

ENVIRONMENTAL CONSULTING

WATER RESOURCE ENGINEERING

August 3, 2022

Routt County Environmental Health Department PO Box 770087 Steamboat Springs, CO 80477

RE: 34130 Whiffle Tree Trail Oak Creek - Routt County, Design Report and Site Plan, Onsite Wastewater Treatment System

Dear RCEHD:

On behalf of the owners Chris Reed & Keelin Regan-Reed, herein included are this design letter and permit drawings for the proposed onsite wastewater treatment system (OWTS). The proposed system is designed to treat and dispose of domestic wastewater from a proposed three-bathroom home. The proposed system (system) has been designed in accordance Routt County Regulations (RCR), which are essentially the Colorado Department of Public Health and Environment Water Quality Control Commission (CDPHE/WQCC) Regulations 43. Note that the design calculations reference Regulation 43. The drainfield will be located north of the home. There is no surface water within 1,000 feet of the proposed drainfield location.

Please note that this design replaces recommendations from Bear Valley Design. The only information we are using from Bear Valley is the confirmation from a January 29, 2022, letter that indicates that there was no water or bedrock encountered within seven feet of the surface near the proposed drainfield location (See Appendix B). Our site investigation was conducted using a power auger that was capable of sampling to four feet. The owners have already purchased the perforated pipe for the drainfield and a 1,250-gallon septic tank based on the Bear Valley recommendations; since both of which are in compliance with Regulation 43 and our design, they can be used.

Site Investigation

I conducted the site and soil investigation July 22, 2022. Two bore holes/soil profile pits/ (SPP) were excavated near the proposed drainfield location. The soil encountered within the SPPs was evaluated visually and tactilely based on texture, structure and the ribbon test as required for the septic system in accordance with CDPHE/WQCC, Regulation #43 Section 43.5.D.2. No signs of groundwater or restrictive layers were encountered in the SPPs as based on my evaluation and Bear Valley Design's inspection. The soils in the vicinity of the drainfield are predominately silty sand.

Design

Based on the results of the soil classification, the drainfield was sized using the methods outlined in CDPHE/WQCC, Regulation #43. The design calculations are presented in the attachments to this report. The design calculation cite pertinent sections of Regulation 43. Product specifications are either called out on the plan set or are provide in the quantities list provide on Sheet 2 of the plan set.

Please let me know if you have any questions or comments about the design of this OWTS.

Best regards,

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Will A. Myers, P.E.

Appendix A Design Calculations

I. Design Flow Rate						
<u>Design Component</u>			Notes/Information Source			
3-Bedroom House						
No. of Bedrooms:	3					
Occupants based on Bedrooms:	6		Table 6.1 Required for two Bedrooms			
Wastewater Flow per Occupant:	75	gpd	Table 6.1			
Daily Design Flow Rate:	450	gpd				
II. Septic Tank Design						
<u>Design Component</u>			Notes/Information Source			
3-Bedroom House						
No. of Bedrooms:	3					
Min. Septic Tank Capacity Based on						
Bedrooms:	1,000	gal	Table 9.1			
Design Septic Tank Capacity ¹	1,250	gal				

Sources/Notes:

All sources of information and design requirements are CDPHE/WQCC, Regulation #43 unless otherwise noted.

III. Drainfield Design					
<u>Design Component</u>			Notes/Information Source		
Peak daily design flow rate:	450	gpd			
Soil texture within/near drainfield:	Sandy Loam				
Soil Type:	1		Table 10-1		
Treatment Level:	2		Table 10-1		
Long-Term Acceptance Rate (LTAR) ¹ :	0.6	gal/day/sq ft	Table 10-1		
Required drainfield absorption area:	750	sq ft			
Width of drainfield trench:	3.0	ft	Standard Size		
Total required length of drainfield:	250.0				
Number of laterals:	4				
Min. Lateral length:	63	ft			
Design Laterail length:	70				
Total lateral length:	280	ft			
Min. distance between trenches:	4	ft	Section 43.10.F.1		
Min. trench CL spacing:	7	ft			
Design trench CL spacing:	10	ft			

Sources/Notes: All sources of information and design requirements are CDPHE/WQCC, Regulation #43 unless ¹ Determined from a field soil evaluation.

Appendix B Bear Valley Soil Report

Bear Ualley Design, Ltd. Engineers - Consultants



P. O. Box #770475 STEAMBOAT SPRINGS, COLORADO, 80477-0475 MOBILE: (970) 879-5454 E-MAIL: <bearbvd@mindspring.com>

January 29, 2022

Mr. Chris Reed 1807 Dorothy Circle Longmont, Colorado, 80503

Subject: Soil investigation and LTAR evaluation for an OWTS for a proposed residence to be located at 34130 Whiffle Tree Trail, in Routt County, Colorado.

Dear Mr. Reed,

Per your request, we performed a soil investigation and evaluation on the subject site in late November of 2021. The investigation was performed for the purpose of providing soil design parameters for the construction of the foundation and the evaluation was performed for the purpose of designing an Onsite Wastewater Treatment System (OWTS) for a single family residence.

The proposed structure is anticipated to be of single story post and beam wood framed construction with a crawl space. The foundation is proposed to be constructed with reinforced concrete walls bearing upon reinforced concrete spread footers. The building site on the lot is nearly flat and is covered with grass and lodgepole pine trees. The site slopes gently downward toward the north.

A test pit was advanced on the parcel, in the immediate vicinity of the proposed residence. A second pit (a profile hole) was located toward the north, downhill from the proposed location of the residence and in the area where it is proposed to construct the absorption field portion of the OWTS, approximately 30 yards to the north from the proposed residence.

Both pits were advanced using a rubber tire mounted backhoe. The test pit revealed approximately one foot of very slightly moist, medium brown, silty fine sandy loam topsoil overlying a two foot thick stratum of moderately dense, very slightly moist, medium brown silty fine sand, which in turn overlay a very slightly moist, very dense, slightly clayey, silty, fine sand subsoil which extended to the maximum depth explored, seven feet. Conditions in the profile hole were virtually identical to those observed in the test . No free ground water was observed in either pit, nor was any bedrock encountered.

Our experience with similar soils, taken together with our observations in the test pit, have led us to form the opinion that the lower, hard, silty fine sand subsoil will provide stable bearing for the foundation of a structure of the proposed type. Based on the soil conditions exposed in the profile hole, we also concluded that the site is, in fact, suitable for the installation of a typical OWTS absorption field.

Spread footers for the proposed structure should be designed to bear on the lower, very dense, silty, fine sand subsoil, with a maximum net bearing pressure of 4.0 KSF, as well as with a minimum dead load of 1.0 KSF, with the latter intended to control any tendency of the bearing soil to swell lest it ever experiences a significant increase in its moisture content. Any retaining structures should be designed to retain pressure equivalent to that which would be exerted by a fluid weighing 45 PCF.

The foundation's footers must be surrounded with a footer drain. This footer drain must be constructed using 4" diameter D-2729 perforated PVC pipe (with the perforations located at 4 and 8 'o'clock'), bedded and covered with ³/₄" screened rock with the rock underlain by a relatively impervious membrane and covered with a geo-fabric such as 'Mirafi' #140N. The footer drain must run from a pair of clean-outs, with a minimum 0.5 % slope around the foundation to a corner opposite the clean-outs, and there be wyed together to drain to daylight via a non-perforated 4" diameter PVC pipe. This drain must be located at a grade low enough to insure that it will prevent water which might penetrate the backfill from soaking the bearing soil beneath the footers. The daylighted end of this drain should be protected from intrusion by critters by means of a screen and cobbles.

Frost protection for the foundation must be provided by maintaining a minimum of 48" of earth cover over them, measured in any direction. The finish grade should provide for a minimum of 2% slope away from the structure in all directions for a minimum of 10 feet, as well as for positive and continuous drainage away from the building without any ponding. Native subsoil materials will provide appropriate backfill. This backfill may be capped with a maximum six inch thick layer of the native topsoil. The native backfill material must be placed in lifts a maximum of 10 inches thick, with each lift moistened and compacted to 93% of its Standard Proctor density.

In order to control moisture as well as to minimize heating costs for the proposed residence the surface of the subsoil in the crawl space must be covered with a six mil thick sheet of visquene. For purposes of fire protection, this sheet of visquene must be covered with a minimum 1.5 inch thick layer of either crusher fines or 'flow-fill.'.

Provision must be provided for positive venting of radon gas from the crawl space should future testing, completed after construction of the residence prove such venting to be necessary. Because of the fact that radon gas is extremely more dense than air, our recommendation is to provide for gravity venting of any radon, and to provide for provision of a forced draft fan to assist with said gravity venting if such a fan were to prove to be necessary.

No slab on grade construction is anticipated in this residence.

The profile hole revealed conditions virtually identical to those described above in the test pit. Neither bedrock nor any free ground water was encountered in the profile hole nor in the test pit. Our evaluation of the soil exposed in this profile hole is that it classifies as a Type 3 silty sandy, slightly clayey loam, per Table 10.1 in Colorado's Regulation #43. Therefore, the leach field for the proposed OWTS should be designed based on a Long Term Acceptance Rate (LTAR) of 0.35 gallons per square foot per day

Thank you for the opportunity to have been of professional service to you in this matter.

Sincerely, Bear Valley Design, Ltd. Eparne L Gregory H. Hermann Colorado P. E. #17422