

943194001

943202001

RCRBD Record Set  
T.A.

03/20/2020

943203001

53

53

53

943301001

5N 88W

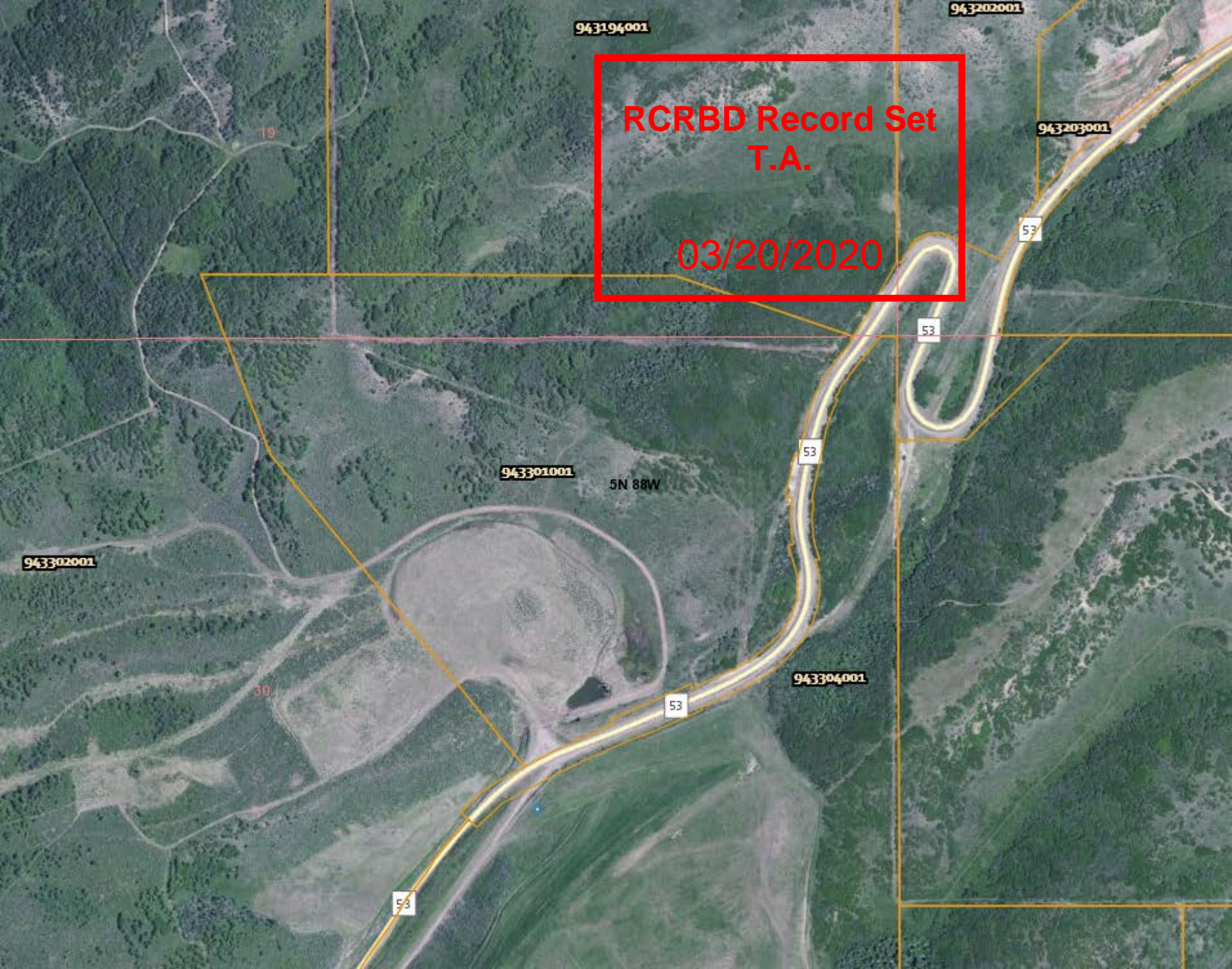
943302001

30

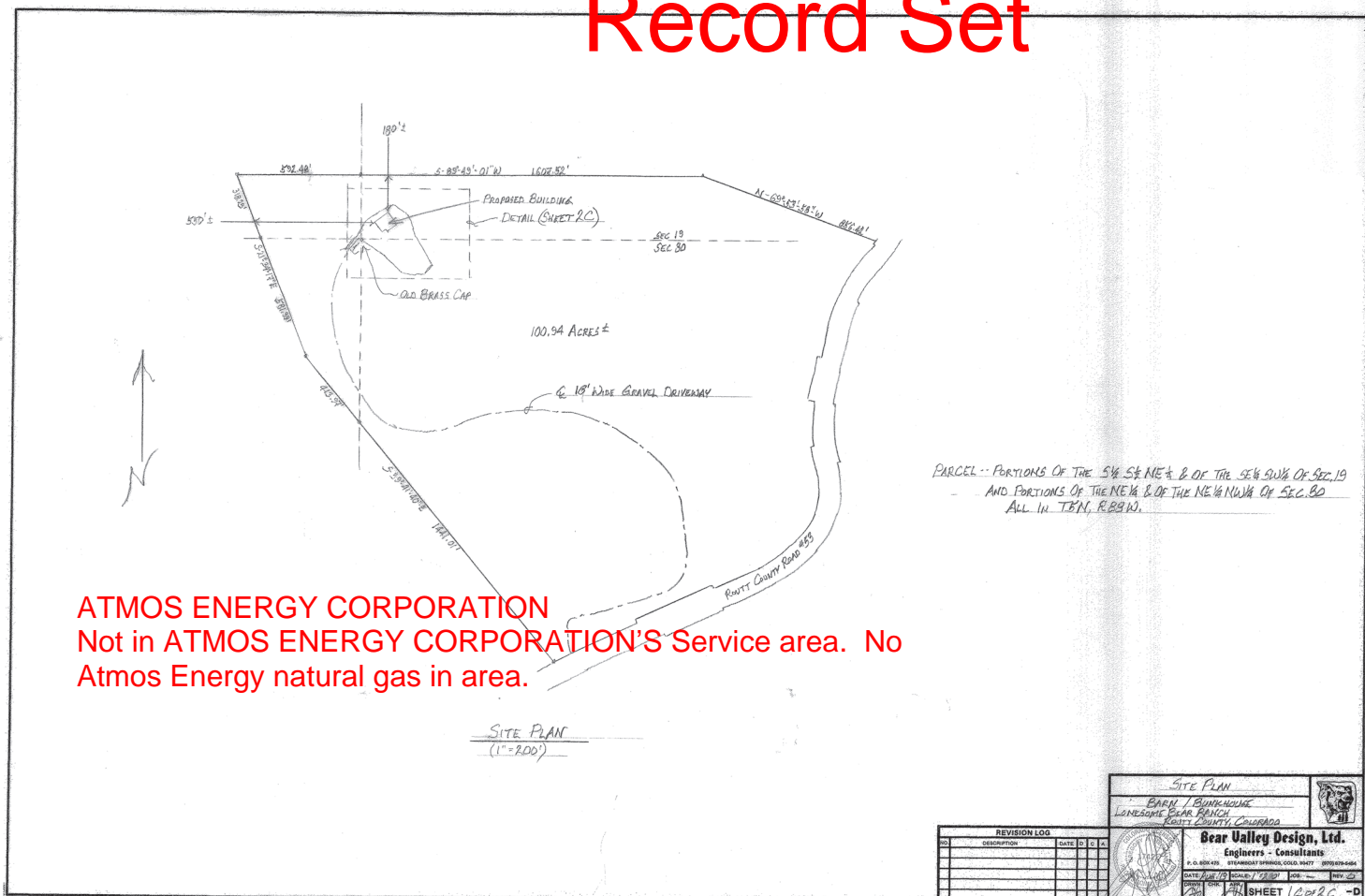
943304001

53

53



# RCRBD Record Set

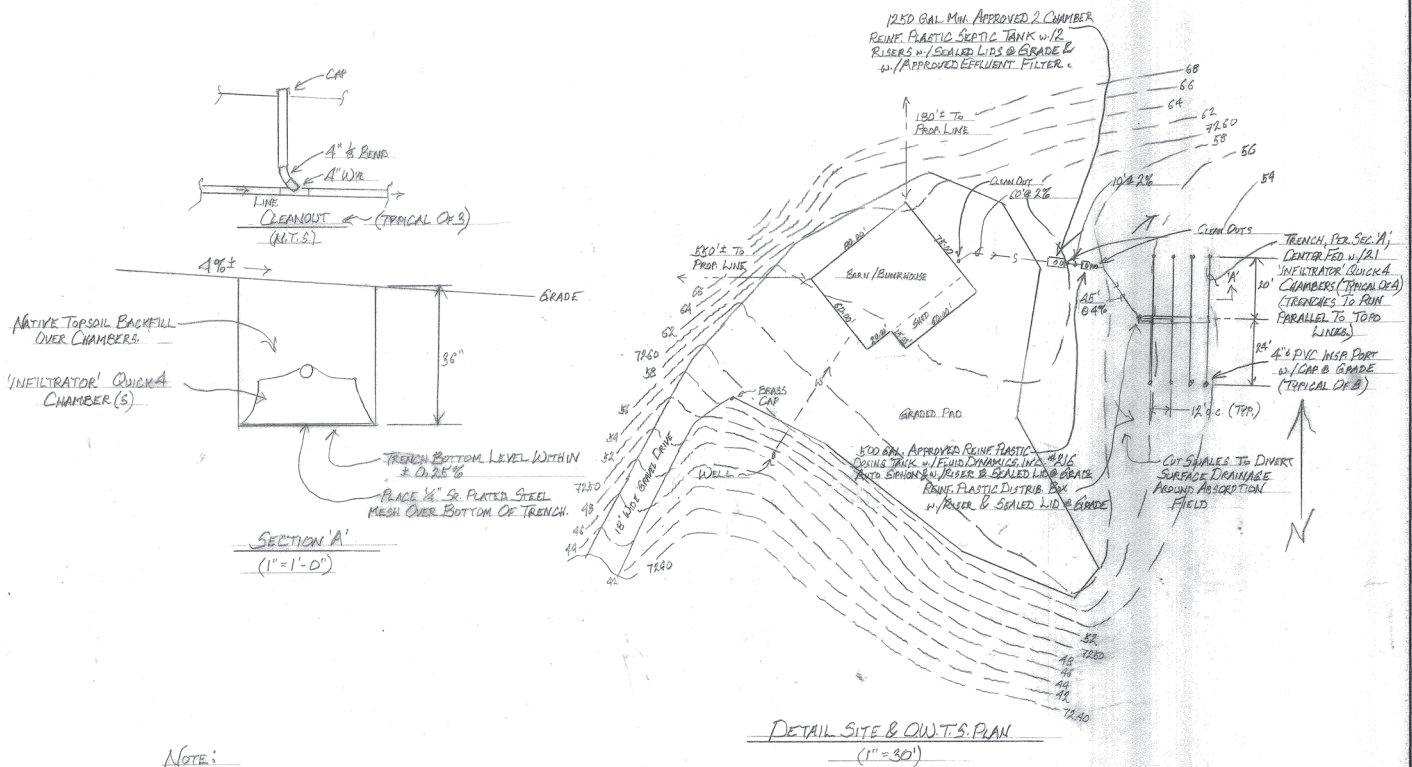


Meter Location will be determined after on site visit with YVEA Craig Field Representative. Meter & meter pedestal are to be located as described by YVEA. They shall not be enclosed, covered or concealed. Violation shall result in termination of service.

Yampa Valley Electric Assn. INC  
This Approval addresses only  
the meter and service location  
Approved: Scott Flowers - YVEA 10/01/19



# RCRBD Record Set



NOTE:

1. ALL SEWER & EFFLUENT LINES TO BE RUN IN 4", SDR 35 PVC, AS SHOWN & BEDDED IN SELECT GRANULAR MATERIAL.
2. FENCE ALL LIVESTOCK OFF OF ABSORPTION FIELD.

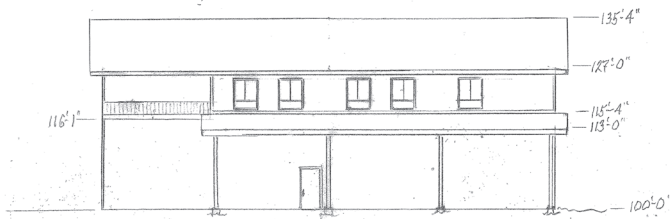
DETAIL SITE & Q.W.T.S. PLAN  
(1" = 30')

REVISION LOG			
NO.	DESCRIPTION	DATE	D

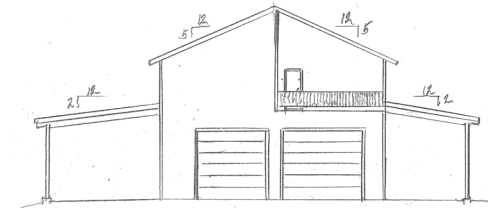
DETAIL SITE & DOWNS PLAN BARN / BUNKHOUSE LONGMONT BEAR RANCH FOUNTAIN COUNTY, COLORADO		
<b>Bear Valley Design, Ltd.</b> Engineers - Consultants P.O. BOX 418    STEARNS/SPRING, COLOR. 80477    (303) 678-6404		
	DATE: 10/15/95    SCALE: 1"=40'    SHEET: 20 OF 22    REV: 1/1	
DESIGN:    CADD:    APP:	SHEET 20 OF 22 - D	

# RCRBD Record Set T.A.

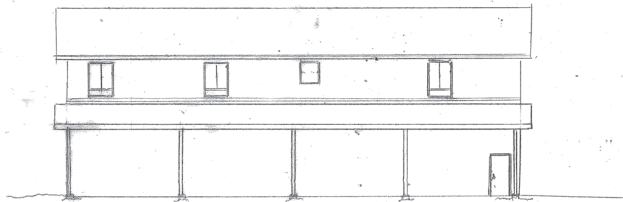
03/20/2020



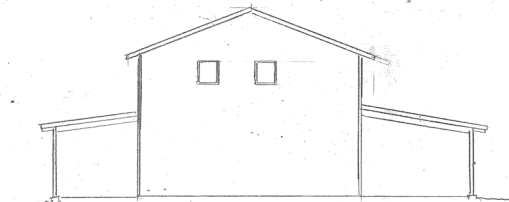
EAST ELEVATION  
( $\frac{1}{8}'' = 1'-0''$ )



SOUTH ELEVATION  
( $\frac{1}{8}'' = 1'-0''$ )



WEST ELEVATION  
( $\frac{1}{8}'' = 1'-0''$ )



NORTH ELEVATION  
( $\frac{1}{8}'' = 1'-0''$ )

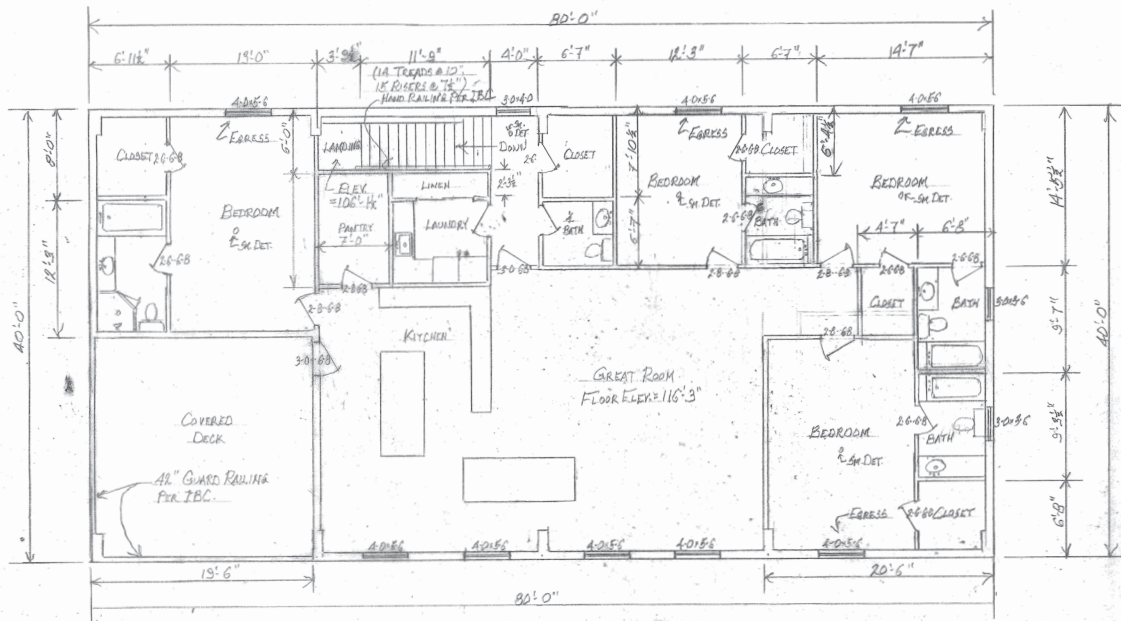
REVISION LOG				
NO.	DESCRIPTION	DATE	BY	CHK.

ELEVATIONS	
BARN / BUNKHOUSE	
LONESOME BEAR RANCH	
SOUTH COUNTY, COLORADO	
Bear Valley Design, Ltd.	
Engineers - Consultants	
P. O. BOX 438 STEADFAST SPRING, COLO. 80557	
DATE: 03/20/20	SCALE: 1/8" = 1'-0"
PROJECT: 2018-001	SHEET: 3A013A-0



# RCRBD Record Set T.A.

03/20/2020



UPPER FLOOR PLAN  
(1/4" = 1'-0")

NOTE:

ATTACHED PAGE (B & 11) OF NOTES APPLY THROUGHOUT.

REVISION LOG			
NO.	DESCRIPTION	DATE	BY

**UPPER FLOOR PLAN**  
BATH & BUNKHOUSE  
LONGHORN RANCH  
SOUTH COUNTY, COLORADO

**Bear Valley Design, Ltd.**  
Engineers - Consultants  
P.O. BOX 418 STEAMBOAT SPRING, COLO. 80487  
CONTACT: PH: 970.861.1212 FAX: 970.861.1213  
WWW.BEARVALLEYDESIGN.COM

**SHEET 1A OF A-D**

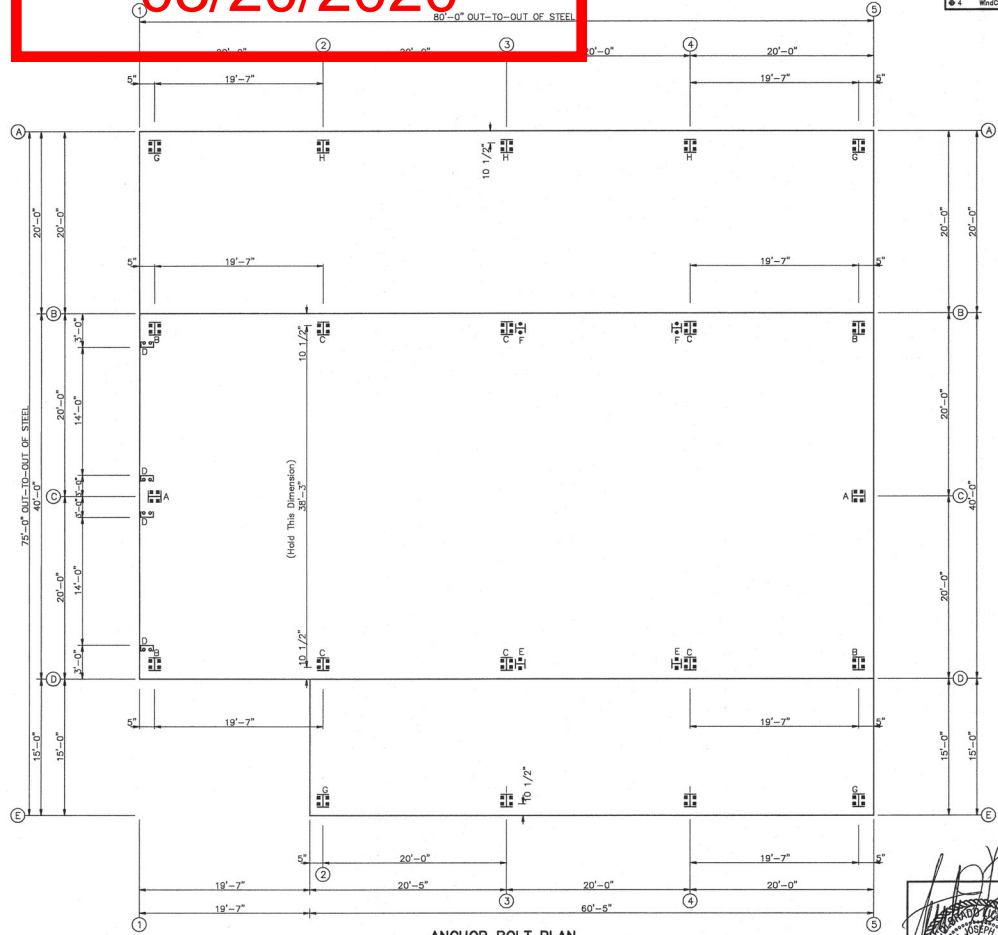
03/20/2020

<table style="width: 100%; border-collapse: collapse;"> <tr> <th colspan="5" style="text-align: center;">REVISION LOG</th> </tr> <tr> <th style="width: 10%;">NO.</th> <th style="width: 40%;">DESCRIPTION</th> <th style="width: 10%;">DATE</th> <th style="width: 10%;">BY</th> <th style="width: 10%;">A</th> </tr> <tr><td> </td><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td><td> </td><td> </td></tr> </table>		REVISION LOG					NO.	DESCRIPTION	DATE	BY	A																										<div style="text-align: center;"> <h2 style="margin: 0;">MAIN FLOOR PLAN</h2> <h3 style="margin: 0;">PARK &amp; BUILD HOUSE</h3> <p style="margin: 0;">LAWSON PARK RANCH</p> <p style="margin: 0;">RENT COUNTY, COLORADO</p> </div> <div style="text-align: right;">  </div> <div style="text-align: center; margin-top: 20px;">  </div> <div style="text-align: center; margin-top: 10px;"> <h2 style="margin: 0;">Bear Valley Design, Ltd.</h2> <h3 style="margin: 0;">Engineers - Consultants</h3> <p style="margin: 0;">P. O. BOX 470    STEADMAN SPRING, COLORADO 80757    (303) 670-0404</p> <p style="margin: 0;">DATE: <u>08/27/79</u>    DRAWN BY: <u>W. L. GIBSON</u>    REV: <u>0</u></p> <p style="margin: 0;">SHEET <u>1</u> OF <u>1</u>    <b>SHEET RACHA-D</b></p> </div>
REVISION LOG																																					
NO.	DESCRIPTION	DATE	BY	A																																	



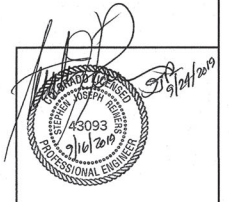


03/20/2020



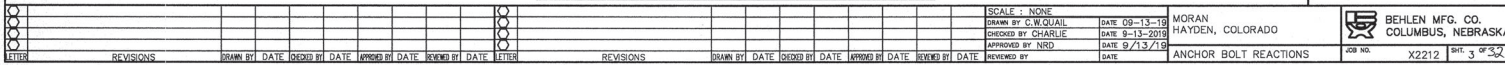
Qty	Locate	Dia (in)	Type	Proj (in)
8	Jamb	1/2"	F1554	1.00
8	Endwall	3/4"	F1554	2.50
78	Frame	3/4"	F1554	2.50
4	WindCol	3/4"	F1554	2.50
4	WindCol	1"	F1554	3.00

**ANCHOR BOLT PLAN**  
NOTE: All Base Plates @ 100'-0" (U.N.)

[illegible]



03/20/2020



03/20/2020

**NOTES:**

1. FOR BUILDINGS DESIGNED AS ENCLOSED, ALL WINDOWS, DOORS, AND LOUVERS SHALL BE RATED TO COMPLY WITH THE WIND DESIGN CRITERIA IDENTIFIED ON SHEET 1 OF THESE PLANS.
2. ALL FIELD LOCATED FRAMED OPENINGS WILL REQUIRE FIELD CUTTING OF GIRTS, PURLINS, AND SHEETING.

 BEHLEN MFG. CO.  
COLUMBUS, NEBRASKA

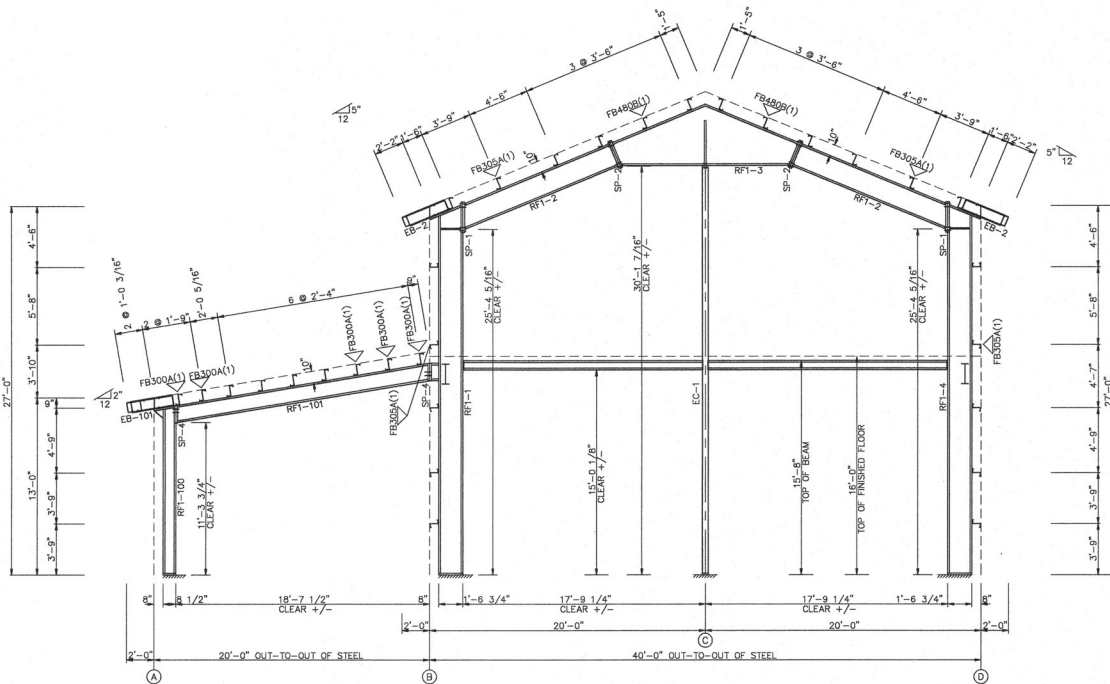


# RCRBD Record Set T.A.

03/20/2020

SPICE PLATE & BOLT TABLE											CAP PLATE		BOLTS			
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length	Mark	Qty	Type	Dia	Length	
SP-1	4	4	0	0	A325	0.750	2.50	1/2"	3/8"	2'-2 1/2"	EC-1	4	A325	0.500	1.50	
SP-2	4	4	0	0	A325	0.750	2.50	3/8"	3/8"	2'-0 7/8"						
SP-4	4	0	0	0	A325	0.750	2.50	3/8"	3/8"	1'-1"						

FLANGE BRACES: Both Sides(U.N.)  
A - L1.5x160  
B - L3.0x120

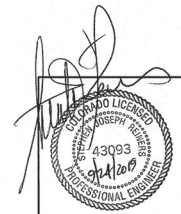


RIGID FRAME ELEVATION: FRAME LINE 1

## ERECTION NOTES:

1. THE "APPLICABLE WALL PANEL ERECTION GUIDE" IS TO BE USED IN CONJUNCTION WITH THESE DRAWINGS TO DETERMINE COMPLETE ERECTION REQUIREMENTS.
2. ALL FLANGE BRACING MUST BE INSTALLED AT FRAME LINES AS SHOWN.

RIGID FRAMES BY THIS MANUFACTURER ARE DESIGNED TO BE FASTENED USING A-325 HIGH STRENGTH BOLTS BY THE "SNUG-TIGHTENED" METHOD, AS DEFINED AND DESCRIBED IN THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS SPECIFICATION (RCS: 12-31-2009), SECTION 4.1, "SNUG-TIGHTENED JOINTS" (REFERENCE SECTION 8.1)




REVISIONS										REVISIONS									
DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY

SCALE: NONE	MORAN HAYDEN	CO 81639	BEHLEN MFG. CO. COLUMBUS, NEBRASKA
DRAWN BY: J. NELSON	DATE: 9/18/19		
CHECKED BY: J. NELSON	DATE: 9/20/19		
APPROVED BY: NRD	DATE: 9/20/19		
REVIEWED BY:	DATE:		

RIGID FRAME ELEVATION JOB NO. X2212 SHEET 9 OF 32

03/20/2020

03/20/2020


SCALE : NONE							
DRAWN BY	J. NELSON	DATE	9/16/19	MORAN HAYDEN		 BEHLEN MFG. CO. COLUMBUS, NEBRASKA	
CHECKED BY	J. NELSON	DATE	9/20/19	CO 81639			
APPROVED BY	NRD	DATE	9/20/19				
REVIEWED BY		DATE		RIGID FRAME ELEVATION			
				JOB NO.		X2212	SHT. 7 OF 32



03/20/2020

▽ FLANGE BRACES: Both Sides(U.N.)  
A - L1.5x16G  
B - L3.0x12G




SCALE: NONE										DRAWN BY: J. NELSON		DATE: 9/16/19		MORAN		CO 81639		 BEHLEN MFG. CO. COLUMBUS, NEBRASKA	
CHECKED BY: J. NELSON										DATE: 9/20/19									
APPROVED BY: NED										DATE: 9/20/19									
RIGID FRAME ELEVATION										JOB NO.		X2212		SHEET		8 OF 32			

03/20/2020




NOTE: JB-1 THRU JB-4 @ 1/3 POINTS.

 - DENOTES FIELD LOCATED ACCESSORY  
(SEE ACCESSORY SHEET)

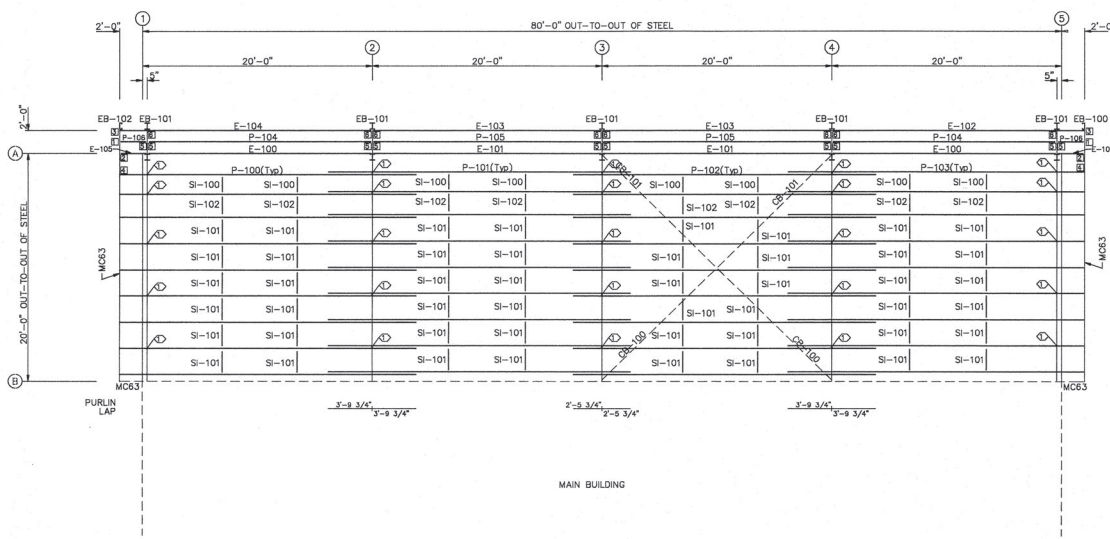
THE ARROW ON THE BARCODE LABEL POINTS TO THE NARROWER FLANGE FOR 8" ZEES;  
ON ALL OTHER PROFILES, THE ARROW TYPICALLY POINTS TO THE OUTSIDE FLANGE  
UNLESS DETAILS INDICATE OTHERWISE.



(SEE PREVIOUS SHEET)										UNLESS STATED OTHERWISE										SCALE: NONE				MORAN		 BEHLEN MFG. CO. COLUMBUS, NEBRASKA			
																				DRAWN BY: J. NELSON				DATE: 9/16/19				CO 81639	
																				CHECKED BY: J. NELSON				DATE: 9/20/19					
																				APPROVED BY: NRD				DATE: 9/23/19					
																				REVISIONS						ROOF FRAMING		JOB NO. X2212	
																				DRAWN BY: DATE: CHECKED BY: DATE: APPROVED BY: DATE: REVIEWED BY: DATE: FILTER						SHEET 9 OF 32			
FILTER										REVISIONS										DRAWN BY: DATE: CHECKED BY: DATE: APPROVED BY: DATE: REVIEWED BY: DATE: FILTER									

03/20/2020

CONNECTION PLATES	
ROOF PLAN	
ID	MARK / PART
1	CP102
2	CP217
3	CP215
4	CP216
5	CP167
6	CP179




ROOF FRAMING PLAN



TO FACILITATE THE PROPER ORIENTATION OF PURLINS/BUNDLES WHEN UNLOADING OR PLACING ON THE ROOF, POSTPOXA THE END OF THE PURLIN/BUNDLE THAT IS TAGGED WITH A BAR CODE LABEL TO THE LEFT WHEN STANDING ON THE OUTSIDE OF THE BUILDING LOOKING IN; THE TOP FLANGE OF ZEE PURLINS SHOULD FACE UPHILL UNLESS DETAILS INDICATE OTHERWISE.

THE ARROW ON THE BARCODE LABEL POINTS TO THE NARROWER FLANGE FOR 8" ZEE'S; ON ALL OTHER PROFILES, THE ARROW TYPICALLY POINTS TO THE OUTSIDE FLANGE UNLESS DETAILS INDICATE OTHERWISE.

										SCALE : NONE		DRAWN BY : J. NELSON		DATE : 9/16/78		MORAN		CO 81639		 BEHLEN MFG. CO. COLUMBUS, NEBRASKA	
										CHECKED BY : J. NELSON		DATE : 9/20/78									
										APPROVED BY : NRD		DATE : 9/23/78									
REVISIONS DRAWN BY DATE DESIGNED BY DATE APPROVED BY DATE REVIEWED BY DATE LETTER										REVISIONS DRAWN BY DATE DESIGNED BY DATE APPROVED BY DATE REVIEWED BY DATE										ROOF FRAMING JOB NO. X2212 SHEET 10 OF 32	



03/20/2020

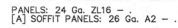


CONNECTION PLATES	
ROOF PLAN	
ID	MARK / PART
1	CP102
2	CP217
3	CP215
4	CP216
5	CP167
6	CP179

[illegible]

03/20/2020

TRIM TABLE	
ROOF PLAN	
ID	PART
1	TFE?
2	TE32
3	MZ16
4	TW33
5	TBQ?
6	TE13
7	TR84
8	TE101-20
9	TR83



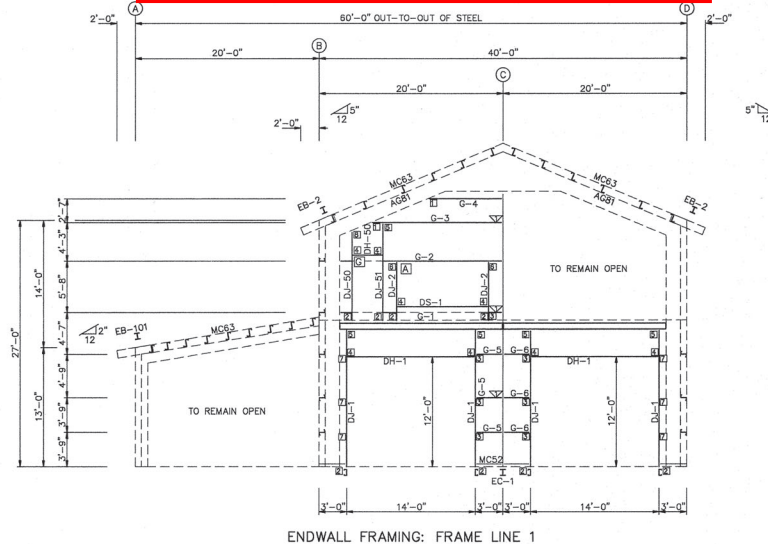
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												DRAWN BY J. NELSON		DATE 9/16/78		CO 81639		COLUMBUS, NEBRASKA					
												CHECKED BY J. NELSON		DATE 9/20/78									
												APPROVED BY NED		DATE 9/23/78									
																ROOF SHEETING		JOB NO. X2212 SHEET 12 OF 32					
REVISIONS												REVISIONS											
DRAWN BY DATE CHECKED BY DATE APPROVED BY DATE REVIEWED BY DATE												DRAWN BY DATE CHECKED BY DATE APPROVED BY DATE REVIEWED BY DATE											



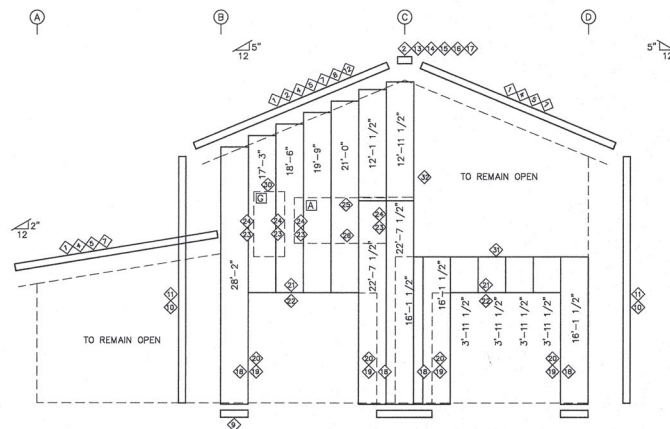




03/20/2020




ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gc. A2 -

 - DENOTES FIELD LOCATED ACCESSORY  
(SEE ACCESSORY SHEET)

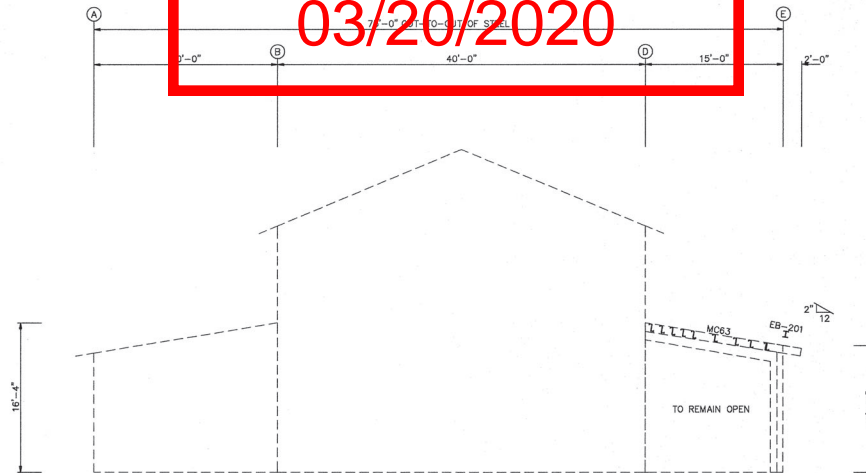
[illegible]

03/20/2020

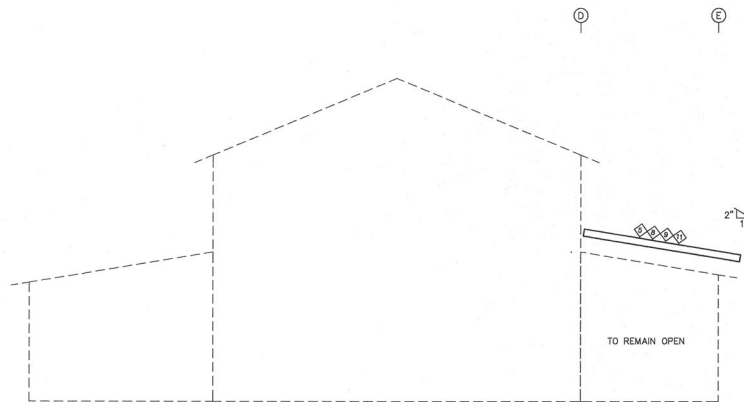
MEMBER TABLE	
FRAME LINE 2	
MARK	LENGTH
EB-201 W0X12	3-0 5/8"

TRIM TABLE	
FRAME LINE 2	
OID PART	
5	M216
8	TR82-20
9	TR66-20
11	TW29




ENDWALL FRAMING: FRAME LINE 2



ENDWALL TRIM: FRAME LINE 2

 - DENOTES FIELD LOCATED ACCESSORY  
(SEE ACCESSORY SHEET)



																SCALE : NONE																MORAN HAYDEN																BEHNEN MFG. CO. COLUMBUS, NEBRASKA																															
																DRAWN BY : J. NELSON																DATE : 9/16/79																CO 81639																															
																CHECKED BY : J. NELSON																DATE : 9/20/79																																															
																APPROVED BY : NRD																DATE : 9/23/79																																															
FILTER      REVISIONS      DRAWN BY      DATE      CHECKED BY      DATE      APPROVED BY      DATE      REVIEWED BY      DATE      FILTER																REVISIONS      DRAWN BY      DATE      CHECKED BY      DATE      APPROVED BY      DATE      REVIEWED BY      DATE																ENDWALL FRAMING																JOB NO.      X2212      SHEET NO.      16 OF 32																															



03/20/2020



PANELS: 26 Ga. A2 -

																								SCALE: NONE												MORAN																																															
																								DRAWN BY J. NELSON												DATE 9/16/19												HAYDEN												CO 81639												BEHLN MFG. CO. COLUMBUS, NEBRASKA											
																								CHECKED BY J. NELSON												DATE 9/20/19																																															
																								APPROVED BY NRD												DATE 9/23/19																																															
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03/20/2020

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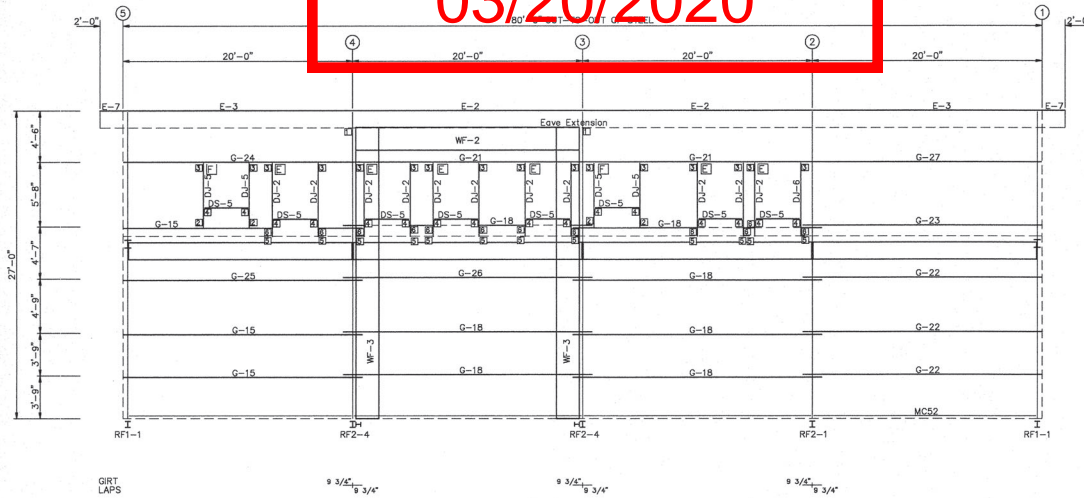
03/20/2020

[illegible]

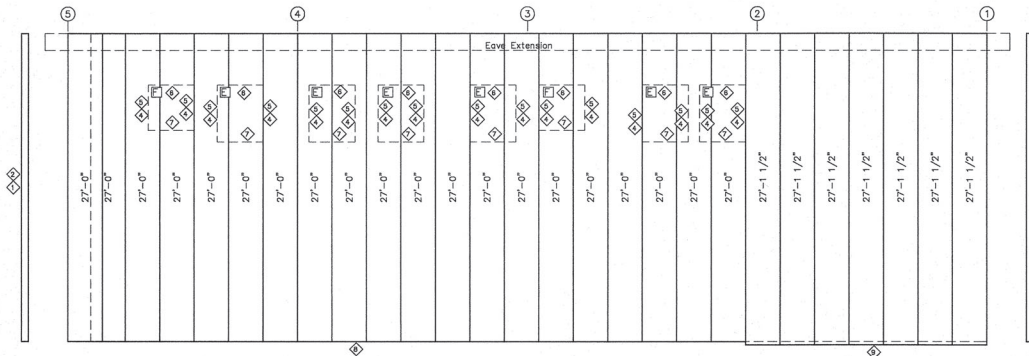


# RCRBD Record Set T.A.

03/20/2020



SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 28 Gg. A2 -

BOLT TABLE				
LOCATION	FRAME LINE	EQW	TYPE	LEN
WF-3 - WF-2	8	A325	3/4"	2 1/2"
WF-3 - RF2-4	4	A325	3/4"	1 1/2"

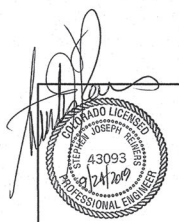
MEMBER TABLE				
MEMBER	TYPE	LEN	THICK	WIDTH
WF-2	B2480819	15'-0"	1/2"	15'-0"
WF-3	B2480819	25'-0"	1/2"	15'-0"
DJ-2	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-3	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-4	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-5	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-6	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-7	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-8	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-9	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-10	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-11	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-12	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-13	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-14	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-15	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-16	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-17	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-18	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-19	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-20	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-21	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-22	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-23	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-24	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-25	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-26	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-27	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-28	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-29	BC16	25'-3 1/4"	1/4"	15'-0"
DJ-30	BC16	25'-3 1/4"	1/4"	15'-0"

CONNECTION PLATES				
FRAME LINE	TYPE	LEN	THICK	WIDTH
WF-2	CP150	15'-0"	1/2"	15'-0"
WF-3	CP150	25'-0"	1/2"	15'-0"
DJ-2	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-3	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-4	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-5	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-6	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-7	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-8	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-9	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-10	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-11	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-12	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-13	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-14	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-15	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-16	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-17	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-18	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-19	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-20	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-21	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-22	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-23	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-24	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-25	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-26	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-27	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-28	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-29	CP150	25'-3 1/4"	1/4"	15'-0"
DJ-30	CP150	25'-3 1/4"	1/4"	15'-0"


  

TRIM TABLE				
FRAME LINE	TYPE	LEN	THICK	WIDTH
WF-2	TW-12	15'-0"	1/2"	15'-0"
WF-3	TW-16	25'-0"	1/2"	15'-0"
DJ-2	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-3	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-4	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-5	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-6	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-7	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-8	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-9	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-10	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-11	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-12	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-13	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-14	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-15	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-16	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-17	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-18	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-19	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-20	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-21	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-22	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-23	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-24	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-25	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-26	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-27	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-28	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-29	TW-16	25'-3 1/4"	1/4"	15'-0"
DJ-30	TW-16	25'-3 1/4"	1/4"	15'-0"



□ - DENOTES FIELD LOCATED ACCESSORY  
(SEE ACCESSORY SHEET)

REVISIONS				REVISIONS				REVISIONS				REVISIONS			
DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY	DATE	BY

SCALE : NONE					
DRAWN BY J. NELSON	DATE 9/16/19	MORAN HAYDEN	CO 81639		BEHLEN MFG. CO.
CHECKED BY J. NELSON	DATE 9/20/19				COLUMBUS, NEBRASKA
APPROVED BY NRD	DATE 9/23/19				
REVIEWED BY	DATE				
SIDEWALL FRAMING				JOB NO.	X2212
					SHT. 20 OF 32

03/20/2020

[illegible]

Technical drawing of an open patio area showing sheeting and trim layout. The drawing includes a series of vertical panels with heights of 17'-7", 16'-4", 15'-1", 13'-10", 12'-7", 11'-4", and 10'-1". A horizontal dimension of 20'-0" is shown at the top. A sloped line with a 5°/12 pitch is indicated. A dashed line represents the 15'-0" frame line. A 116°-0" angle is marked at the bottom left. The drawing is labeled "OPEN PATIO AREA SHEETING & TRIM: 15'-0" FRAME LINE" and "PANELS: 26 Ga. A2 - .". The drawing is dated 11/1/11 and includes a scale of 1/8" = 1'-0".

[illegible]







[illegible][illegible]

# RCRBD Record Set T.A.

03/20/2020

DECK : 1.5C22

DECK ERECTION:

PLACE DECK SHEETS SIDE TO SIDE BEGINNING AT THE CORNER OF THE MEZZANINE, MAINTAINING ALIGNMENT. WHEN LAPPING, MAKE ALIGNMENT ADJUSTMENTS IF NECESSARY. PLACE SHEETS WITH EDGES UP, MAKE SIDE LAPS ONE-HALF CORRUGATION. DO NOT STAGGER END LAPS. MINIMUM BEARING OF THE SHEETS SHALL BE 1 1/2".

SHEETS SHALL BE ATTACHED TO SUPPORTS WITH SCREWS. MINIMUM SCREW REQUIREMENTS ARE AS FOLLOWS:

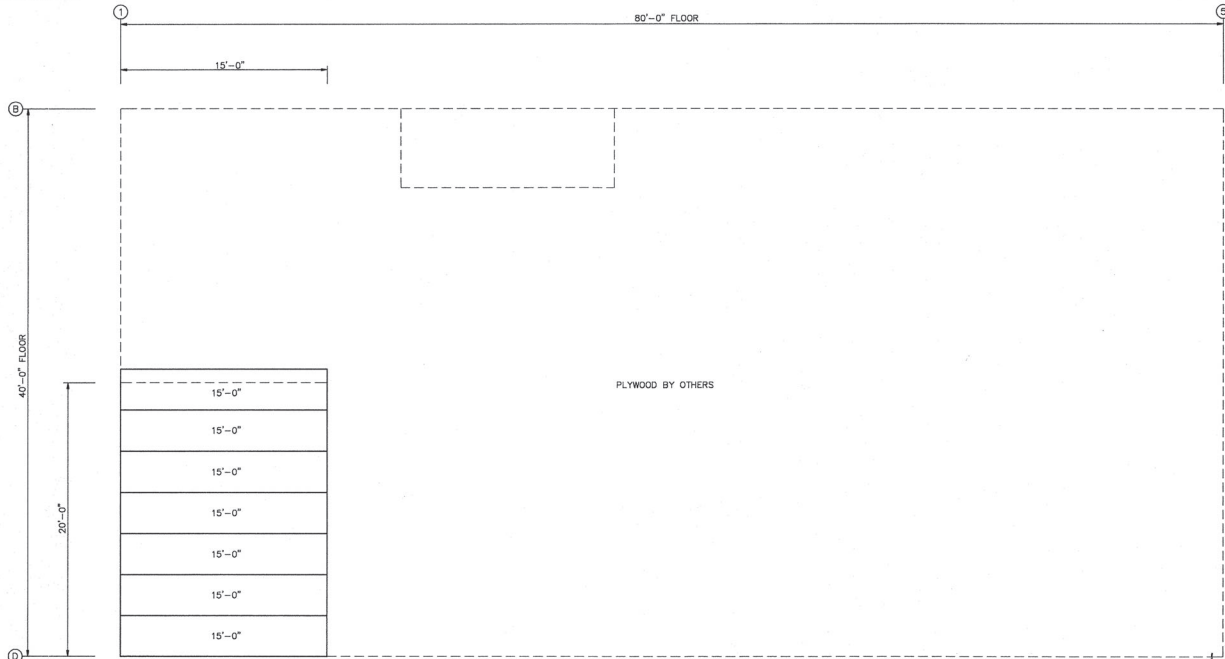
- A) END OF SHEETS:  
SCREW EACH END TO EACH SIDE LAP PLUS ONE INTERMEDIATE LOCATION. ((4) #12 SCREWS PER SHEET.)
- B) END LAPS:  
SCREW EACH END LAP AT EACH SIDE LAP PLUS ONE INTERMEDIATE LOCATION. ((4) #12 SCREWS PER SHEET.)
- C) SIDE LAPS:  
USE (1) #12 SCREW AT EACH SUPPORT

FIELD CUT DECKING AS REQUIRED.

DECK : PLYWOOD BY OTHERS

DECK ERECTION:

DESIGN AND SUPPLY ATTACHMENT OF WOOD DECK TO JOIST BY OTHERS.  
SHEAR TO BE RESISTED BY SCREWS IS 110#/FT (0.7E)



## MEZZANINE DECKING PLAN

DECK: 1.5C22

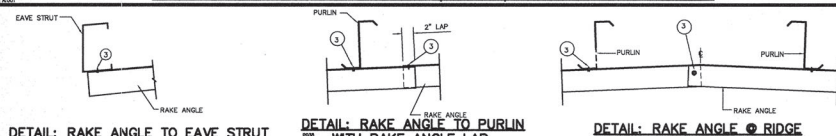
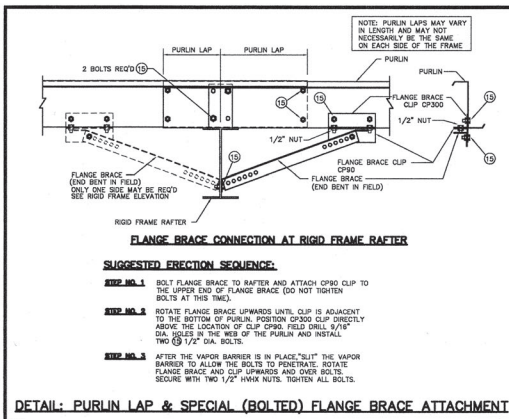
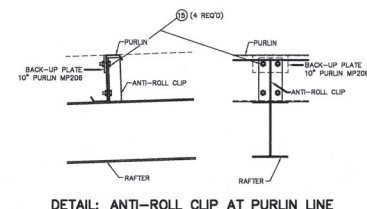
NOTE : - FIELD CUT DECK AT OPENING AS REQ'D



SCALE : NONE	MORAN	CO	81639	BEHLEN MFG. CO.
DRAWN BY : J. NELSON	HAYDEN			COLUMBUS, NEBRASKA
CHECKED BY : J. NELSON				
DATE : 9/16/19				
DATE : 9/20/19				
APPROVED BY : NRD				
DATE : 9/23/19				
REVIEWED BY :				
DATE :				
FLOOR SHEETING & TRIM				
JOB NO.				
X2212				
SHEET				
25				
32				



03/20/2020



PANEL		PASTHENER SCHEDULE		WALL INDEX	FINISH	LOCATION
1	100	100	100	100	100	100
2	100	100	100	100	100	100
3	100	100	100	100	100	100
4	100	100	100	100	100	100
5	100	100	100	100	100	100
6	100	100	100	100	100	100
7	100	100	100	100	100	100
8	100	100	100	100	100	100
9	100	100	100	100	100	100
10	100	100	100	100	100	100
11	100	100	100	100	100	100
12	100	100	100	100	100	100
13	100	100	100	100	100	100
14	100	100	100	100	100	100
15	100	100	100	100	100	100
16	100	100	100	100	100	100
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18	100	100	100	100	100	100
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20	100	100	100	100	100	100
21	100	100	100	100	100	100
22	100	100	100	100	100	100
23	100	100	100	100	100	100
24	100	100	100	100	100	100
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27	100	100	100	100	100	100
28	100	100	100	100	100	100
29	100	100	100	100	100	100
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37	100	100	100	100	100	100
38	100	100	100	100	100	100
39	100	100	100	100	100	100
40	100	100	100	100	100	100
41	100	100	100	100	100	100
42	100	100	100	100	100	100
43	100	100	100	100	100	100
44	100	100	100	100	100	100
45	100	100	100	100	100	100
46	100	100	100	100	100	100
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59	100	100	100	100	100	100
60	100	100	100	100	100	100
61	100	100	100	100	100	100
62	100	100	100	100	100	100
63	100	100	100	100	100	100
64	100	100	100	100	100	100
65	100	100	100	100	100	100
66	100	100	100	100	100	100
67	100	100	100	100	100	100
68	100	100	100	100	100	100
69	100	100	100	100	100	100
70	100	100	100	100	100	100
71	100	100	100	100	100	100
72	100	100	100	100	100	100
73	100	100	100	100	100	100
74	100	100	100	100	100	100
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83	100	100	100	100	100	100
84	100	100	100	100	100	100
85	100	100	100	100	100	100
86	100	100	100	100	100	100
87	100	100	100	100	100	100
88	100	100	100	100	100	100
89	100	100	100	100	100	100
90	100	100	100	100	100	100
91	100	100	100	100	100	100
92	100	100	100	100	100	100
93	100	100	100	100	100	100
94	100	100	100	100	100	100
95	100	100	100	100	100	100
96	100	100	100	100	100	100
97	100	100	100	100	100	100
98	100	100	100	100	100	100
99	100	100	100	100	100	100
100	100	100	100	100	100	100

UNLESS OTHERWISE NOTED, CONNECTIONS BY THIS MANUFACTURER USING A-325 HIGH STRENGTH BOLTS ARE DESIGNED TO BE FASTENED USING THE METHOD NOTED IN THE METHOD OF JOINTS AND END PLATE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS SPECIFICATION (MISC. 6-83-2000), SECTION 41 "WELD-TIGHTENED JOINTS" (REFERENCE SECTION A.1).

NOTES:

1. FOR SEQUENCE OF ERECTION - SEE APPLICABLE WALL PANEL ERECTION GUIDE.
2. FOR FLANGE BEAM LOCATIONS - SEE FRAME CROSS SECTION AND ROOF FRAMING PLANS.
3. SOME FIELD DRILLING AND/OR FIELD CUTTING OF STEEL COMPONENTS MAY BE REQUIRED DURING THE ERECTION OF THIS BUILDING.
4. THE LENGTH OF THE FLANGE BEAM REQUIRED AT EACH LOCATION IS TO BE DETERMINED USING THE STANDARD LAP JOINT LOCATION (1'-8", 1'-4", 2'-0" OR 3'-0" IS TO BE USED).
5. ATTACH FLANGE BEAM TO HOLE IN GIRT W/HEAVY BOLT. SET THE FLANGE BEAM LENGTH SUPPLIED FOR THE LOCATION.

[illegible]

03/20/2020

[illegible]



# RCRBD Record Set T.A.

03/20/2020

## ASHA GIRT CONNECTION REQUIREMENTS

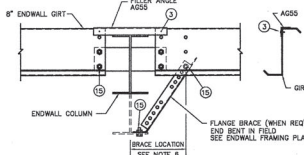
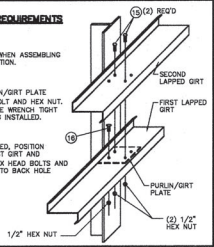
USE THE FOLLOWING PROCEDURE WHEN ASSEMBLING A LAPSED BY-PASS GIRT CONNECTION.

### STEP NO. 1

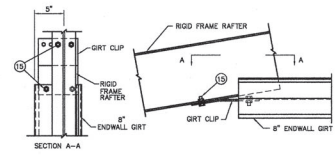
ATTACH FIRST GIRT TO THE PURLIN/GIRT PLATE WITH A (5) 1/2" X 1/4" FLAT HEAD BOLT AND HEX NUT. THE BOLT/NUT ASSEMBLY MUST BE WINDON TIGHT PRIOR TO THE SECOND GIRT BEING INSTALLED.

### STEP NO. 2

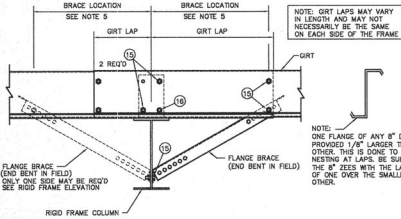
LEAVING FLAT HEAD BOLT INSTALLED, POSITION HOLES IN SECOND GIRT OVER FIRST GIRT AND INSTALL TWO (2) 1/2" X 1/4" FLAT HEAD BOLTS AND HEX NUTS IN A DIAGONAL FRONT TO BACK HOLE PATTERN.



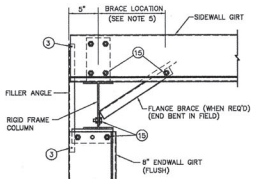
NOTE: CEE GIRT CONNECTION SIMILAR.  
**DETAIL: GIRT TO ENDWALL COLUMN  
W/ 1" GIRT PROJECTION**



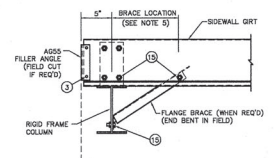
**DETAIL: GIRT TO ENDWALL RAFTER**



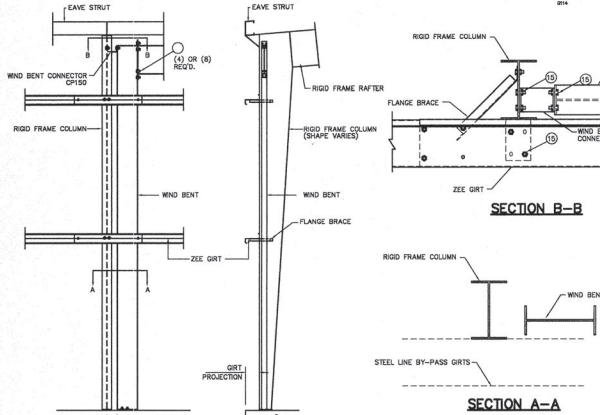
**DETAIL: GIRT LAP & FLANGE BRACE PLACEMENT**



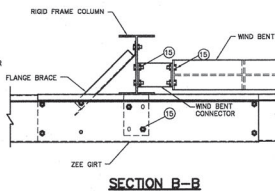
NOTE: CEE GIRT CONNECTION SIMILAR.  
**DETAIL: GIRT CONNECTION AT CORNER**



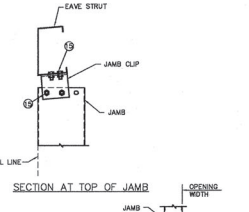
**DETAIL: GIRT CONNECTION AT CORNER**



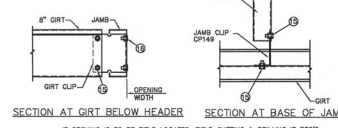
**DETAIL: WIND BENT AT RIGID FRAME COLUMN W/ BY-PASS GIRTS  
W/ 2 BOLT STANDARD WIND BENT CONNECTOR**



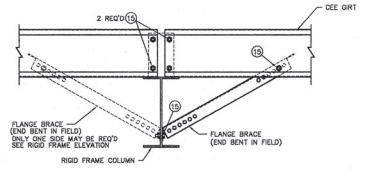
**SECTION B-B**



**SECTION A-A**



**DETAIL: JAMB CONNECTIONS FOR OPEN PATIO AREA**

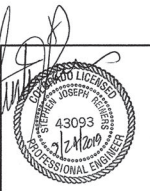


**DETAIL: CEE GIRT CONNECTION & FLANGE BRACE PLACEMENT**

ITEM NO.		DESCRIPTION	QUANTITY	UNIT	REMARKS
11	AS NOTED ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
12	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
13	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
14	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
15	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
16	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
17	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
18	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
19	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
20	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
21	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
22	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
23	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
24	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
25	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
26	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
27	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
28	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
29	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
30	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
31	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
32	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
33	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
34	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
35	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
36	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
37	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
38	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
39	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
40	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
41	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
42	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
43	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
44	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
45	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
46	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
47	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
48	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
49	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
50	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
51	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
52	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
53	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
54	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
55	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
56	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
57	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
58	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
59	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
60	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
61	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
62	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
63	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
64	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
65	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
66	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
67	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
68	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
69	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
70	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
71	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
72	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
73	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
74	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
75	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
76	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
77	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
78	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
79	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
80	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
81	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
82	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
83	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
84	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
85	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
86	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
87	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
88	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
89	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
90	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
91	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
92	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
93	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
94	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
95	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
96	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
97	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
98	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
99	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING
100	3/8" DIA. NUTS ON RIGID FRAME ELEVATION	1	EA		DO NOT DRILLING

UNLESS OTHERWISE NOTED, CONNECTIONS BY THIS MANUFACTURER USING 3/8" - 330 HIGH STRENGTH BOLTS ARE DESIGNED TO BE FASTENED USING THE "SHAKE TIGHTENED" METHOD. AS DEFINED AND DESCRIBED IN THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS SPECIFICATION (RSCC, 6-23-2000), SECTION 4.1 "SHAKE-TIGHTENED JOINTS" (REFERENCE SECTION 4.1).
<p>NOTES:</p> <ol style="list-style-type: none"> <li>1. FOR SEQUENCE OF LOCATION - SEE APPLICABLE WALL PANEL ERECTION GUIDE.</li> <li>2. FOR FLANGE BRACE ORIENTATION - SEE FRAME CROSS SECTION AND ROOF FRAMING PLAN.</li> <li>3. SOME FIELD CUTTING AND/OR FIELD DOUTING OF STEEL COMPONENTS MAY BE REQUIRED DURING THE ERECTION OF THIS BUILDING.</li> <li>4. THE LENGTH OF THE FLANGE BRACE SUPPLIED AT EACH LOCATION TO DETERMINE WHICH STANDARD LAP HOLE LOCATION (A, 1-1, 2-3, 0-3 OR 3-3)</li> <li>5. ATTACH FLANGE BRACE TO HOLE IN GIRT WHICH BEST FITS THE FLANGE BRACE LENGTH SUPPLIED FOR THE LOCATION.</li> </ol>

REVISIONS	DATE	BY	DATE	BY	DATE	BY	DATE	BY
1	9/18/19	J. NELSON	9/20/19	J. NELSON	9/23/19	J. NELSON		

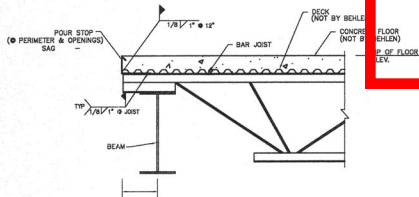




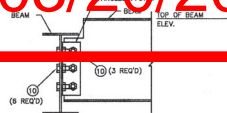
03/20/2020

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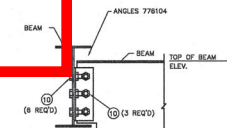
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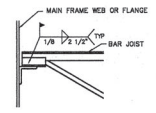
DETAIL: POUR STOP & CONCRETE



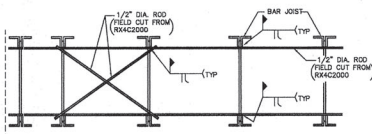
DETAIL: MEZZANINE BEAM TO COLUMN



DETAIL: MEZZANINE BEAM TO COLUMN

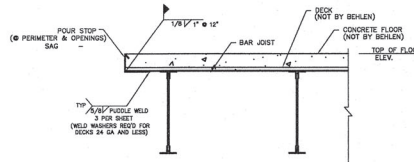


DETAIL: JOIST CONNECTION

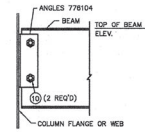


NOTE: TOP CHORD IS DIRECTLY ABOVE BOTTOM CHORD

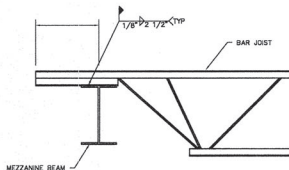
**DETAIL: HORIZONTAL ROD BRIDGING TO BAR JOIST**



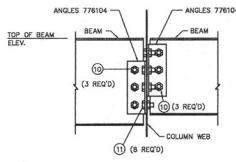
DETAIL: POUR STOP & CONCRETE



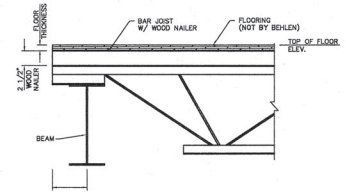
DETAIL: MEZZANINE BEAM TO COLUMN



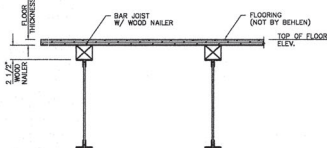
DETAIL: BAR JOIST CONNECTION @ END



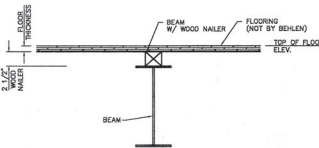
DETAIL: BEAM TO COLUMN



DETAIL: BAR JOIST WITH WOOD NAILER



DETAIL: BAR JOIST WITH WOOD NAILER




DETAIL: BEAM WITH WOOD NAILER

[illegible]

UNLESS OTHERWISE NOTED, CONNECTIONS BY THIS MANUFACTURER USING A-325 HIGH STRENGTH BOLTS ARE DESIGNED TO BE FASTENED USING THE "SNUG TIGHTENED" METHOD, AS DEFINED AND DESCRIBED IN THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS SPECIFICATION (RCS, 6-23-2000 SECTION 4.1 "SNUG-TIGHTENED JOINTS" (REFERENCE SECTION 8.1).

NOTES:

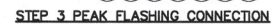
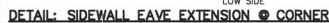
1. FOR SEQUENCE OF ERECTION -- SEE APPLICABLE WALL PANEL ERECTION GUIDE.
2. FOR FLANGE BRACE LOCATIONS -- SEE FRAME CROSS SECTION AND ROOF FRAMING PLANS.
3. SOME FIELD DRILLING AND/OR FIELD CUTTING OF STEEL COMPONENTS MAY BE REQUIRED DURING OF ERECTION OF THIS BUILDING.
4. THE LENGTH OF THE FLANGE BRACE SUPPLIED EACH LOCATION WILL DETERMINE WHICH STANDARD LAP HOLE LOCATION (A, 1-B, 2-B OR 3-B) IS TO BE USED.
5. ATTACH FLANGE BRACE TO HOLE IN GIRT WHICH BEST FITS THE FLANGE BRACE LENGTH SUPPLIED FOR THE LOCATION.

[illegible]

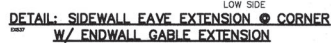
BEHLEN MFG. CO.  
COLUMBUS, NEBRASKA



03/20/2020



DETAIL: RAKE TRIM ATTACHMENT AT BUILDING PEAK (GABLE EXTENSION)



UNLESS OTHERWISE NOTED, CONNECTIONS BY THIS MANUFACTURER USING A-325 HIGH STRENGTH BOLTS ARE DESIGNED TO BE FASTENED USING THE "BOLT TIGHTENING METHOD" AS SPECIFIED IN THE AISC STEEL Erection Manual, SECTION 10 ON STRUCTURAL CONNECTIONS SPECIFICATION (FIGURE 8-23-2000).  
SIGHT ALL "BOLT-TIGHTENED JOINTS" (REFERENCE SECTION 6.1.1).

NOTES:

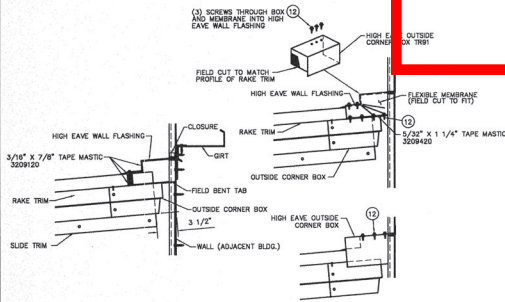
1. FOR SEQUENCE OF ERECTION - SEE PLACEMENT WALL PANEL Erection GUIDE.
2. FOR FLANGE BRACE LOCATIONS - SEE APPRAISE CROSS SECTION AND ROOF FRAMING PLAN.
3. SOME FIELD DRILLING AND/OR FIELD CUTTING OF STEEL COMPONENTS MAY BE REQUIRED DURING THE ERECTION OF THIS BUILDING.
4. THE LENGTH OF THE FLANGE BRACE SUPPLIED AT EACH LOCATION WILL DETERMINE WHICH STANDARD TEST FITS (1'-0", 2'-0" OR 3'-0") IS TO BE USED.
5. ATTACH FLANGE BRACE TO HOLE IN GIRT WHICH BEST FITS THE FLANGE BRACE LENGTH SUPPLIED FOR THE LOCATION.

[illegible]

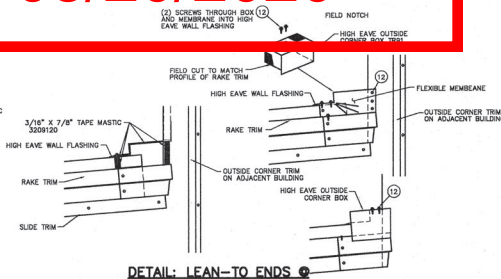


# RCRBD Record Set T.A.

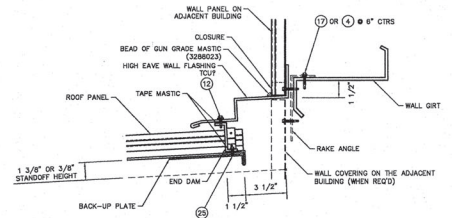
03/20/2020



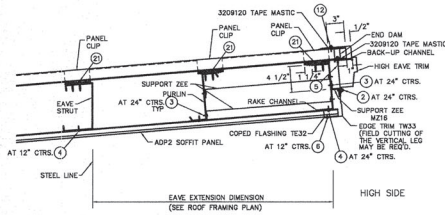
DETAIL: LEAN-TO STOP SHORT OF  
ADJACENT BUILDING CORNER



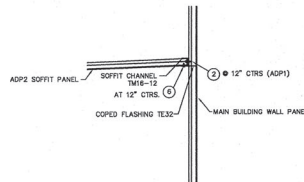
DETAIL: LEAN-TO ENDS  
ADJACENT BUILDING CORNER



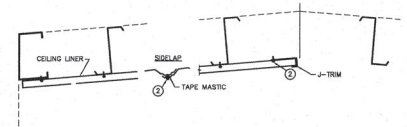
DETAIL: TRIM ALONG HIGH EAVE OF LEAN-TO-BUILDING



DETAIL: SIDEWALL EAVE EXTENSION AT CORNER  
OF HIGH EAVE OF LEAN-TO BUILDINGS



DETAIL: LEAN-TO SOFFIT PANEL TO MAIN BUILDING

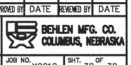


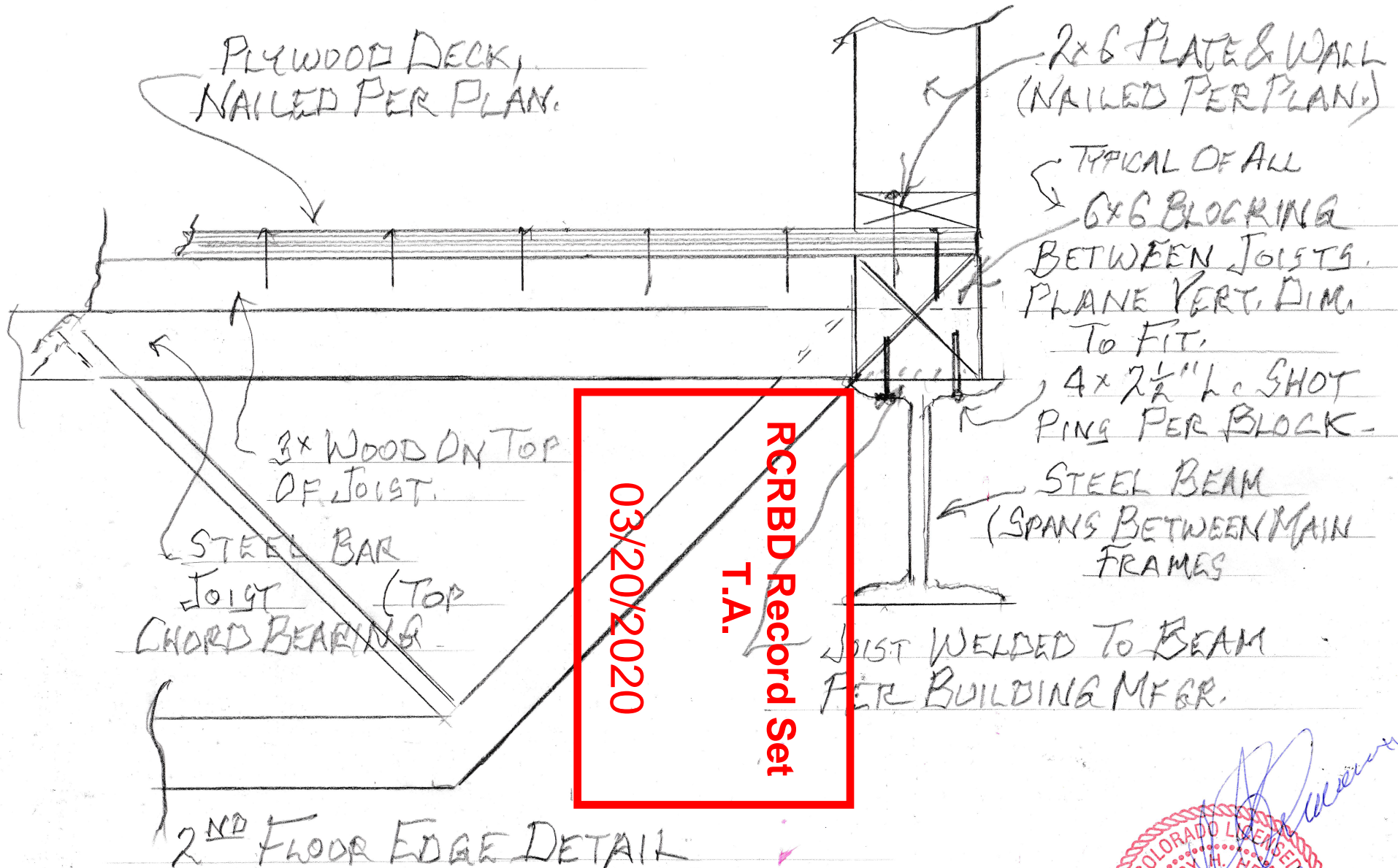
DETAIL: CEILING LINER

LOC. TAG NUMBER		FURNISHER SCHEDULE		ABBREVIATIONS
LOC.	TAG NUMBER	DESCRIPTION	DESCRIPTION	
11	1/2" x 1/			

<p>UNLESS OTHERWISE NOTED, CONNECTIONS BY THIS MANUFACTURER USING A-325 HIGH STRENGTH BOLTS ARE DESIGNED TO BE FASTENED USING THE "TIGHTENED METHOD, AS DEFINED AND SPECIFIED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, CONNECTION SPECIFICATION (SECTION 6-3-2000), SECTION 4.1 "TIGHT-TIGHTENED JOINTS" (REFERENCE FIGURE 6-3.1)</p>	
<p>NOTES:</p>	
<p>1. FOR SEQUENCE OF ERECTION - SEE APPLICABLE WALL PANEL ERECTION GUIDE.</p>	
<p>2. FOR FLANGE BRACE LOCATION - SEE FRAME CROSS SECTION AND ROOF FRAMING PLANS.</p>	<p>4. THE LENGTH OF THE FLANGE BRACE SUPPLIED AT EACH JOINT WILL DETERMINE WHICH STANDARD LAP HOLE LOCATION (1, 4, 8, 3 or 5) IS TO BE USED.</p>
<p>3. SOFT TIE BRACE AND FIELD CUTTING OF STEEL COMPONENTS MAY BE REQUIRED DURING THE ERECTION OF THIS BUILDING.</p>	<p>5. ATTACH FLANGE BRACE TO HOLE IN GIRT WHICH BEST FITS THE FLANGE BRACE LENGTH STANDARD FOR THE LOCATION.</p>

13	1/2"																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
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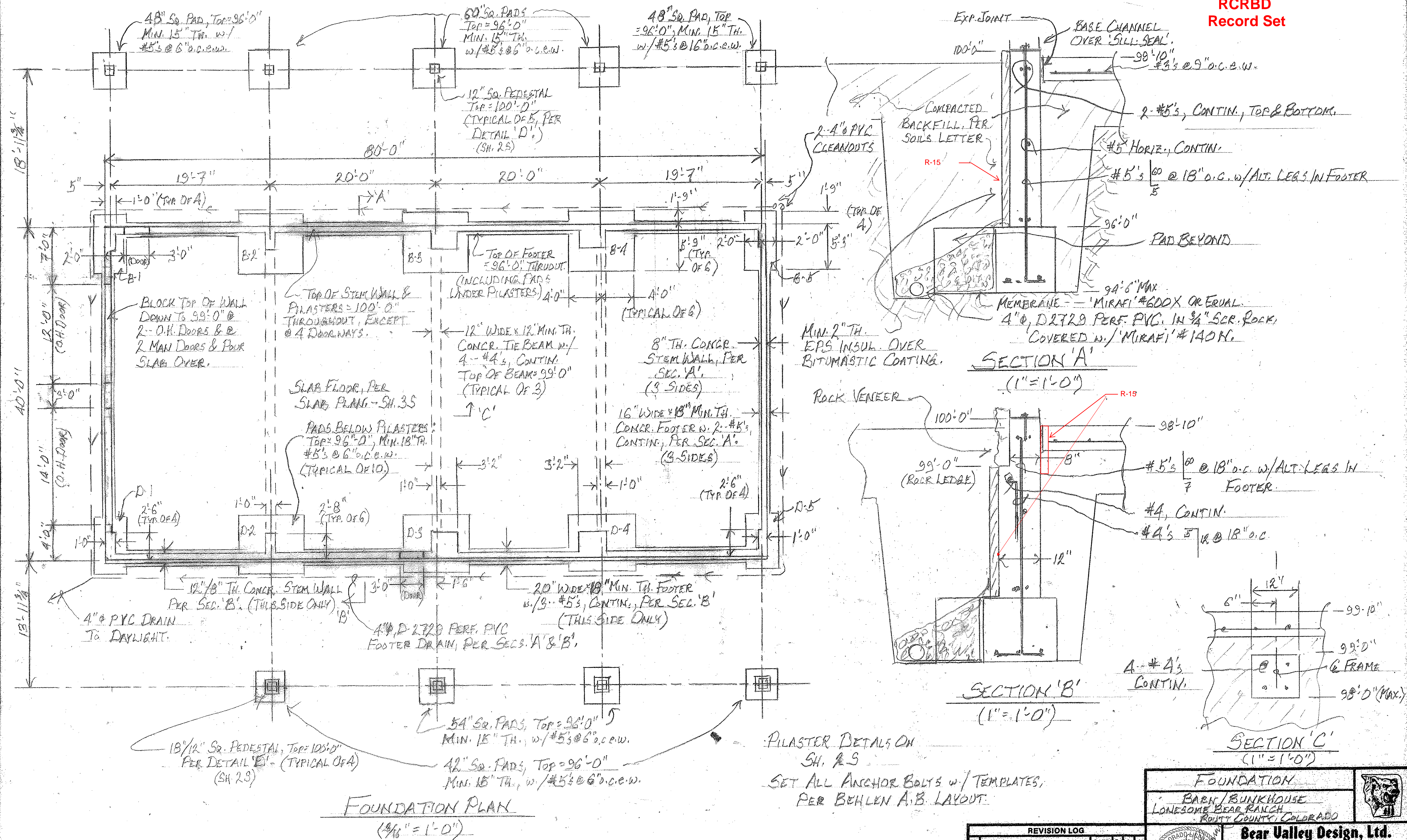
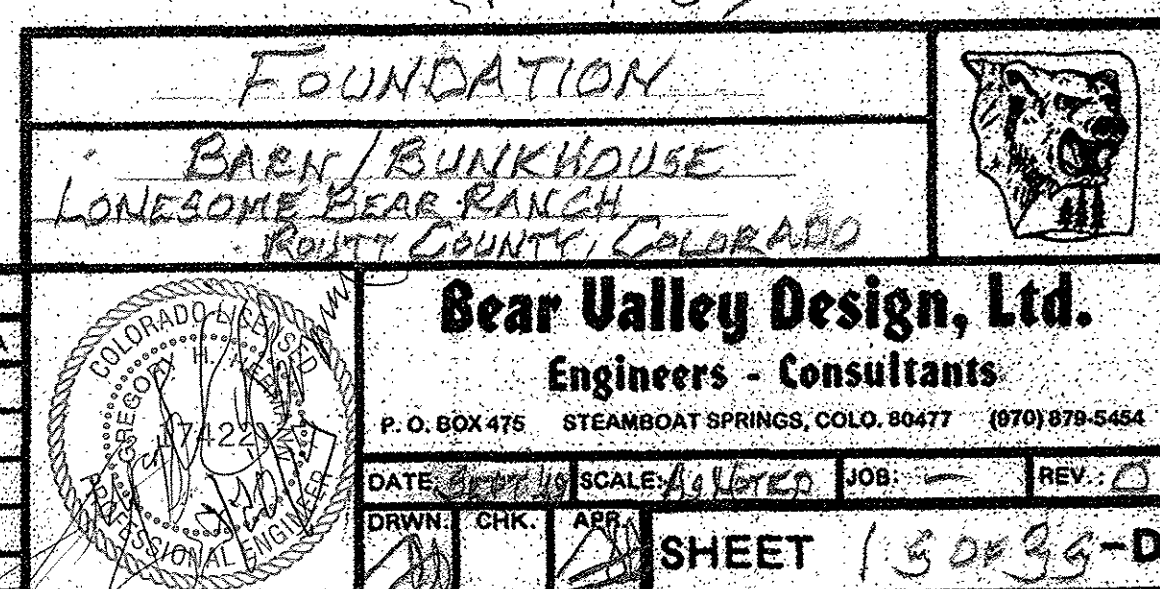


MORAN STEEL BUILDING  
LONESOME BEAR RANCH  
ROUTT COUNTY, CO.

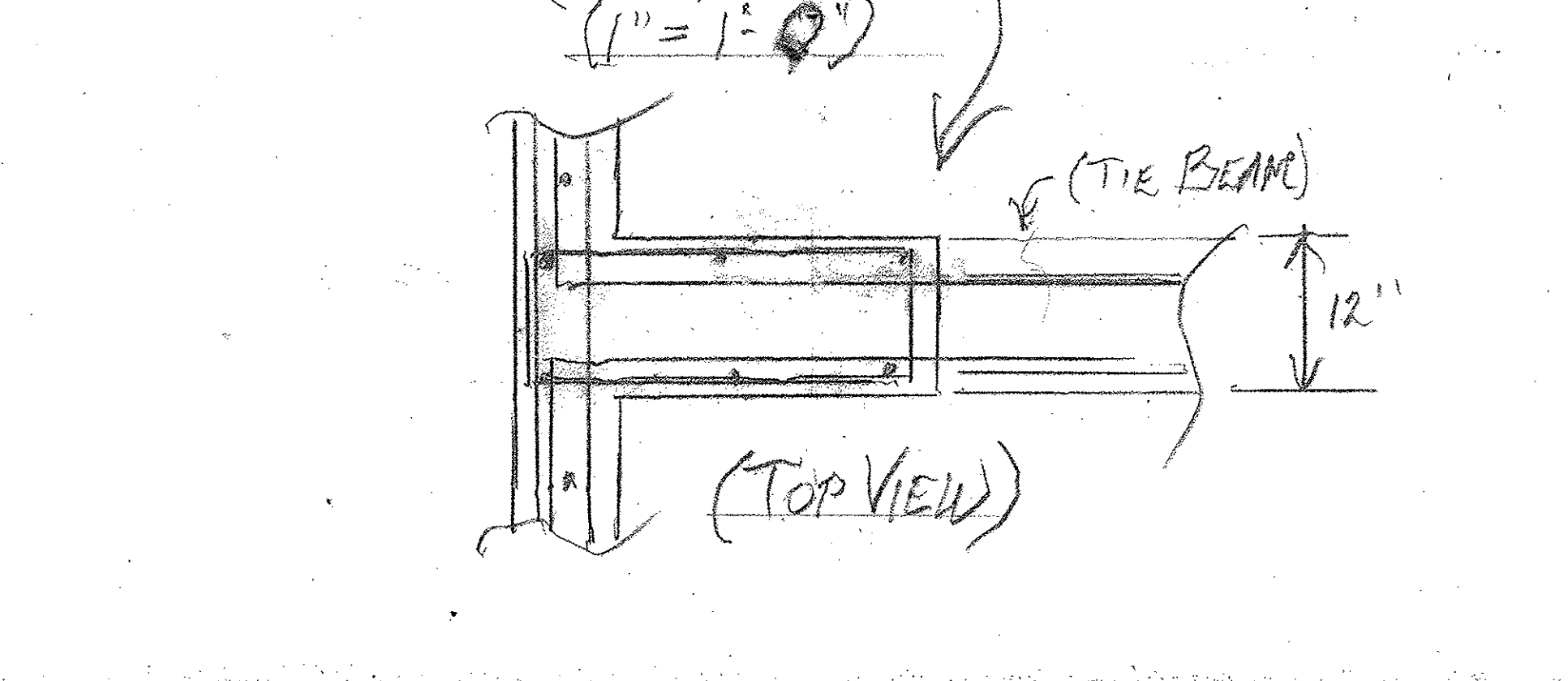
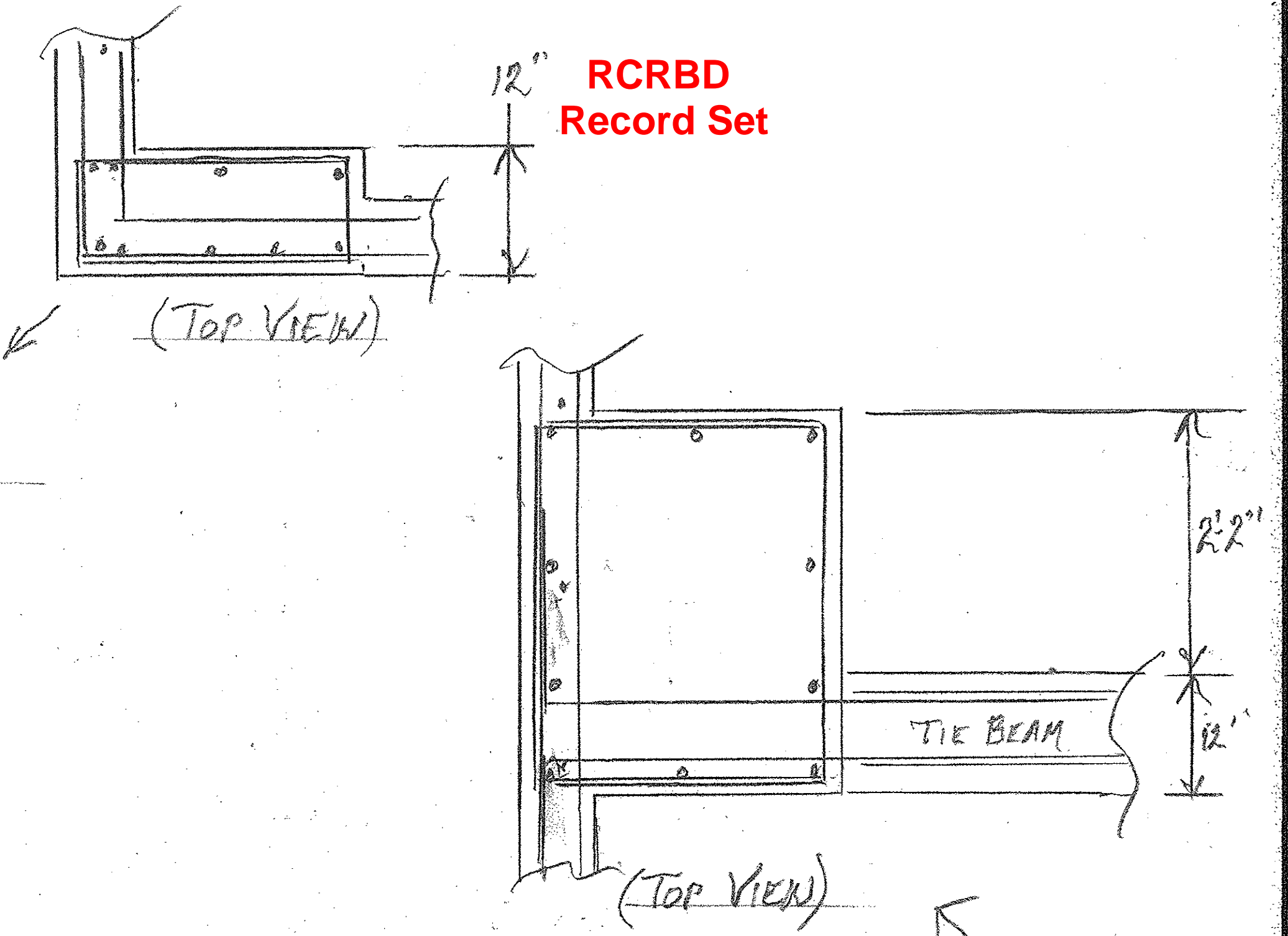
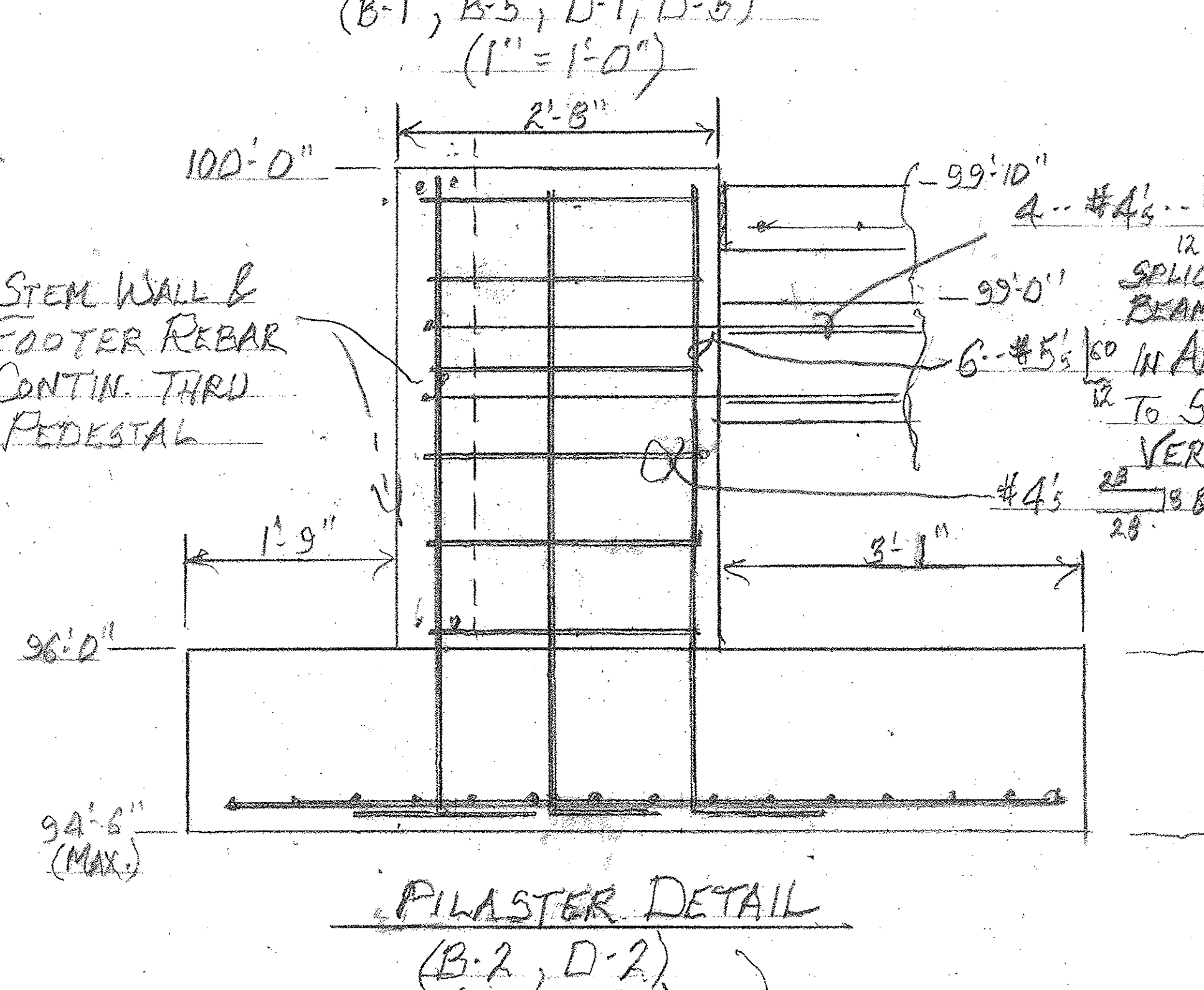
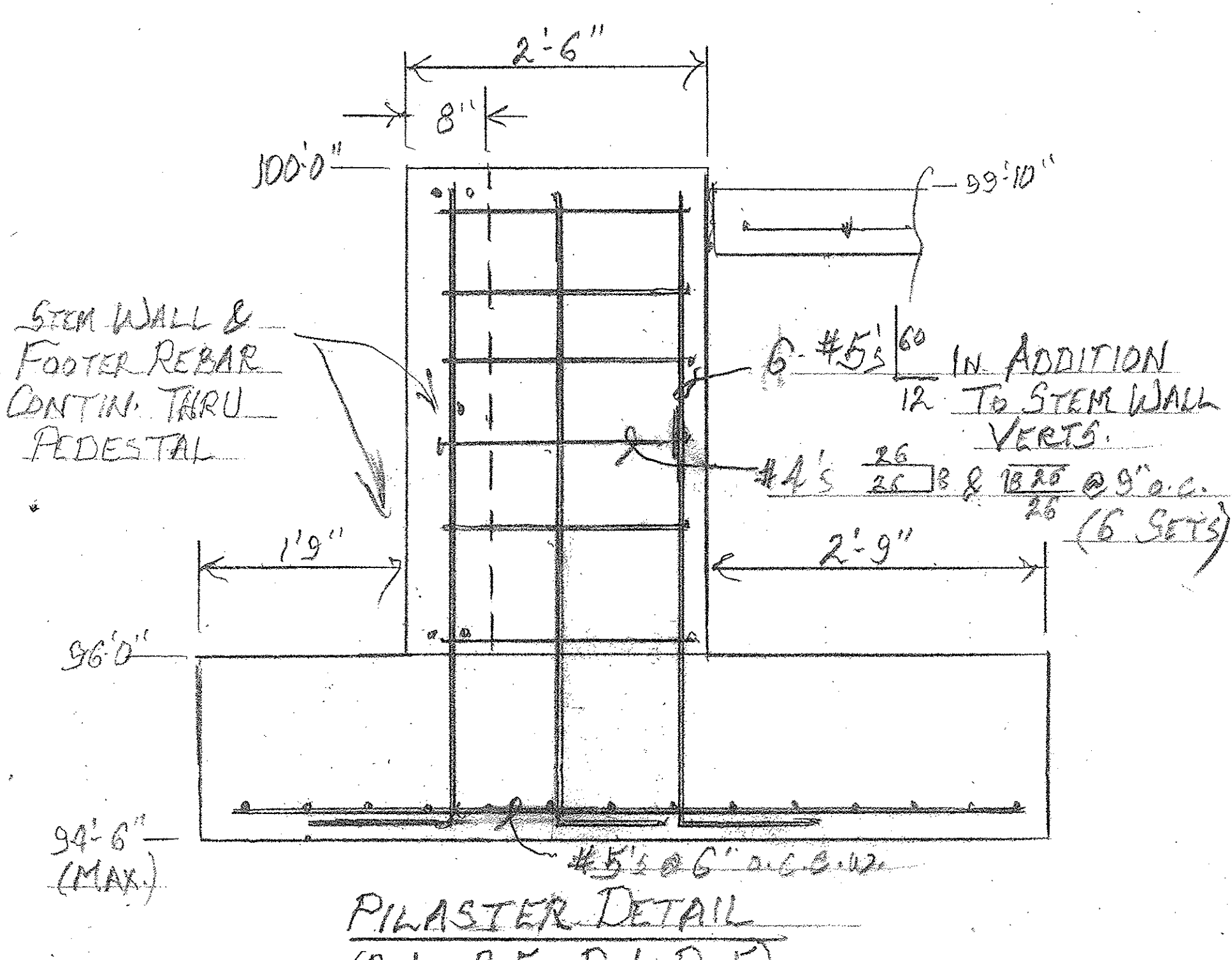
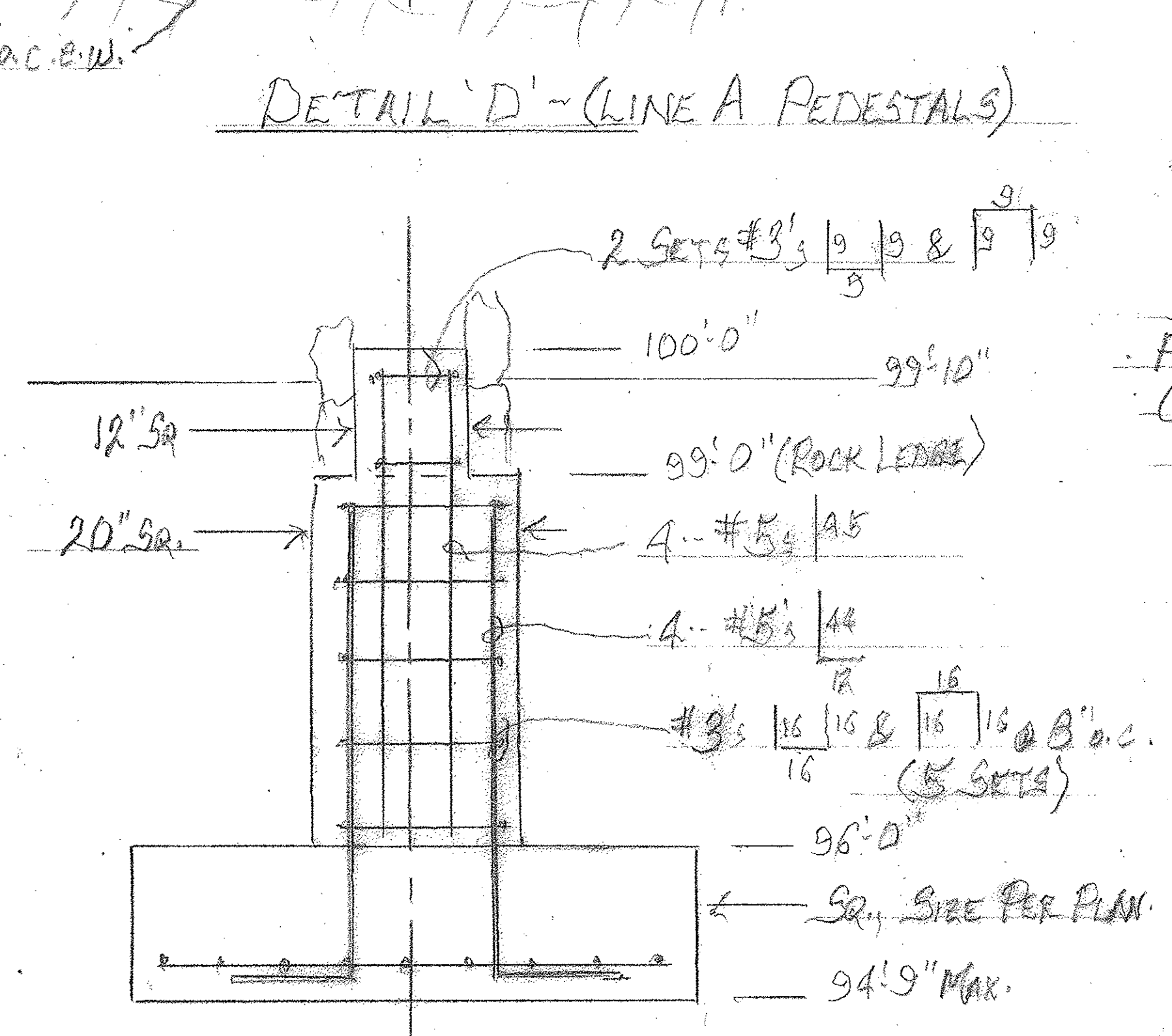
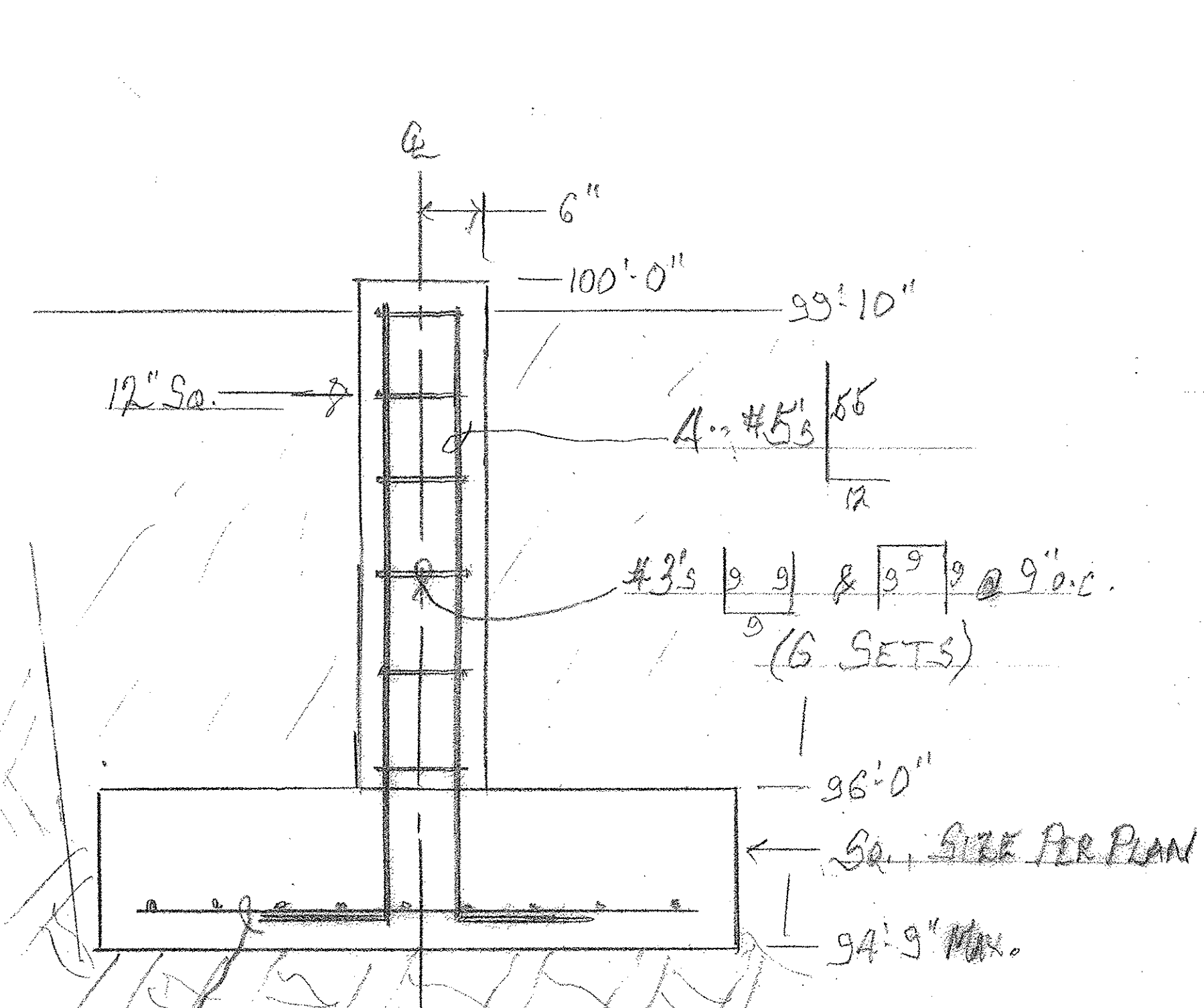
RCRBD  
PERMIT # TB-19-830  
3/19/20





[illegible]





RCRBD  
Record Set

(TOP VIEW)

(TOP VIEW)

2'-2"

12"

TIE BEAM

2'-8"

100'-0"

99'-10"

99'-0"

TIE BEAM

3'-1"

1'-9"

96'-0"

94'-6" (MAX.)

PILASTER DETAIL  
(B-3, B-4, D-3, D-4)  
(1" = 1'-0")

FOUNDATION  
BARN/BUNKHOUSE  
LONESOME BEAR RANCH  
ROBERT COUNTY, COLORADO

Bear Valley Design, Ltd.  
Engineers - Consultants  
P.O. BOX 475 STEAMBOAT SPRINGS, COLO. 80477 (970) 879-5454

DATE: 6/27/19 SCALE: 1" = 1'-0" JOB: REV: 0

DRAWN: CHK: APR: 20

SHEET 2.5 OF 36-D

REVISION LOG				
NO.	DESCRIPTION	DATE	D	A



RCRBD  
Record Set

Hi Ted--

Per our discussion this afternoon, Mike and I will be working with Scott Myller in order to address all of the issues you have raised with respect to occupancy separation and other 'upstairs' issues in the subject building.

Yes, the slab on the main floor will incorporate hydronic radiant heating, and insulation under the slab, between the 'Barrier' vapor barrier I specified on the drawing and the foam material (with buttons for locating the tubing) with have an R value of at least R5.

The anchor bolts will be as follows:

3/4" D., imbedded in the pilasters (5 locations on each side) will be 36" long with a 4" right angle hook at the bottom and 5" length of 3/4" D. UNC thread at the top.

3/4" D. imbedded in the end walls for end wall columns will be 18" long with a 3" right angle hook at the bottom and 5" length of 3/4" D. UNC thread at the top.

All 1/2" D. bolts for miscellaneous door frame attachments may be typically available commercial drill in type cinch anchors.

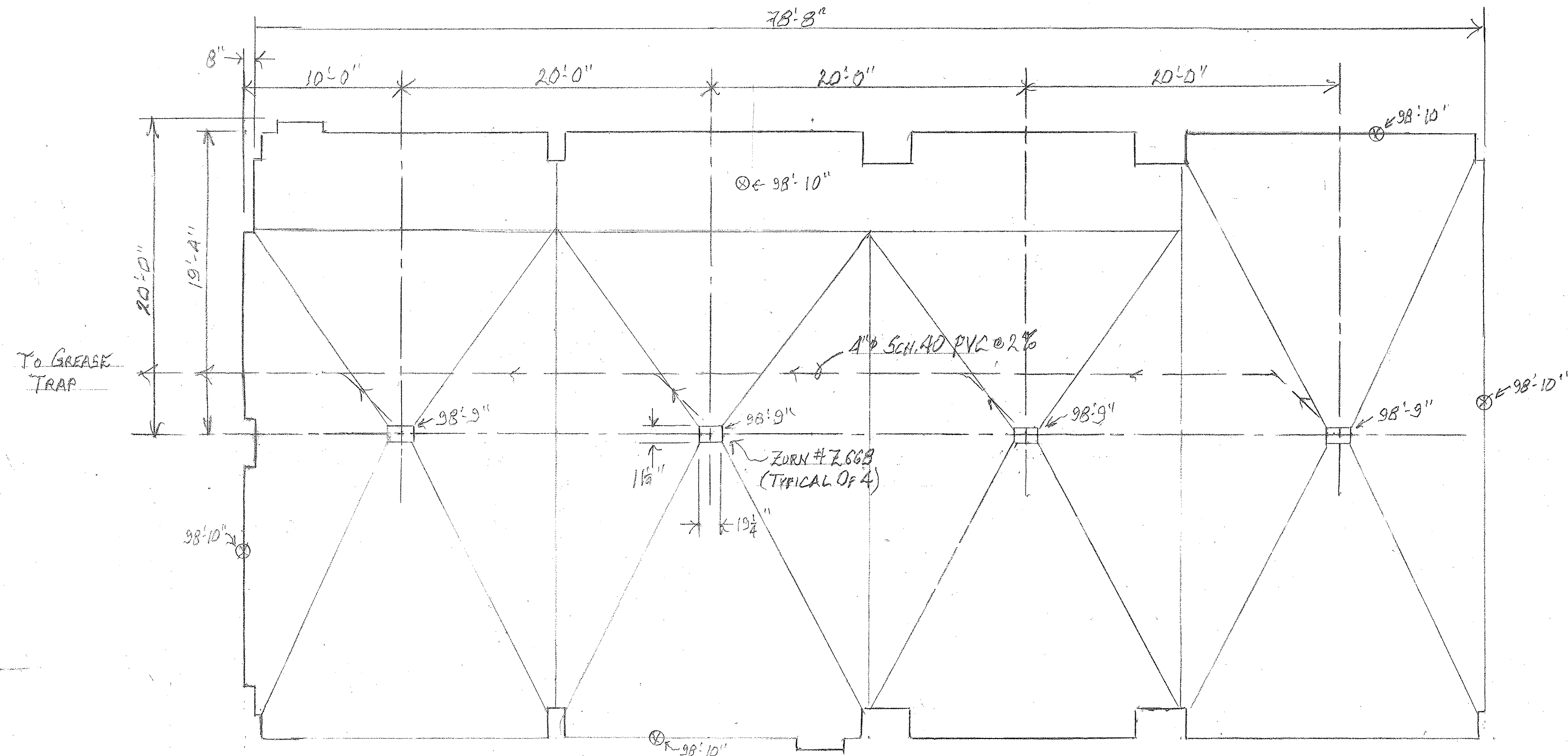
All 3/4" bolt shall be either of A-36 steel or meet the building manufacturer's noted 'F1554' spec, whichever is more stringent.

At the present time, Mike and I are discussing alternative methods for dealing with the insulation gap you pointed out where the rock veneer (which is not extend more than 4' above grade) extends below grade. We will have this detail worked out before it is time to pour the floor slab.

Mike is planning to come by your office today (Friday 10/4) and I hope that this info is sufficient so that you will be able to issue the foundation permit for the building when he arrives there.

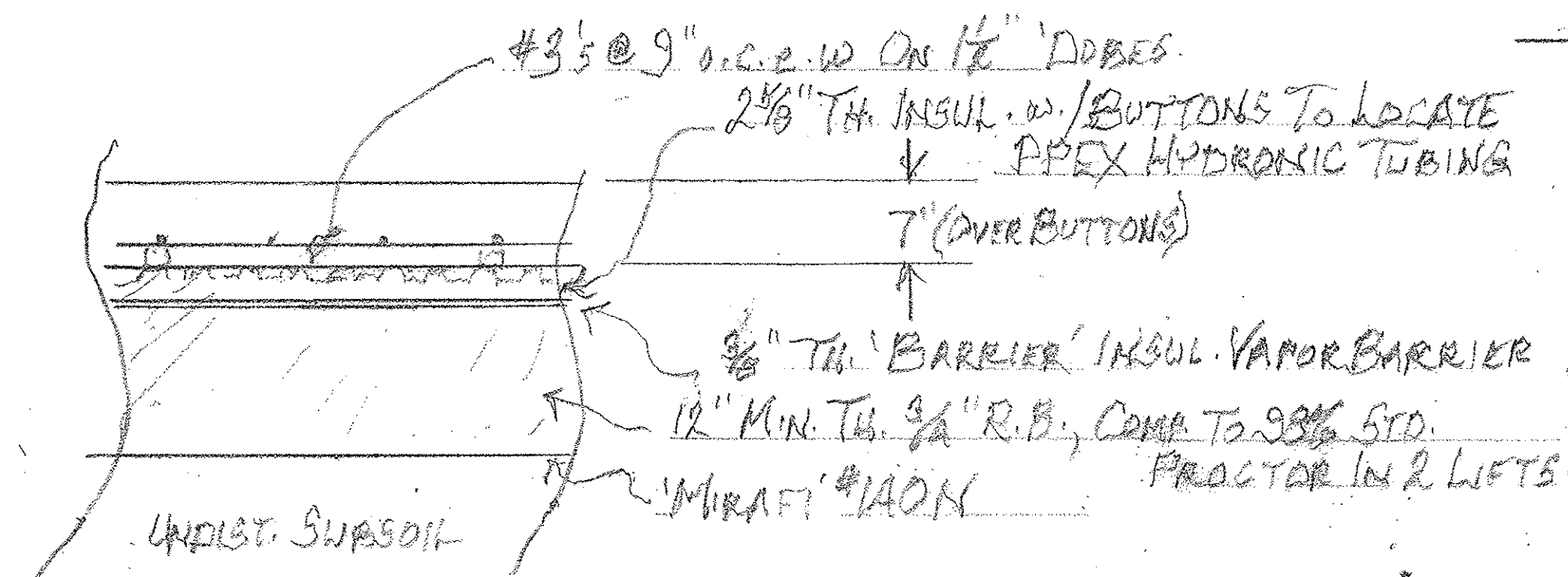
Thanks, and best regards,

Greg



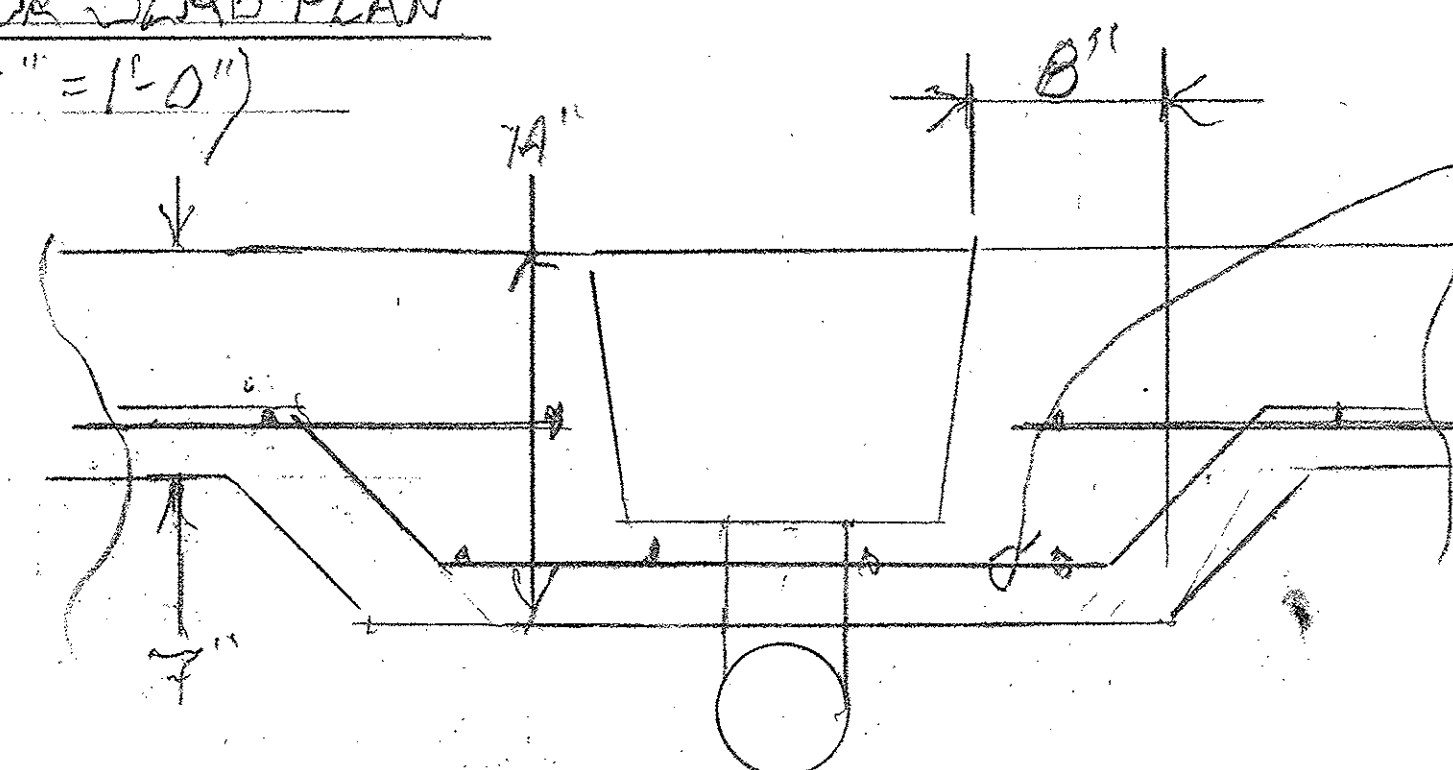
FLOOR SLAB PLAN

(1/2" = 1'-0")



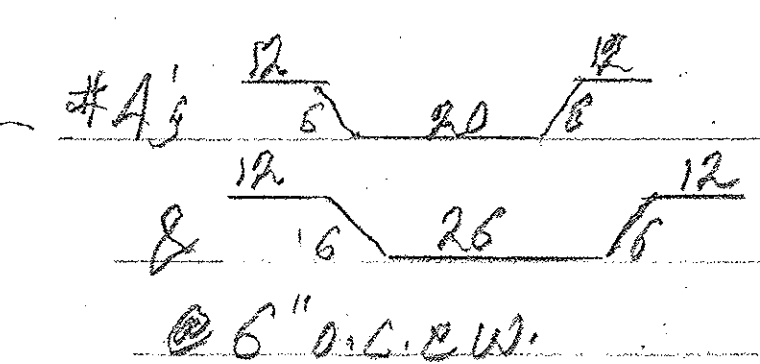
TYPICAL SLAB SECTION

(1" = 1'-0")



SLAB DETAIL @ FLOOR DRAINS

(1/2" = 1'-0")



REVISION LOG									
NO.	DESCRIPTION	DATE	D	C	A				

SLAB PLAN & DETAILS

BARN & BUNKHOUSE

LONGHORN BEAR RANCH

ROBERT COUNTY, COLORADO

**Bear Valley Design, Ltd.**

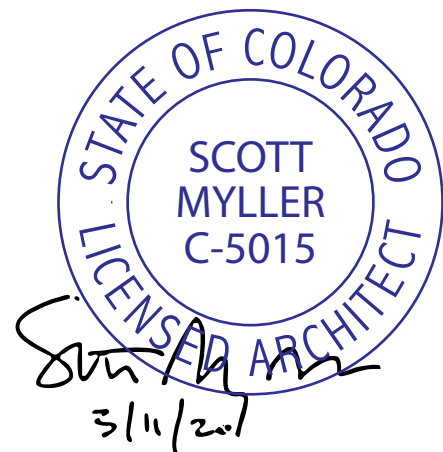
Engineers - Consultants

P.O. BOX 475 STEAMBOAT SPRINGS, COLO. 80477 (970) 879-5454

DATE: 10/1/10 SCALE: AS NOTED JOB: REV: 0

DRWN: CHK: APP: SHEET 38 OF 38-D



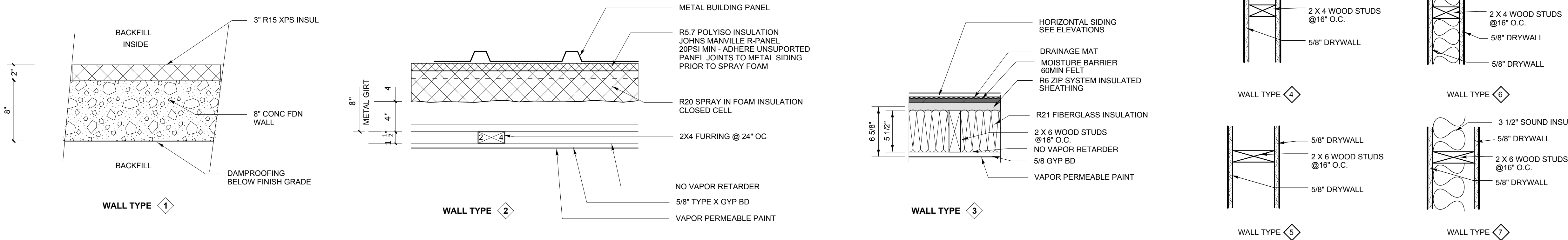


Architect  
**scott myller. architect**  
652 Ruby Square  
P. O. Box 771854  
Steamboat Springs, CO 80477  
970.846.1700

Structural Engineer

RCRBD Record Set  
T.A.

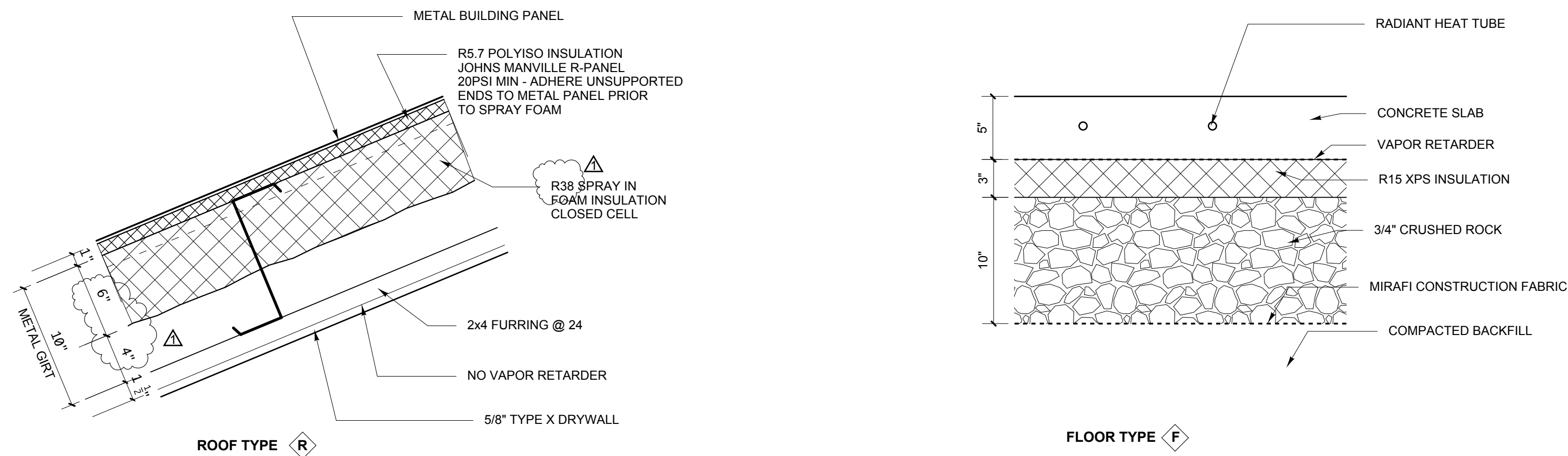
03/20/2020



## 1 WALL TYPES

A3.2

1 1/2" =1'- 0"



## 2 ROOF TYPES

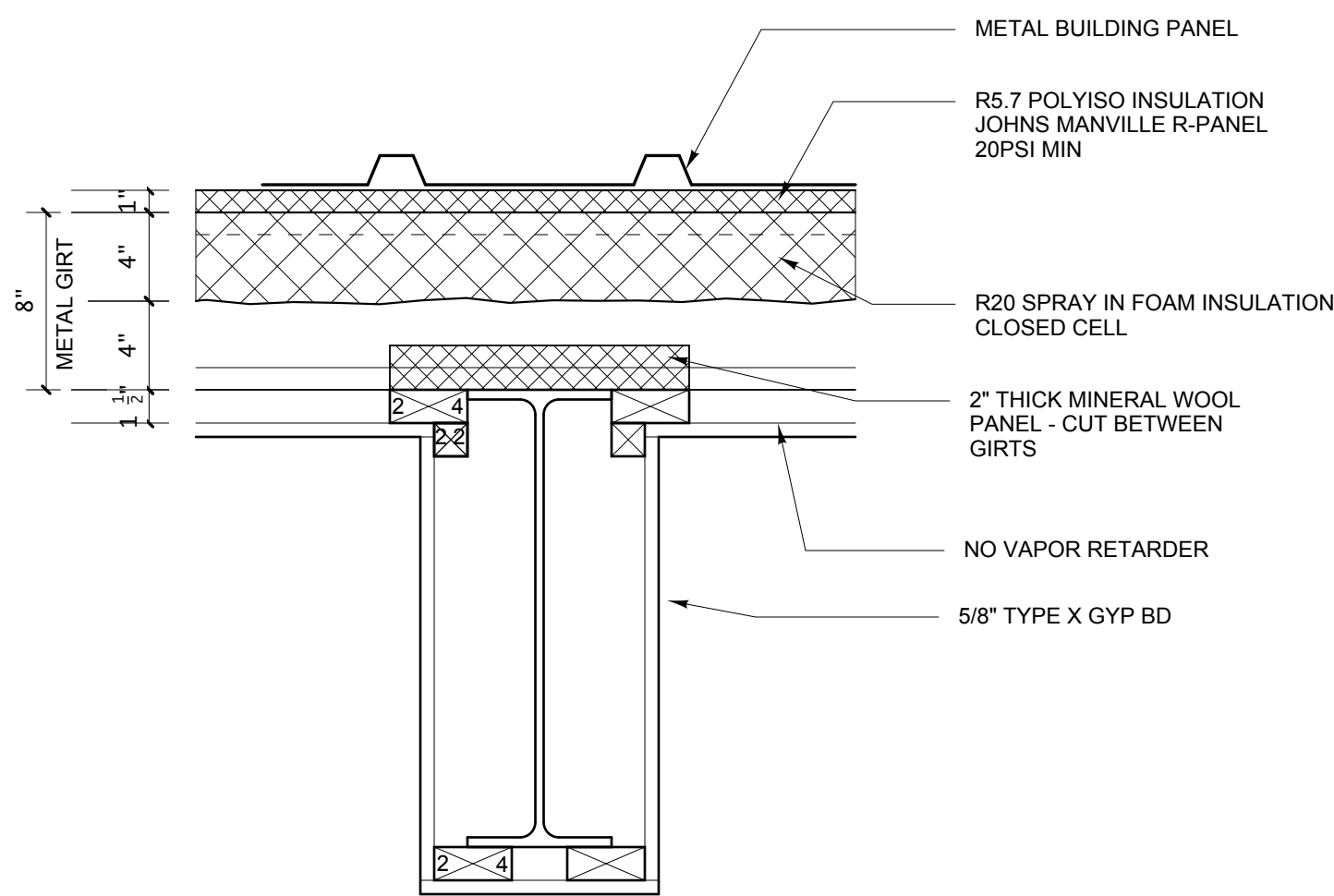
A3.2

1 1/2" =1'- 0"

## 3 FLOOR TYPES

A3.2

1 1/2" =1'- 0"



## 4 COLUMN FIRE PROTECTION

A3.2

1 1/2" =1'- 0"

MORAN  
RESIDENCE

30857 COUNTY RD 53  
HAYDEN  
COLORADO

### REVISIONS:

REV. DATE:	REV. NAME:	REV. NO.:
------------	------------	-----------

3/11/20	INSULATION R-VALUE	Δ
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### NOTES:

WALL TYPES  
FLOOR TYPES  
ROOF TYPE

date: 9 MAR 2020  
scale: 1/4" = 1' - 0"

A3.2



03/20/2020

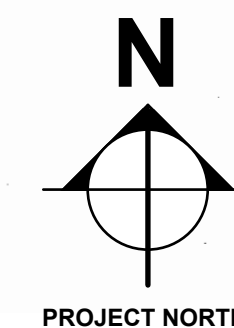
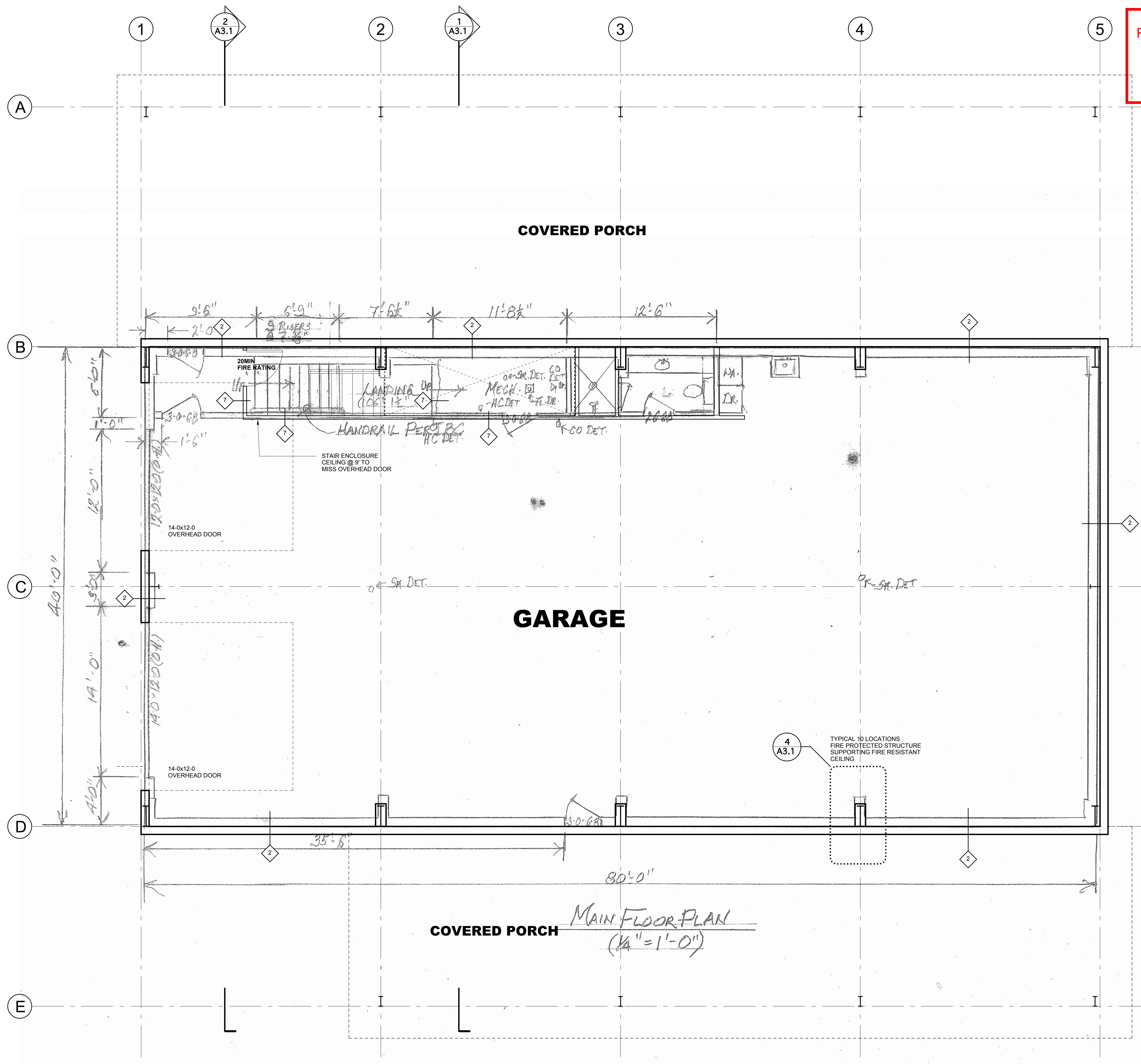
**CLIMATE ZONE 7**  
All Windows and Doors shall carry a U-Value of 0.32 min.

30857 COUNTY RD 53  
HAYDEN  
COLORADO

## REV. DATE:                      REV. NAME:                      REV. NO:

date: 9 MAR 2020  
scale: 1/4" = 1' - 0"

## A2.1





03/20/2020

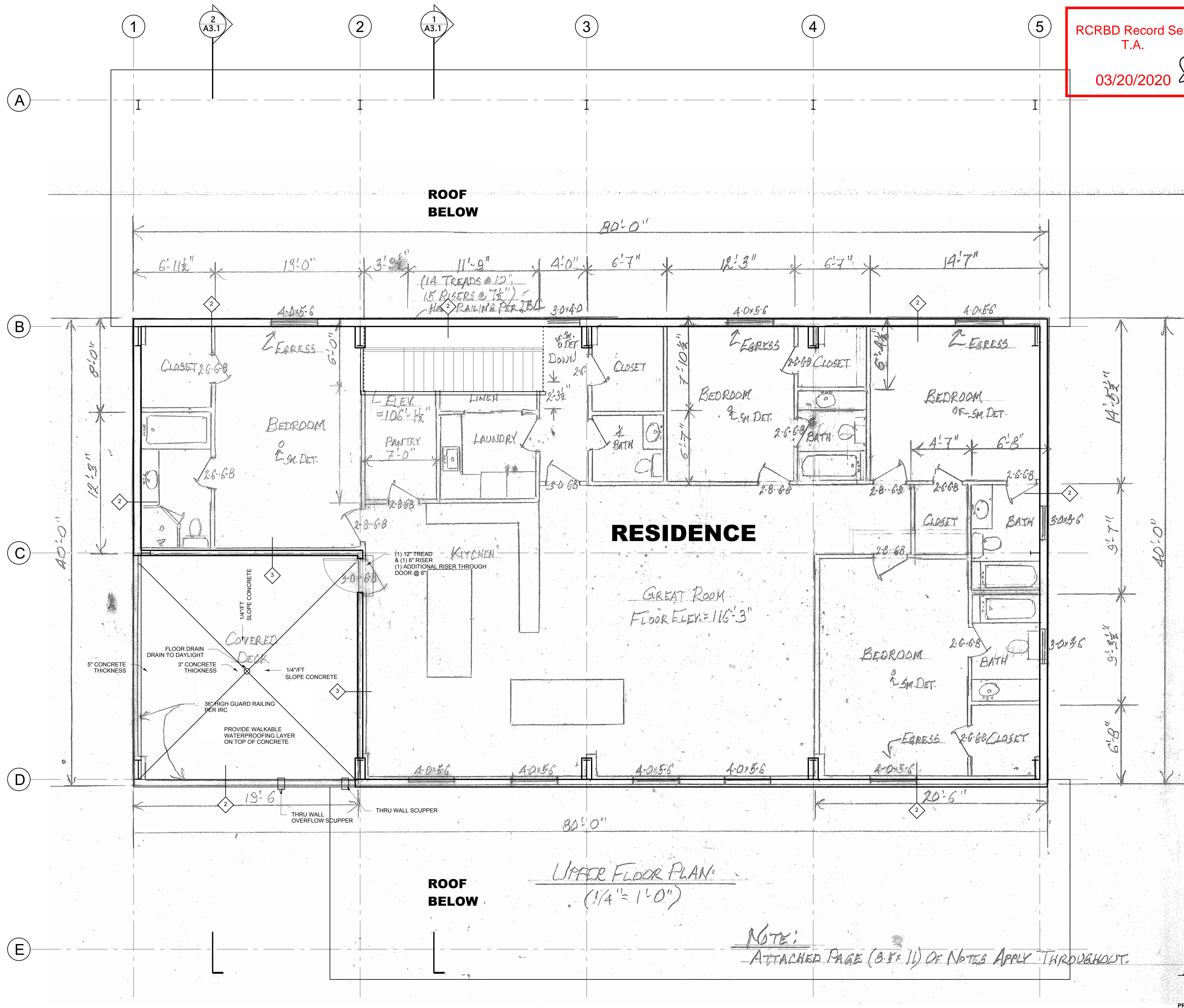
3/9/27

30857 COUNTY RD 53  
HAYDEN  
COLORADO

NOTES:

date: 9 MAR 2020  
scale: 1/4" = 1' - 0"

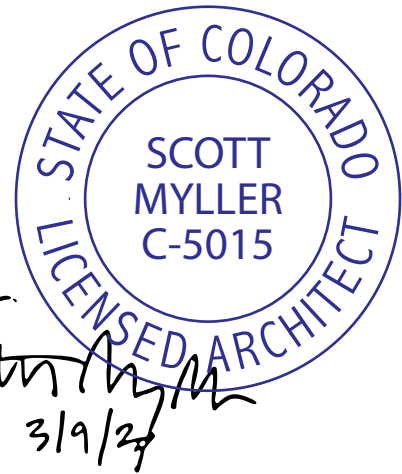
## A2.2





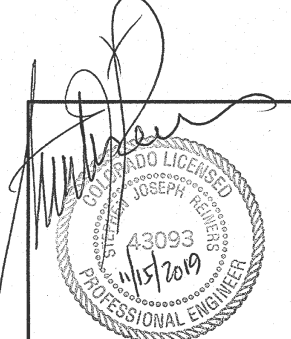
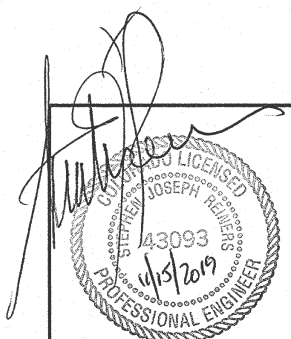
652 Ruby Square  
P. O. Box 771854  
Steamboat Springs, CO 80477

## Structural Engineer



03/20/2020

3/9/24



30857 COUNTY RD 53  
HAYDEN  
COLORADO

[illegible]

### A3.1

**1 1/2" = 1'- 0"**

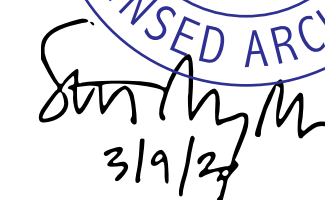
### A3.1

**1 1/2" = 1'- 0"**

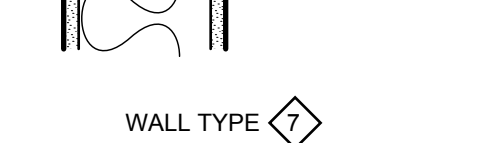
date: 9 MAR 2020  
scale: 1/4" = 1' - 0"

# A3.1





Structural Engineer



### A3.2

**1 1/2" = 1'- 0"**



### A3.2

1 1/2" = 1'- 0"



**A3.2**

**1 1/2" = 1' - 0"**



### A3.2

**1 1/2" = 1' - 0"**

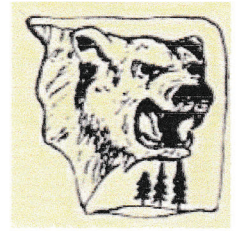
03/20/2020 8:42:51 AM

scale: 1/4" = 1' - 0"

## A3.2

# Bear Valley Design, Ltd.

## Engineers - Consultants



P. O. Box #770475  
STEAMBOAT SPRINGS, COLORADO, 80477-0475  
MOBILE: (970) 879-5454  
E-MAIL: <bearbyd@mindspring.com>

June 27, 2019

**RCRBD**  
**Record Set**

Mr. Carrol Moran  
PO Box 209  
Chatfield, Texas, 75105

Subject: Soil investigation and LTAR evaluation for a two buildings and an Onsite Wastewater Treatment System (OWTS) on a 100.94 acre tract of land on the Lonesome Bear Ranch in T5N R868W, in Routt County, Colorado.

Dear Mr. Moran,

Per your request, we performed a soil investigation and evaluation on the subject site earlier in June of this year. The investigation was performed for the purpose of providing soil design parameters for the foundations for a steel building and for a residence, and the evaluation was performed the purpose of designing an Onsite Wastewater Treatment System (OWTS) for use by the steel building (which will include a bunkhouse).

The proposed steel structure is anticipated to be of typical, single story, red iron steel framed construction, including a slab on grade main floor and a bunkhouse upper floor, all to be founded on reinforced concrete stem walls which bear upon reinforced concrete spread footers. The proposed residence is anticipated to be of typical wood framed construction, with a slab on grade lower (walk-out) lower floor, a main floor and an upper floor. The building site is located on top of a hogback with slopes downward to the north, aouth, and east, and a slope upward to the west. The vegetation on the site consists of grass and oak brush. Although the building site is nearly level, the slopes downward to the south and east are relatively steep, and the slope upward to the west is fairly steep.



Three test pits and a profile hole (four pits total) were advanced on the lot, three in the relatively flat area where the buildings are expected to be situated. The profile hole was advanced somewhat downslope to the north of the proposed building pad, in the logical location for the OWTS absorption field. The pits were advanced using a crawler mounted excavator.

All three test pits revealed 12 to 24 inches of very slightly moist, medium brown sandy, slightly silty loam, topsoil overlying a native, moderately dense, very slightly moist medium sand subsoil which extended to the maximum depth explored of eight feet.

The fourth test pit (the profile hole) revealed approximately 30 inches of similar, moderately moist topsoil overlying subsoils similar to those exposed in the first three test pits, but less dense, and containing small amounts of silt and clay. The profile hole was advanced to a depth of nine feet, and revealed no signs of free ground water and no bedrock.

No bedrock or free water was encountered in any of the four test pits.

Our experience with similar soils, taken together with our observations in the test pits, have led us to form the opinion that the moderately dense sand subsoil observed in the first three test pits will provide stable bearing for the foundations of both of the proposed structures. We also concluded that the site and the subsoil observed in the profile hole are, in fact, suitable for the installation of an OWTS with a leach field of the type of design detailed below.

Spread footers for both of the proposed structures should be designed to bear on the moderately dense sand subsoil observed in the first three test pits, with a maximum net bearing pressure of 2.0 KSF. No minimum dead load will be necessary on any of the footers. Any retaining structures should be designed to retain pressure equivalent to that which would be exerted by a fluid weighing 40 PCF.

The footers for both foundations must be surrounded with a footer drain constructed using 4" diameter D-2729 perforated PVC pipe (with the perforations located at 4 and 8 'o'clock'), bedded and covered with 3/4" screened rock, which in turn must be wrapped in a geo-fabric such as 'Mirafi' #140N. Both footer drains must run from a pair of clean-outs, have a minimum 1% slope around the foundation to a corner opposite the clean-outs, and at that point be welded together to drain to daylight via a non-perforated 4" diameter PVC pipe. These drains must be located at a low enough grade so that it will prevent



water which might penetrate the backfill from soaking the bearing soil beneath the footers. The daylighted end of these drains should be protected from intrusion by critters by means of a screen and cobbles.

Frost protection for the foundations must be provided by maintaining a minimum of 48" of earth cover over them, measured in any direction. The finish grade should provide for a minimum of 2% slope away from the structures in all directions for a minimum of 10 feet, as well as for positive and continuous drainage away from the buildings without any ponding. Native subsoil materials will provide appropriate backfill. It is anticipated that a large portion of both buildings' perimeters will be surrounded with a graveled driving surface. Backfill not situated beneath a graveled driving surface may be capped with a maximum six inch thick layer of topsoil. The native backfill material must be placed in lifts a maximum of 10 inches thick, with each lift moistened and compacted to 93% of its Standard Proctor density.

In order to control moisture as well as to minimize heating costs for the proposed building, as well as to provide for proper curing of the concrete, all slab on grade floors must be placed directly on top of a minimum six mil thick sheet of visquene. The slabs on grade must be isolated from the subgrade by a minimum twelve inch thick layer of compacted  $\frac{3}{4}$ " road base gravel. This gravel fill must be isolated from the underlying material by means of a sheet of 'Mirafi' #140N (or equal). It is anticipated that the slab on grade floors will be provided with hydronic, in floor heating. In this case, underslab insulation per energy code requirements must be provided. We highly recommend the use of foam insulation provided with 'buttons' for positively locating the (O2 barrier type) Ppex tubing, and providing a layer of 'Barrier' insulating vapor barrier beneath the foam insulation.

All structural elements of the building must be isolated so that the slab on grade floors are free to float with respect to the rest of the buildings. All partitions located directly above any slab on grade floors must be constructed with a minimum 1- $\frac{1}{2}$  inch high expansion joint, built per typical local practice, at the bottom of the framing of said partitions.

The native slightly clayey sand subsoil encountered in the profile hole classifies as a Type 3 soil per CDOPH& E Regulation #43. Therefore, absorption trenches for the proposed OWTs should be designed based on a Long Term Acceptance Rate (LTAR) of 0.35 gallons per square foot per day.

We re-emphasize that no free ground water was observed in either the profile hole or in the other test pits, and the observed subsoil in the profile hole extended more than four feet below the expected design elevation of the bottom of the proposed absorption trenches without encountering any free ground water.

Thank you for the opportunity to have been of professional service to you in this matter.

Sincerely,

Bear Valley Design, Ltd.

Gregory H. Hermann  
Colorado P.E. #17422

**Structural Notes--Lonesome Bear Ranch, Barn/Bunkhouse, Routt County,**  
**Colorado**

1. All concrete shall contain six 90 pound sacks of Type II cement per cubic yard,  $\frac{3}{4}$  inch maximum size aggregate, 2% to 4% entrained air, and shall be placed in full accordance with all provisions of the current version of ACI-318.
2. All reinforcing steel shall conform to ASTM A-615, Grade 60
3. Design slab on grade floor load is 250 lbs./sq. ft., live;
4. Design nominal snow load is 80 lbs./sq. ft.
5. Design wind load is 90 mph, per IRC 'Exposure B' requirements at a density altitude of 7,200 ft., mean sea level.
6. Design earthquake is per IRC 'Zone B' requirements.
7. Design soil conditions are 2.0 KiP/sq. ft., maximum net bearing and 0.0 KiP/sq. ft., minimum dead load, per Bear Valley Design, Ltd. Letter dated June 27, 2019.
8. All details enumerated in the letter referenced in 7. Must be executed in full.
9. The steel ('red iron') frame building above the foundation is to be designed, engineered, certified, and fabricated by others. Installation of the building and anchor bolts in the foundation is to be per the building mfgr's. sizing and layout dimensions and specifications
10. Heating plans to be provided on a design/build basis by mechanical contractor.
11. Electrical plans to be provided on a design/build basis by electrical contractor.
12. Plumbing plans to be provided on a design/build basis by plumbing contractor.
13. All above grade, enclosed portions of the building are to be insulated using spray-on, 2 part urethane foam insulation, with minimum R values of R-50 in the roof and R-30 in the exterior walls.

A handwritten signature in black ink is written over a red circular professional engineer seal. The seal contains the text "COLORADO", "GREGORY H. KLINE", "17422", and "PROFESSIONAL ENGINEER".



100.94 Acre Parcel on Lonesome Bear Ranch, Routt County, OWTS Calculations

Proposed Barn and Bunkhouse:

4 Bedrooms = 7 persons @ 75 Gallons/ day/ person = 525 Gal./day design flow

Minimum tank size = 1,250 Gallons, with 2 chambers and an approved effluent filter plus a 500 Gallon Dosing Tank with a Fluid Dynamics, Inc., #216 Auto-siphon, giving average 30 gpm flow for an approx. 250 gallon dose.

Leach Field Sizing : 'Infiltrator' Quick 4 chambers in center fed trenches.

Long Term Acceptance Rate (LTAR) = 0.35 Gal./ sq. ft. / day into Type 3 Soil, sandy clay loam (per Bear Valley Design, Ltd., letter, dated 6/27/19)

$(525 \text{ Gal./day}) \times (.7 \text{ (reduction factor for use of chambers)}) \times (.9 \text{ (reduction factor for dosing)}) / (0.35 \text{ Gal./day/ sq. ft.}) = \underline{945 \text{ sq. ft. (required absorption area)}}$

System will be designed to use 'Infiltrator' 'Quick 4' chambers. Said chambers provide an effective absorption area 48" x 34".  $(48 \times 34)/144 = \underline{11.33 \text{ sq. ft./chamber}}$

$945 \text{ sq. ft.} / (11.33 \text{ sq. ft./ chamber}) = \underline{84 \text{ chambers required (4 rows of 21), center fed}}$



**scott myller. architect**

652 Ruby Square  
Steamboat Springs, CO 80487

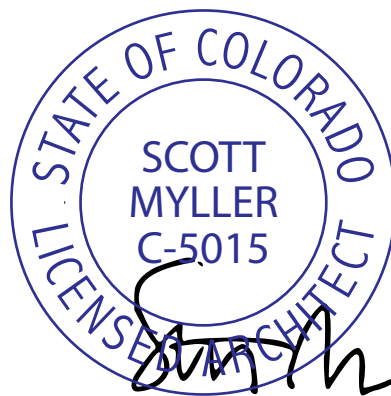
Routt County Regional Building Department  
136 Sixth Street  
P.O. Box 773840  
Steamboat Springs, CO 80477

March 1, 2020

RE: TB-19-830

The following list is a direct response to the items listed on the Plan Review Comments requiring corrections:

1. Noted.
2. While the name of the project is called a bunkhouse, this is the only building on the site and is intended to be used by the owner and his family and friends. No "guest ranch" uses are proposed. No "agricultural" uses are intended either and no hazardous storage is proposed under the covered porches. The 2015 IRC shall be used for review with a 3487sf residence and a 2913sf garage.
3. A propane fired boiler is proposed to supply radiant heat to the garage slab and for staple up tubing below the residence floor.
4. See attached details for insulation values.
5. See attached wall type details and locations.
6. The metal building shop drawings were provided prior to foundation permit.
7. The Structural Engineer of record (Bear Valley Designs) shall provide special inspections for all high strength bolts – if any.



3/9/20

**RCRBD Record Set  
T.A.**

**03/20/2020**

**970.846.1700**

www.ythatguy.com  
myller@ythatguy.com



September 12, 2019

## LETTER OF DESIGN CERTIFICATION

Reference Number : X2212  
 Building Description : 1 - 40'-0" x 80'-0" x 27'-0"  
 Building Owner/Location : MORAN, HAYDEN, CO  
 Builder : Charchalis Construction and In

This document shall serve to certify that the above referenced building has been designed by this IAS AC472 accredited manufacturer in accordance with the order documents and information shown below:

Design Standard : 2015 IBC

### GRAVITY LOAD DATA

Roof Live Load (psf) : 20.00 \*  
 Uniform Roof Snow Load (psf) : 97.30  
 Snow Importance Factor : 1.00  
 Rain on Snow (psf) : 0.00  
 Pg (psf) : 139.00  
 Pf (psf) : 97.3  
 Ce : 1.00  
 Ct : 1.00  
 Collateral Load (psf) : 3.0

### EARTHQUAKE LOAD DATA

Site Class : D  
 Sds (%g) : 27.1  
 Sd1 (%g) : 7.5  
 Seismic Design Category : B  
 Seismic Importance Factor : 1.00  
 R : 3.00  
 Cs : I x Sds / R  
 Basic Structural System : NDFS  
 Analysis Procedure : Equivalent Lateral Force

### WIND LOAD DATA

Basic Wind Speed (mph) : 115  
 Wind Exposure : C  
 Wind Importance : 1.00  
 GCpi : ± 0.18

\* Roof Live Load is Non-Reducible

Risk Category : II

Steel members are designed in general accordance with the 14th Edition of the AISC Manual for Steel Construction and the 2012 Edition of the AISI Cold Form Steel Design Manual.

This certification is strictly limited to the design of structural components designed and manufactured by Behlen Mfg. Co. for the loads and standards shown. Certification does not extend to foundation, mechanical, electrical, plumbing, fire protection, civil work, architectural responsibilities, overall project coordination, erection supervision or inspection, or other aspects of code or specification compliance not so indicated. When properly erected, according to the Behlen plans, on an adequate foundation, this Behlen building has been designed to safely sustain these loads.

BEHLEN BUILDING SYSTEMS



**RCRBD Record Set  
T.A.**

**03/20/2020**

