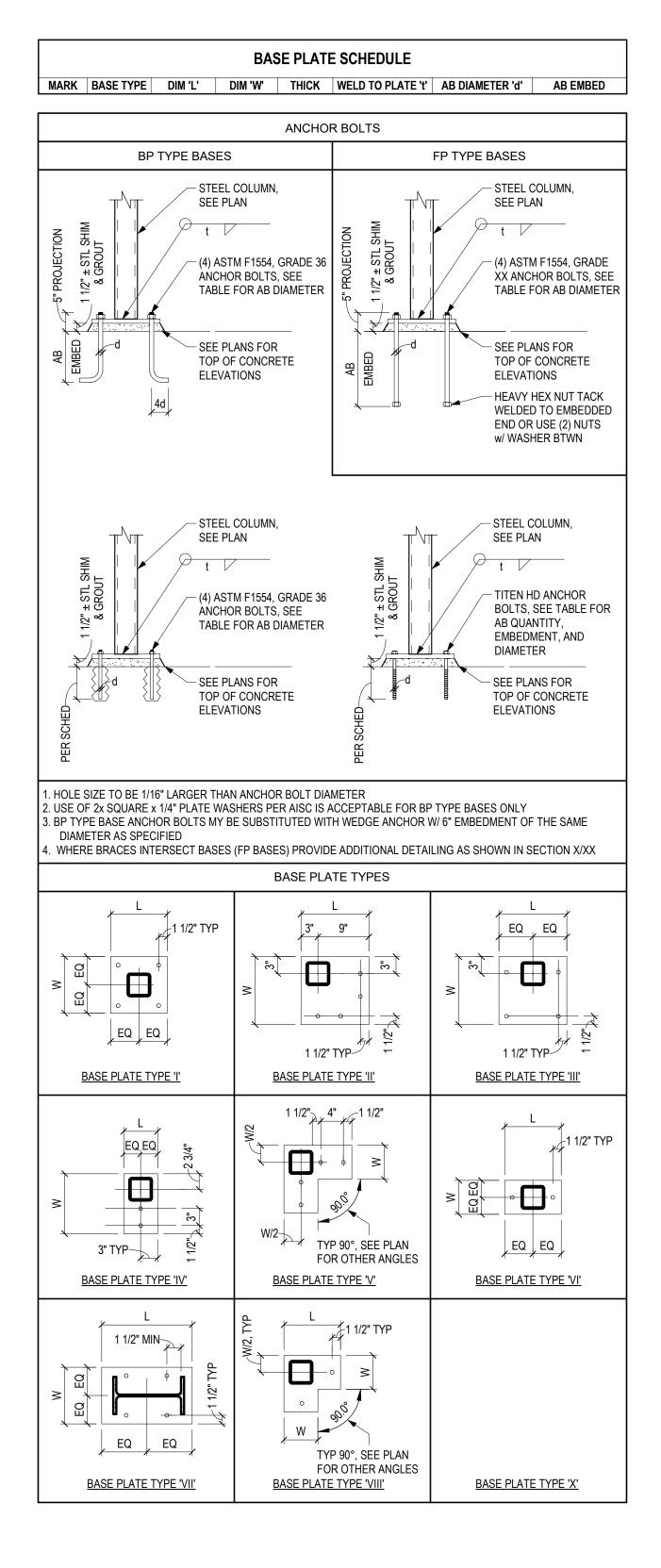






CONCRETE FOOTING SCHEDULE (CONT)				
MARK	WIDTH	THICKNESS	REINFORCEMENT	
F12	1'-0"	1'-0"	(2) #5's	
F14	1'-2"	1'-0"	(2) #5's	
F16	1'-4"	1'-0"	(3) #5's	





CONCRETE FOOTING SCHEDULE (ISOLATED PADS)						
MARK	LENGTH	WIDTH	THICKNESS	TOP REINFORCEMENT		
F1.5	1'-6"	1'-6"	1'-0"	(3) #5's EA WAY		
F1.16	1'-2"	1'-2"	1'-0"	(3) #5's EA WAY		
F2.0	2'-0"	2'-0"	1'-0"	(4) #5's EA WAY		
F2.0A	2'-0"	2'-0"	1'-6"	(4) #5's EA WAY		
F2.5	2'-6"	2'-6"	1'-0"	(4) #5's EA WAY		
F2.5A	2'-6"	2'-6"	1'-6"	(4) #5's EA WAY		
F3.0	3'-0"	3'-0"	1'-0"	(4) #5's EA WAY		
F3.5	3'-6"	3'-6"	1'-0"	(4) #5's EA WAY		
F4.0	4'-0"	4'-0"	1'-0"	(4) #5's EA WAY		
F4.5	4'-6"	4'-6"	1'-0"	(4) #5's EA WAY		
F5.0	5'-0"	5'-0"	1'-0"	(5) #5's EA WAY		

CONCRETE FOOTING SCHEDULE (ISOLATED PADS) NOT TO SCALE





KING STUD TABLE			
KING STUDS	OPENING WIDTH		
(2)	2'-1"		
(3)	6-'4"		
(4) 14'-9"			
(5)	19'-0"		
SILL	PLATE TABLE		
SILL	OPENING WIDTH		
SILL (1) 2x6	OPENING WIDTH 6'-6"		
SILL (1) 2x6 (2) 2x6			

	HANGER SCHEDULE					
X	HANGER TYPE					
1	HUC28-3 CONCEALED FACE MOUNT HANGER					
2	HUCQ610-SDS CONCEALED FACE MOUNT HANGER					
3	HUC210-2 CONCEALED FACE MOUNT HANGER					
4	IUS2.06/11.88 FACE MOUNT HANGER					
5	ABU88Z POST BASE W/ (2) 5/8"Øx5" TITEN HD ANCHORS. PACK GROUT IN SPACE BELOW COLUMN CAP PER SIMPSON STRONG-TIE INSTRUCTIONS					
6	IUS3.56/11.88 FACE MOUNT HANGER					
7	HANGER BY TRUSS MANUFACTURER					
8	LSU26 FACE MOUNT HANGER					
9	LSSU210 FACE MOUNT HANGER					
10	LUS214-2 FACE MOUNT HANGER					
11	LUS26-2 FACE MOUNT HANGER					
12	BA3.56/11.88 TOP FLANGE HANGER					
13	HUCQ412-SDS CONCEALED FLANGE HANGER					
14	ITS2.06/16 TOP FLANGE HANGER					
15	ITS3.56/16 TOP FLANGE HANGER					
16	HGU5.5-SDS H=11.813 FACE MOUNT HANGER					
17	LUS26 FACE MOUNT HANGER					

→ HANGER SCHEDULE

(2)	2'-1"					
(3)	6-'4"					
(4)	14'-9"					
(5)	19'-0"					
SILL PLATE TABLE						
SILL	OPENING WIDTH					
(1) 2x6	6'-6"					
(2) 2x6	9'-2"					
(3) 2x6	11'-3"					
(4) 2x6	13'-0"					

KING STUD & SILL PLATE

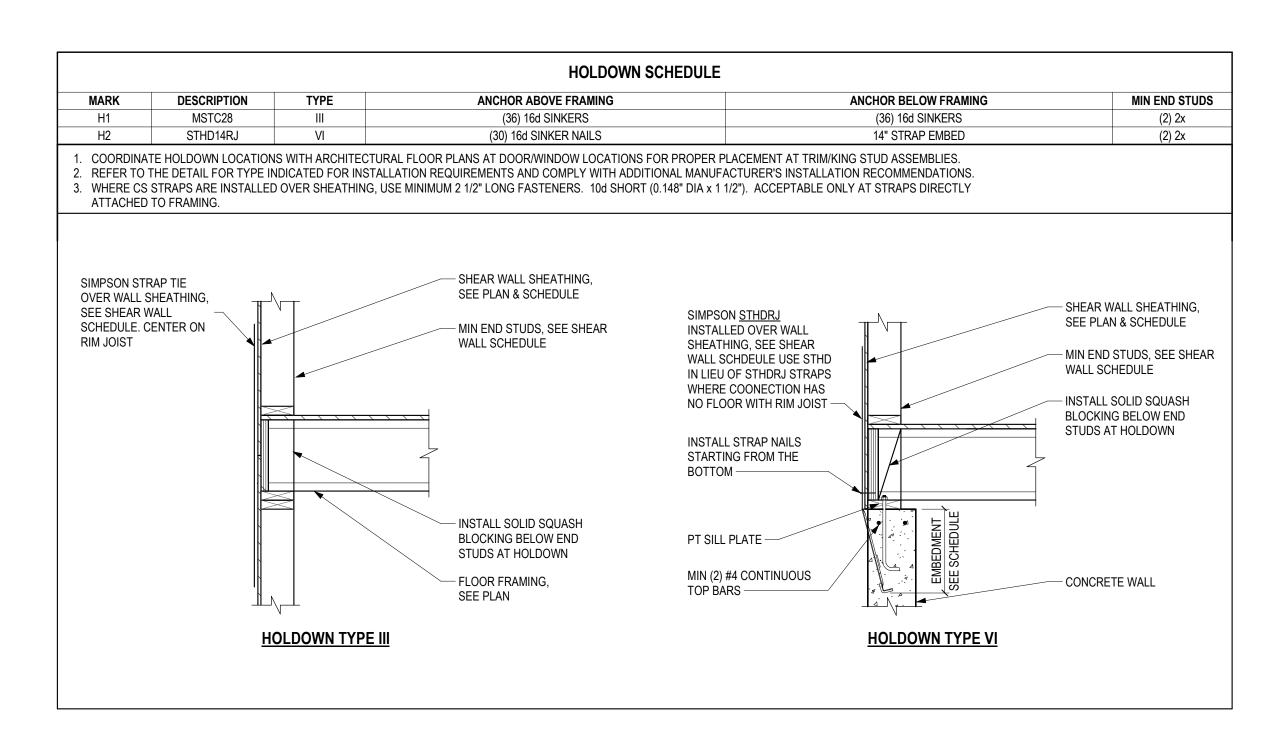
SCHEDULE

6

6	IOT TO SCALE			NOT TO SCALE				
	WOOD SHEAR WALL SCHEDULE							
		FASTENERS		SOLE PLATE CONNECTION (UNO)		TOP OF WALL C		
			PANEL EDGE			BLKG OR FRA		

WOOD SHEAR WALL SCHEDULE										
		FASTENERS		SOLE PLATE CON	TOP OF WALL CONN TO					
TAG	SHEATHING	TYPE/SIZE	PANEL EDGE SPACING	ANCHORS TO CONCRETE	CONNECTION TO FRAMING	BLKG OR FRAMING (UNO)				
SW1	1/2" GYP BD (BOTH SIDES)	#6x1 1/4" DRYWALL SCREW	8"	1/2"Ø @ 48"	12d GUN NAILS @ 8"	A35 CLIPS @ 16"				
SW2	R-6 ZIP SHEATHING	10d GUN NAIL	4"	1/2"Ø @ 24" OR 5/8"Ø @ 32"	12d GUN NAILS @ 3"	A35 CLIPS @ 12"				
SW3	R-6 ZIP SHEATHING	10d GUN NAIL	3"	1/2"Ø @ 24" OR 5/8"Ø @ 32"	12d GUN NAILS @ 3"	A35 CLIPS @ 12"				
SW4	7/16" (ONE SIDE)	8d GUN NAIL	4"	1/2"Ø @ 24" OR 5/8"Ø @ 32"	12d GUN NAILS @ 3"	A35 CLIPS @ 12"				





\ HOLDOWN SCHEDULE 4 4 NOT TO SCALE



