

#### GENERAL NOTES

1.1 FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1

1.2	MATERIALS HOT ROLLED STEEL SHAPES (W, & C)	ASTM DESIGNATION A572	MIN. YIELD STRENGTH
			Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42  KSI
	STRUCTURAL TUBING	A500	Fy = 42  KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50  KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fv = 55  KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fv = 55 KSI
	ROOF & WALL SHEETS	A792/A653	$F_{y} = 50, 80 \text{ KSI}$
	CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
	1100 5111102	7.00	1) 00 1.01
			MIN. TENSILE STRENGTH
	MACHINE BOLTS & NUTS	A307	Fu = 60 KSI
	HIGH STRENGTH BOLTS (1" & LESS)	A325-TYPE 1	Fu = 120 KSI
	HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø)		
	ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)	A36/A307/F1554	Fu = 60 KSI
	ANCHOR BOLIS (NOT SOFT LIED BY MI.B.S.)	A30/A30//11334	1 u = 00 K3

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
WINDSCAPE HANDING AND ADDITION OF THE SHOP PRIMER PAINT AS A RESULT OF MPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION).

NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

A) ENDWALL RAFTER SPLICE - 5/8" ø x 0"-1 3/4" A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION D) FLANSE BRACECONNECTIONS - 1/2" ø x 0"-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325—N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325—N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE — BETWEEN ROOF PANELS & RIDGE CAP — BETWEEN WALL PANELS & EAVE/GABLE TRIM

1.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL
BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

- 1.9 ERECTION AND UNLOADING NOT BY G.W.B.
- 1.10 <u>SHORTAGES</u> ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.
- CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
  CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL B DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

#### BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
  STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
  DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
  METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
  TO THE CONTRARY NOTWITHISTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
  BUYER/FAID USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
  EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
  IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STELE PLANS AND PLANS FOR
  OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD
- PRACTICES, LATEST EDITION)
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/
  END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL
  FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND
  SHIPPING SCHEDULE

- 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS, USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)
- 2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE
- UNIESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS M.B.S. DOES NOT DESIGN AND UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE NEITHER THE FABRICATION NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>WARNING</u> IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN—OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY, MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED. WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

(IN PSF)	(IN INCHES)	WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20
NOTE:	•	

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FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

# BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

				- 1	
DESIGN LOADS:	<u>B1 (A):</u>	<u>B2 (B):</u>	D.W.	ME.Z AA	T
DESIGN LOADS:  DESIGN CODE / WIND CODE  OCCUPANCY / RISK CATEGORY  ENCLOSURE  ROOF DEAD LOAD (D) (PSF)  ROOF COLATERAL LOAD (C) (PSF)	: IBC - 15	: IBC - 15			- 1
OCCUPANCY / RISK CATEGORY	:II — Normal	: II — Normal	DATE	07/21/20	-
ENCLOSURE	: Closed	:Partially Enclosed	70	~   &   &	1
ROOF DEAD LOAD (D) (PSF)	: 2.00	: 2.00			
ROOF COLLATERAL LOAD (C) (PSF)	: 3.00	: 1.00			
WIND LOAD			اسا	≅I≡	
ULTIMATE WIND SPEED, (VULT) (MPH)	: 115.00	: 115.00	ISSUE	APPROVAL PERMIT	
WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, (GCpi) WALL PANEL DESIGN WIND PRESSURE (PSF)	: C	: C	157	AP	
INTERNAL PRESSURE COEFFICIENT, (GCpi)	: 0.18/-0.18	: 0.55/-0.55			
WALL PANEL DESIGN WIND PRESSURE (PSF)	: 31.15/-33.79	: 35.47/-37.67			L
	: Closed	:Partially Enclosed			
LIVE LOAD	00.00	20.00			,
PRIMARY FRAMING (PSF)	: 20.00	: 20.00 : No		,	/
TRIB. AREA REDUCTION	: No				/
` ,	: 20.00	: 20.00			/
SNOW LOAD  GROUND SNOW LOAD, (Pg) (PSF) ROOF SNOW LOAD, (Pf) (PSF) SNOW EXPOSURE FACTOR, (Ce) SNOW IMPORTANCE FACTOR, (Is) THERMAL FACTOR, (Ct)	100.00	100.00	$\perp$	/	,
GROUND SNOW LOAD, (Pg) (PSF)	: 100.00	: 100.00		/	/
ROUF SNOW LUAD, (PT) (PSF)	: 100	: 100 : 1.00	`	\/	η.
SNOW IMPORTANCE FACTOR (ID)	.1.00	:1.00		/	/
THERMAL FACTOR (C+)	.1.00	: 1.00	/	′,	K
SEISMIC LOAD			$\mid$	$\dashv$	
SEISMIC LOAD  SEISMIC IMPORTANCE FACTOR, (Ie) SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY	.1.00	.1.00	\	,	\
SITE CLASSIFICATION	· D	. n. oo			/
SPECTRAL RESPONSE ACCELERATION	$S_{S} = 0.270  S_{1} = 0.074$	·Ss = 0.270 ·S1 = 0.074		\	\
SPECTRAL RESPONSE COEFFICIENTS	: Sds = 0.285 : Sd1 = 0.118	: Sds = 0.285 : Sd1 = 0.118			/
BASIC SEISMIC FORCE RESISTING SYSTEM	:STEEL SYSTEM NOT SPECIFICALLY		$\vdash$		_
	DETAILED FOR RESISTANCE		뷔	8	3
	:RIGID FRAMES (OMF)			2	3
TOTAL DESIGN BASE SHEAR, (V) (KIPS)	:BRACED FRAMES (OCBF/OMF)		I A	5	ş
TOTAL DESIGN BASE SHEAR, (V) (KIPS)	:LONGITUDINAL = 14.90		止	(	1
	: TRANSVERSE = 12.40		핑	Š	2
RESPONSE MODIFICATION FACTORS, (R)	· DICID EDAMES - 700	5.50	BOGENHAGEN/JEFF LAROCHE	JEFE LLC STEAMBOAT SPRINGS CO 80488	-
RESPONSE MODIFICATION FACTORS, (R)	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2.50	AG		
	:SW X-BRACING = 3.00 $\Omega$ = 2 :SW WIND BENT = 3.00 $\Omega$ = 2	2.50		0 2	2
	. 34 MIND DEINT - 3.00 32-2		1 8	EL JEFE LLC TRN STFAMRC	į
SEISMIC RESPONSE COEFFICIENTS, (Cs)	:RIGID FRAMES = 0.0951			비볐	<u>.</u>
, , ,	:SW $X-BRACING = 0.0951$		(S)	베	(
	:SW WIND BENT = $0.0951$		CASEY		<u> </u>
				. NOF	Ë
ANALYSIS PROCEDURE USED	: EQUIVALENT LATERAL FORCE PROC	CEDURE	STOMER NAME:	OJECT LOCATION:	SOUN
OTHER LOADS/REQUIREMENTS			OME	ECT	OJECT
			IS G	5I 3	13

<b>BUILDING DESCRIPTION:</b>	<u>B1 (A):</u>	<u>B2 (B):</u>
WIDTH (FT)	: 50.00	: 12.00
LENGTH (FT)	: 80.00	: 80.00
EAVE HEIGHT AT BSW (FT	<b>)</b> : 18	: 13
EAVE HEIGHT AT FSW (FT)	):18	: 15
ROOF SLOPE AT BSW	: 4.0:12	: 2.0:12
ROOF SLOPE AT FSW	: 4.0:12	: N/A
BAY SPACING (FT)	: 4 at 20	: 4 at 20
COVERING AND TRIMS:		
ROOF PANELS & TRIMS		
PANEL TYPE	: 26 GA. PBR	:26 GA. PBR
PANEL COLOR	: GALVALUME	: GALVALUME
TRIM COLORS		

GABLE/EAVE : NEED STD. COLOR : NEED STD. COLOR WALL PANELS & TRIMS :26 GA. PBR PANEL TYPE : N/A PANEL COLOR : NEED STD. COLOR : N/A TRIM COLORS : NEED STD. COLOR : NEED STD. COLOR CORNER FRAMED OPENING : NEED STD. COLOR : NEED STD. COLOR BASE : NEED STD. COLOR : NEED STD. COLOR INSULATION ROOF INSULATION :12" (R-41) WMP-VR : N/A :9-1/2" (R-30) WMP-VR WALL INSULATION : N/A



FLOOR

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PARKER

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TWESTERNBUILDINGS.

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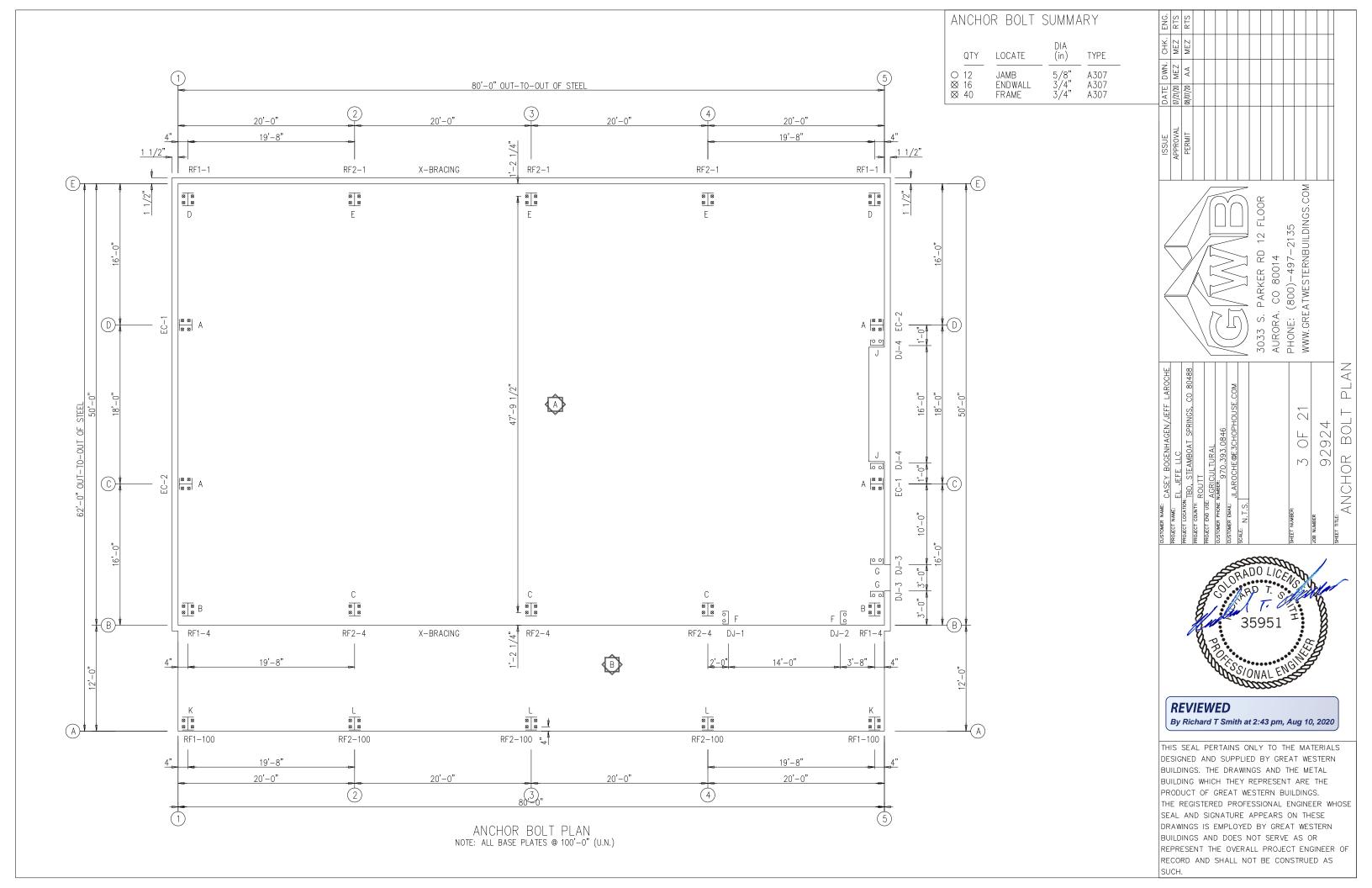
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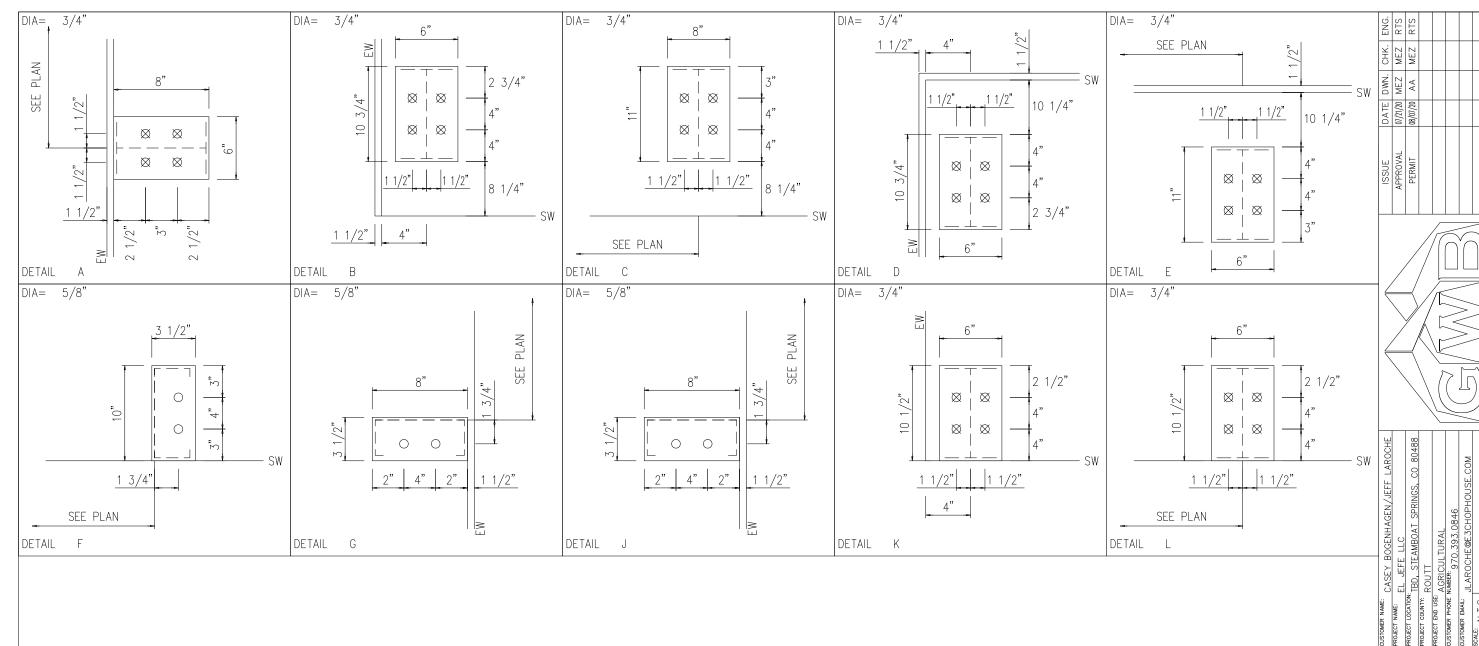
## **REVIEWED**

MEZ MEZ

By Richard T Smith at 2:43 pm, Aug 10, 2020

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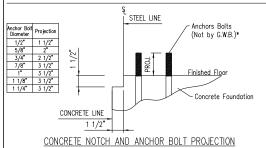


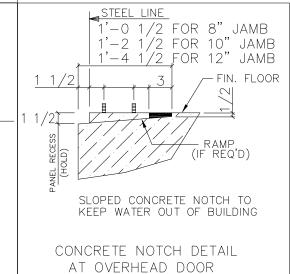
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

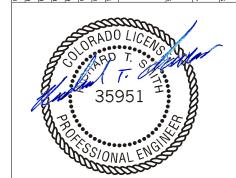
ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

 ${\underline{\tt NOTE:}}$  ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.







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BOL.

ANCHOR

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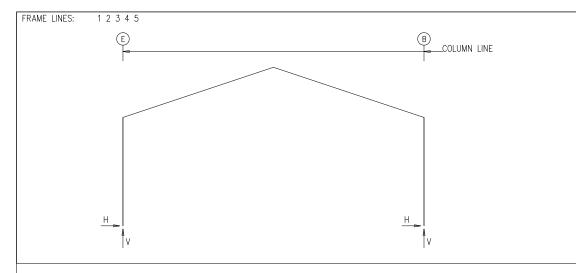
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Frm	Col	Load	——— Colı Hmax	umn_Reac V	tions(k Load	) Hmin		Bol	t(in)	Base	e_Plate(in)		Grout
Line	Line	ld	H	Vmax	ld_	H	Vmin	QTY	DIA	Width	Length ´	Thick	(in)
1*	E	1	11.9	34.9	3	-2.4	-3.9	4	0.750	6.000	10.75	0.375	0.0
1*	В	5 2	2.7 -11.7	-2.5 41.9	1 4	-11.7 1.7	41.0 -4.3	4	0.750	6.000	10.75	0.375	0.0

RIGID	FRAME:		MAXIMUM	REACTION	S, ANC	HOR BOLT	S, & BASI	E PLATE	ES				
Frr Lin		Load Id	Hmax H	umn_React V Vmax	tions(k Load Id	) Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	E	1	21.3	62.2	3 6	-4.5 -0.3	-7.3 -8.0	4	0.750	6.000	11.00	0.375	0.0
2*	В	5 2	5.0 -20.9	-4.7 76.0	1 7	-21.0 -2.7	74.1 -8.4	4	0.750	8.000	11.00	0.375	0.0
2*	FRAME I	ines:	2 3 4										

END	WALL	COLUM	MN:	BASIC	COLUMN	REACTIONS	(k )	
Frm Line 1 1 5	Col Line D C C D	Dead Vert 0.2 0.2 0.2 0.2	Wind Press Horz -4.4 -4.4 -4.4	Wind Suct Horz 4.9 4.9 4.9				

	ENDWALL	COLUMN:	MAXIMUM RFA	CTIONS. ANCHOR	BOLTS.	& BASE	PLATES
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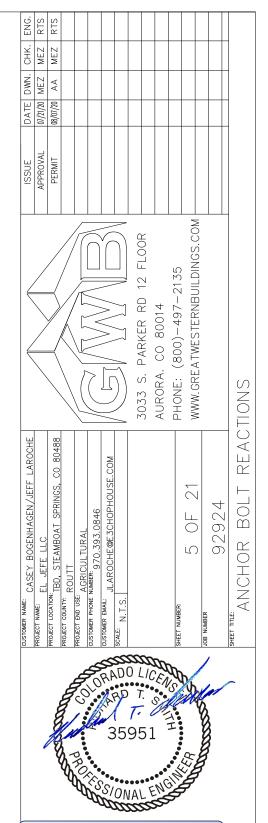
Frm Line	Col Line	Load Id	—— Coli Hmax H	ımn_Reac V Vmax	tions(k Load Id - —	) Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
1	D	8 10	2.9 0.0	0.1 0.2	9	-2.7	0.1	4	0.750	6.000	8.000	0.375	0.0
1	С	8 10	2.9 0.0	0.1 0.2	9	-2.7	0.1	4	0.750	6.000	8.000	0.375	0.0
5	С	8 11	2.9 0.0	0.1 0.2	9	-2.7	0.1	4	0.750	6.000	8.000	0.375	0.0
5	D	8 11	2.9 0.0	0.1 0.2	9	-2.7	0.1	4	0.750	6.000	8.000	0.375	0.0

ANCHO	R BOLT	SUMMA	ARY
QTY ── ○ 12 ⋈ 16 ⋈ 40	LOCATE JAMB ENDWALL FRAME	DIA (in) 5/8" 3/4" 3/4"	A307 A307 A307

RIGIE	FRAN	ЛЕ:	BASI	C COLUM	N REACT	IONS (k )	)						
FRAME Line 1* 1*	Column Line E B	Horiz 0.4 -0.4	-Dead Vert 1.5 1.7		oteral— Vert 1.0 1.0		-Live Vert 6.0 7.2	Horiz 11.1 –10.9	-Snow Vert 32.4 38.2	Snov Horiz 0.0 0.0	v_Drift- Vert 0.0 0.3	−−Slide Horiz 0.0 0.0	e_Snow- Vert 0.0 0.9
FRAME Line 1*	Column Line E B	Horiz	H_Left1- Vert -8.1 -5.2	-Wind_ Horiz 0.2 3.2	Right1- Vert -5.0 -8.9	Wind Horiz -3.7 -0.1	Left2- Vert -4.7 -2.1	-Wind_ Horiz 1.0 4.9	_Right2- Vert -1.6 -5.8	Wind Horiz -0.6 -1.1	_Long1- Vert -6.2 -5.7	Wind Horiz -1.4 -1.9	I_Long2- Vert -5.7 -6.2
FRAME Line 1*	Column Line E B	Horiz −1.0	ic_Left Vert -0.7 0.7	Horiz	_Right Vert 0.7 -0.7	F1UNB_ Horiz 9.2 -9.2	SL_L- Vert 26.5 15.3	F1UNB_ Horiz 9.2 -9.2	SL_R- Vert 15.4 26.3				
FRAME Line 2* 2*	Column Line E B	Horiz 0.7 -0.7	-Dead Vert 2.5 2.8	-—-Collo Horiz 0.6 -0.6	nteral— Vert 1.7 1.8	 Horiz 4.1 -4.0	-Live Vert 10.8 13.1	Horiz 19.9 –19.6	-Snow Vert 58.0 69.5	Snov Horiz 0.0 0.0	v_Drift- Vert 0.0 0.7	Slide Horiz 0.0 0.1	e_Snow- Vert 0.0 1.9
FRAME Line 2* 2*	Column Line E B	Wind Horiz -8.2 -3.5	l_Left1- Vert -14.6 -9.3	-Wind_ Horiz 0.2 5.7	Right1- Vert -9.0 -16.2	Wind Horiz -6.6 -0.1	Left2- Vert -8.4 -3.7	-Wind_ Horiz 1.9 9.0	_Right2- Vert -2.7 -10.6	Wind Horiz -1.2 -2.3	_Long1- Vert -15.7 -15.9	Wind Horiz -2.6 -3.7	I_Long2- Vert -14.8 -16.8
FRAME Line 2* 2*	Column Line E B	-Seism Horiz -1.7 -1.9	ic_Left Vert -1.2 1.3		_Right Vert 1.2 -1.2	-Seism Horiz 0.3 0.5	ic_Long Vert -4.7 -7.1	F2UNB. Horiz 16.6 –16.6	_SL_L- Vert 47.4 27.4	F2UNB_ Horiz 16.6 -16.6	SL_R- Vert 27.7 47.2		
	FRAME lir FRAME lir		1 5 2 3 4	ļ									

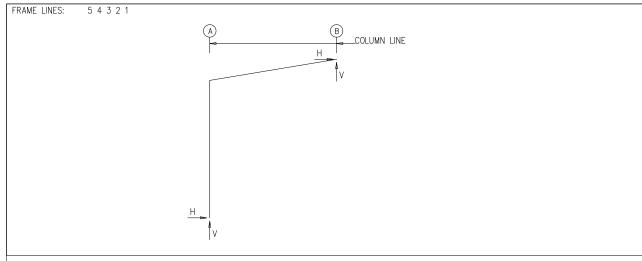
			_							
NOTES	FOR	REACTION	IS							
	followir Width Length Eave H Roof S Dead   Collate Live L Snow Ultima Wind (Closed Import Seismi	i (ft) Height (ft) Slope (rise/12) Load (psf) eral Load (psf) oad (psf) Load (psf) Load (psf) to Wind Speed Load (psf)	a: (mph)	= = = = = = = = = = = = = = = = = = =	50.00 80.00 18 / 18 4.0:12 / 4.0:12 2.00 3.00 20.00 100 115.00 IBC - 15 C Closed 1.00 1.00 B 0.43					
ID	Descrip	otion								
1 Dead+Collateral+Snow 2 Dead+Collateral+Snow+Slide_Snow 3 0.6Dead+0.6Wind_Left1 4 0.6Dead+0.6Wind_Right1 5 0.6Dead+0.6Wind_Right2 6 0.6Dead+0.6Wind_Long1L 7 0.6Dead+0.6Wind_Long1L 8 0.6Dead+0.6Wind_Long2L 8 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L 10 1.03Dead+1.03Collateral+0.52Seismic_Right+0.75E1UNB_SL_R 11 1.03Dead+1.03Collateral+0.52Seismic_Right+0.75E2UNB_SL_R										

BUIL	DING	BRACI	NG RE	:ACTI(	DNS				
w	all —	– Col			ons(k ) - —Sei		Panel_ - (Ib	_Shear /ft)	
Loc	Line	Line	Horz			Vert	Wind	Śeis	Note
L_EW	1								
F_SW	В	2,3	7.3	5.7	8.7	6.8			(h)
R_EW B_SW	B 5 E	3,2	5.5	4.4	6.2	5.0			(h)
(h)Rigid	d frame	e at endv	vall						





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REPRESENT THE OVERALL PROJECT ENGINEER OF
RECORD AND SHALL NOT BE CONSTRUED AS
SLICH



_				ımn_React					<i>(.</i> )	_			
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Boli QTY	:(in) DIA 	Base Width	e_Plate(in) Length -	Thick	Grout (in) 
5*	Α	5 1	0.9 0.2	-0.8 7.8	3 4	-0.9 0.9	0.3 -1.1	4	0.750	6.000	10.50	0.375	0.0
5*	В	4 1	1.5 -0.2	-0.7 7.0	3 2	-1.0 0.5	-0.2 $-0.9$	0	0.000	0.000	0.000	0.000	0.0

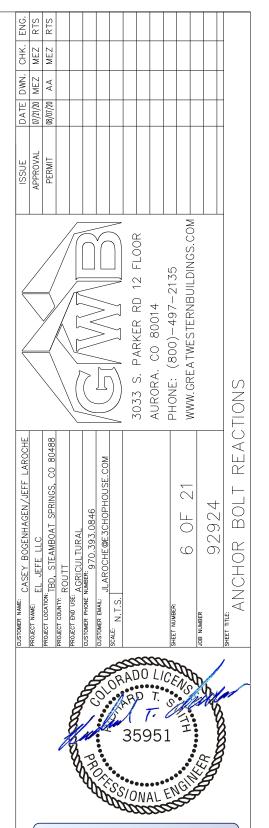
RIGID	FRAME:		MAXIMUM	REACTION	NS, ANCH	HOR BOLT	S, & BAS	E PLATE	ES				
Frm Line		Load Id	Hmax H	ımn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
4*	Α	5 1	1.7 0.4	-1.8 15.1	3 4	-1.7 1.7	0.4 -2.4	4	0.750	6.000	10.50	0.375	0.0
4*	В	5 1	3.0 -0.4	-1.0 13.8	3 2	-2.0 1.0	−0.4 −1.8	0	0.000	0.000	0.000	0.000	0.0
4*	FRAME li	nes:	4 3 2										

NOTES FOR REACTIONS	
Building reactions are based on the following building data:  Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= 12.00 = 80.00 = 13 / 15 = 2.0:12 = 2.00 = 1.00 = 100 = 115.00 = 18C - 15 = C = Partially Enclosed = 1.00 = 1.00 = 1.00 = 0.43
ID Description	
1 Dead+Collateral+Snow+Slide_Snow 2 0.6Dead+0.6Wind_Left1 3 0.6Dead+0.6Wind_Left2 4 0.6Dead+0.6Wind_Long1R 5 0.6Dead+0.6Wind_Long2R	

RIGII	) FRAM	ΛE:	BASI	C COLUM	IN REACT	TONS (k	)						
FRAME Line 5* 5*	Column Line A B	Horiz 0.0 0.0	-Dead Vert 0.4 0.2		ateral— Vert 0.1 0.1	Horiz 0.0 0.0	-Live Vert 1.3 1.2	Horiz 0.2 –0.2	-Snow Vert 6.3 5.9	Sno Horiz 0.0 0.0	w_Drift- Vert 0.0 0.3	Slide Horiz 0.0 0.0	Snow- Vert 1.0 0.9
FRAME Line 5* 5*	Column Line A B	Wind Horiz 0.1 0.9	_Left1-   Vert   -2.0   -1.7	-Wind_ Horiz 1.3 2.1	Right1- Vert -1.8 -1.1	Wind Horiz -1.5 -1.7	I_Left2- Vert 0.1 -0.5	-Wind_ Horiz -0.3 -0.5	Right2- Vert 0.3 0.1	Wind Horiz 1.4 2.4	d_Long1- Vert -2.3 -1.4	Wind Horiz 1.4 2.3	_Long2- Vert -1.8 -1.0
FRAME Line 5* 5*	Column Line A B	-Seism Horiz 0.0 -0.3	ic_Left Vert 0.0 0.0	Seismic Horiz 0.0 0.3	:_Right Vert 0.0 0.0	-MIN_S Horiz 0.0 0.0	NOW Vert 1.3 1.2						
FRAME Line 4* 4*	E Column Line A B	Horiz 0.0 0.0	-Dead Vert 0.5 0.3	-—-Collo Horiz 0.0 0.0	ateral— Vert 0.1 0.1	 Horiz 0.1 -0.1	-Live Vert 2.5 2.3	Horiz 0.3 -0.3	-Snow Vert 12.5 11.5	Sno Horiz 0.0 0.0	w_Drift- Vert 0.1 0.6	Slide Horiz 0.1 -0.1	e_Snow- Vert 2.0 1.8
FRAME Line 4* 4*	Column Line A B	Wind Horiz 0.2 1.7	l_Left1- Vert -4.0 -3.3	-Wind_ Horiz 2.5 4.1	Right1- Vert -3.5 -2.1	Wind Horiz -2.9 -3.3	I_Left2- Vert 0.1 -1.0	-Wind_ Horiz -0.6 -0.9	Right2- Vert 0.6 0.3	Wind Horiz 2.8 4.3	d_Long1- Vert -4.4 -2.9	Wind Horiz 2.8 5.0	_Long2- Vert -3.5 -1.9
FRAME Line 4* 4*	Column Line A B	-Seism Horiz 0.0 -0.6	ic_Left Vert 0.1 -0.1	Seismic Horiz 0.0 0.6	:_Right Vert -0.1 0.1	-Seism Horiz 0.0 -0.8	ic_Long Vert 0.1 -0.1	-MIN_S Horiz 0.1 -0.1	NOW Vert 2.5 2.3				
5* 4*	FRAME lir FRAME lir		5 1 4 3 2	) -									

AΝ	<b>ICHO</b>	R BOLT	SUMMA	RY
×	QTY 20	LOCATE FRAME	DIA (in) 3/4"	TYPE

BUILE	DING	BRACIN	G RE.	ACTIO	NS			
Wo	ıll — Line					mic — Vert		Note
L_EW F_SW R_EW B_SW	5 B 1 A	Torsional	Bracing	Used				(h) (e) (h)
(e)Brac (h)Rigic	ing load d frame	ds must be at endwal	e applied I	d to sup	porting	building		





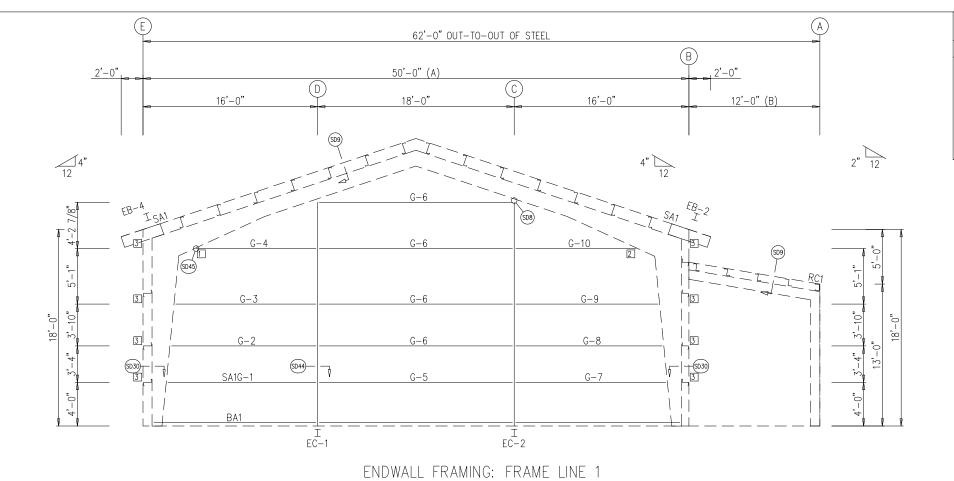
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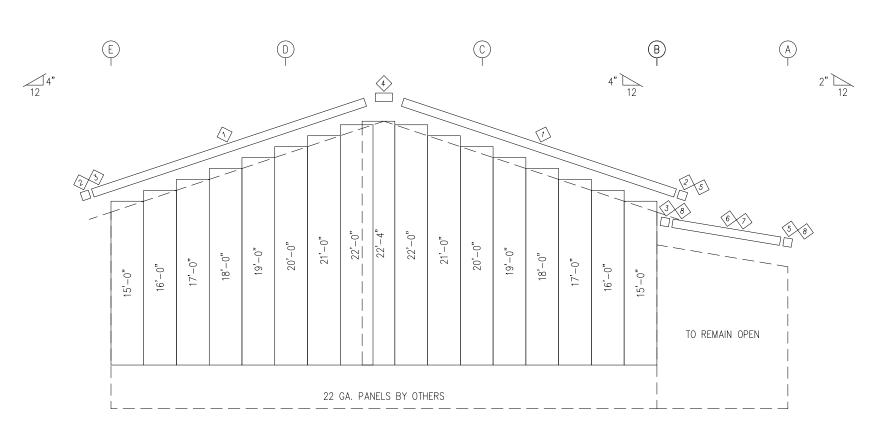
SPLICE BOLT TABLE	MEMBER 1	ARIF					
Qty	Mark	Web Depth	Web Plate	Outside Flange	ath	Inside Flange	
Mark   Top Bot Int Type Dia Length	RF1-1	Start/End 10.0/21.8 21.8/27.7	Thick Length 0.164 10'-0" 0.188 4'-11 13	Outside Flange W x Thk x Ler 5 x 1/4" x 1' 5 x 1/4" x 3	7'-1 7/8" '-4 1/2"	Inside Flange W x Thk x Length 5 x 3/8" x 9'-8 9/16" 5 x 1/2" x 5'-7 1/8"	MEZ RTS RTS RTS
SP-1       4       4       2       A325       7/8"       2       1/2"         SP-2       4       4       2       A325       5/8"       2"         SP-3       4       0       0       A325       5/8"       1       3/4"	RF1-2	21.8/27.7 27.7/28.0 27.0/16.0 16.0/16.0	Web Plate	/16" /16"		5 x 1/4" x 8'-7 3/8" 5 x 1/4" x 14'-6 9/16"	DATE DWN. 01/21/20 MEZ 06/07/20 AA
▼FLANGE BRĄCĘS: FBxx (1 or 2)	RF1-3	16.0/16.0	0.135 5'-0" 0.135 5'-0"	5 x 5/16" x 5 x 1/4" x 1		5 x 1/4" x 14'-6 9/16" 5 x 1/4" x 8'-9 1/2"	DATE 07/21/2 08/07/2
xx=length(in) (1) One Side; (2) Two Sides	RF1-4	160/160	0.164   10'-0" 0.250   8'-8 15	/16"   5 x 1/4" x 1"			ν VAL
A - 2X2X14Ga B - 2X2X1/8		16.0/27.0 28.0/27.7 27.7/21.9 21.9/10.0	0.188   2-11 / 0.188   4'-11 3 0.164   10'-0"	/16"		5 x 1/2" x 5'-6 1/2" 5 x 3/8" x 9'-8 9/16"	ISSUE APPROVAL
	EB-2 EB-4 RF1-100	11.5/11.5	0.135   4'-5 3/	/8"	'-5 3/8" <u>'-5 3/8"</u>	5 x 1/4" x 4'-5 3/8" 5 x 1/4" x 4'-5 3/8"	Σ
	RF1-100	10.0/10.0	0.135 12'-4 1 0.135 11'-4 1,	5 x 1/4" x 1	2 -3 0 3/8" 1'-2 7/16"	5 x 1/4" x 11'-6 3/8" 5 x 1/4" x 11'-2 7/16"	3033 S. PARKER RD 12 FLOOR AURORA. CO 80014 PHONE: (800)-497-2135 www.greatwesternbuildings.com
4		,	,				135 135 1135
							RD 14 14 2 37 - 2 3 N B U I
, , , 1/4"			26'-4 1/4"				3KER 8000 00)-4.0 ESTEI
26-4-118"	ž. /	7	@ 3'-6 7/8"				800 C E ATW
TB34A(1) FB34A(1) S	*		7/8"				10 NE N. GR. W. GR.
04(1)	T - FB33.	<sup>8</sup> A(1)		4"7			303 303 Www
12 4" FB33.8A(1) FB33.8A(1) FB33.8A(1)	1	FB33.8A(1)	FB34A(1)	4"	2		S, CO 80488 SE.COM
2:-15/16" SD115 SD115 SD115	RF1-3			FB40A(1)	2'-1 5/16"		WE CASEY BOGENHAGEN/JEFF LAROCHE TO THE LIC TITY: ROUTT WE AGRICULTURAL JUENCHE©E3CHOPHOUSE.COM WE JLAROCHE©E3CHOPHOUSE.COM S.    POR DATA  92924  PURCHE BOGENHAGEN S.    92924
FBAUTY				9"	12'-2"	2" 12	BOGENHAGEN/J E LLC EAMBOAT SPRIN LTURAL 70.393.0846 CHE©E3CHOPHO 7 OF 2 92924 FRAME
		 		EB-2, E	4 @ 2'-10 1/4"	12	OGENH LLC LLC LUCAL TURAL 0.393.0 FEGE30 FEGE30 FEGE30
[B-2, EB-A]			T		30A(1) 💆		CASEY BOGENHAGEN/JEFF EL JEFE LLC **TBD, STEAMBOAT SPRINGS, GROUTT **ACRICULTURAL **NAWBER: 970,393,0846 JLAROCHE®E3CHOPHOUSE  JLAROCHE®E3CHOPHOUSE  SOUTH STANKER   100
					FB30A(1)	7 - 0	NAME: C COATION: II  TOOUNTY: R PHONE NU USE: A II  TI S.   U II T.S.   U II  TERMIT: A II  TERMIT:
FB38.5B(1)  FB38.5B(1)  SF  SI  CLEAR +/-		 		1 1 1 2 ~	RF1-101		CUSTOMER PROJECT IN PROJECT IC USTOMER CUSTOMER SHEET NUM SHEET TITL
		 			115)		School 100 L/Cein
			15'-6 3/4"	F1-4 4.3A(1)		£ 1 £ 1	DE T. S. JAMES
FB34.8A			15,-	FB33	11'-6 13/16" CLEAR +/- RF1-100	13,-0	35951
		 			11,-6 CLEAF RF1		
							S/ONAL ENGL
		II ∐	V	— nam			Manuser
10 1/4" 43'-8" CLEAR +/-				8 1/4"	11'-1 1/2" CLEAR +/-	10 1/2"	REVIEWED  By Richard T Smith at 2:43 pm, Aug 10, 2020
					·		
2'-0" 50'-0" OUT-TO-OUT OF STEEL (A)				2'-0" 12'-	- <u>0" OUT-TO-OUT OF STEE</u> (B)		THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL
E				(d		A	BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS.
RIGID FRAME E	LEVATION:	FRAME LIN	IE 1 5				THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE
							DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR

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SUCH.

SPLICE BOLT TABLE	MEMBER TABL	F				(6) (0) (0)
Qty	Mark S	eb Depth Start/End	Web Plate	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length	- NA TA
Mark Top Bot Int Type Dia Length	RF2-1 1	<u>Start/End</u> 0.0/34.0	Thick Length 0.250 18'-1 5/16"	W x Thk x Length 6 x 1/4" x 17'-1 7/8"	W x Thk x Length 6 x 3/4" x 10'-4 15/16"	MEZ MEZ
SP-1       4       4       4       325       1"       2       3/4"         SP-2       4       4       2       A325       5/8"       2       1/4"         SP-3       4       0       0       A325       5/8"       1       3/4"				6 x 1/4" x 17'-1 7/8" 6 x 1/4" x 3'-10 13/16" 6 x 1/4" x 6'-11" 6 x 5/16" x 10'-0" 6 x 3/8" x 5'-4" 6 x 3/8" x 15'-4" 6 x 1/4" x 7'-1 1/8"	W x 1hk x Length  6 x 3/4" x 10'-4 15/16"  6 x 1/2" x 4'-8 3/4"  6 x 5/16" x 10'-4"  6 x 1/4" x 4'-1 9/16"  6 x 1/4" x 4'-1 9/16"  6 x 5/16" x 10'-4"  6 x 1/2" x 8'-4 3/16"  6 x 1/2" x 4'-8 1/16"  6 x 3/4" x 10'-4 15/16"  5 x 1/4" x 4'-5 3/8"	0 2 2
SP-2     4     4     2     A325     5/8"     2     1/4"       SP-3     4     0     0     A325     5/8"     1     3/4"	RF2-2 3	31.0/19.0 an/1an	0.250   8 -1 //16"	6 x 1/4" x 6"-11" 6 x 5/16" x 10'-0"	6 x 1/2" x 8'-2 1/8" 6 x 5/16" x 10'-4"	DWNN.
	1	9.0/19.0	0.164 5',-0"	6 × 3/8" × 5'-4"	6 x 1/4" x 4'-1 9/16"	01/21/20 08/07/20
VELANGE BRACES: EByx (1 or 2)	RF2-3	9.0/19.0	0.164   5'-0"	6 x 3/8" x 15'-4"	6 x 1/4" x 4'-1 9/16"	07/21
FLANGE BRACES: FBxx (1 or 2) xx=length(in) (1) One Side; (2) Two Sides	1	81.0/19.0 9.0/19.0 9.0/19.0 9.0/19.0 9.0/19.0 9.0/31.0 34.0/33.6 33.6/10.0	0.250 8'-1 7/16" 0.188 10'-0" 0.164 5'-0" 0.164 5'-0" 0.188 10'-0" 0.250 8'-3 1/2" 0.313 3'-4 1/16" 0.250 14'-8 1/2" 0.135 4'-5 3/8"		6 x 1/2" x 8'-4 3/16"	
(1) One Side; (2) Two Sides	RF2-4 3	34.0/33.6	0.313 3'-4 1/16" 0.250 14'-8 1/2" 0.135 4'-5 3/8"	6 x 1/4" x 3'-8 11/16"	6 x 1/2" x 4'-8 1/16"	HIT HIT HIS
B - 2X2X1/8 A - 2X2X14Ga		1.0/11.0	0.135 4'-5 3/8"	5 x 1/4" x 1/-1 3/10 5 x 1/4" x 4'-5 3/8"	5 x 1/4" x 4'-5 3/8"	ISSUE ISSUE PERMIT
	RF2-100 1	0.0/10.0	0.135 12'-4 11/16"	5 x 1/4" x 12'-3"	5 x 1/4" x 11'-6 3/8"	
	RF2-101 1	0.0/10.0	0.135 11'-4 1/8"	6 x 1/4" x 3'-8 11/16" 6 x 1/4" x 17'-1 3/16" 5 x 1/4" x 4'-5 3/8" 5 x 1/4" x 12'-3" 5 x 1/4" x 10 3/8" 5 x 1/4" x 11'-2 7/16"	5 x 1/4" x 11'-2 7/16"	
						VER RD 12 FLOOR 80014 9-497-2135 STERNBUILDINGS.COM
						//(∩, ∩) 8. 8. 8.
						7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
						0 4 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
		_				KER RC 80014 80014 SSTERNE
2, 4, 14,		26'-	4 1/4"			KES O O S & S KE
26		7.0 2	1/4"			PARKER CO 800 (800)-49
103-6710	/		3'-6 7/8"			3033 S. PARKE AURORA. CO 81 PHONE: (800)- WWW.GREATWEST
FB35.8A(1) FB35.8A(1) PB35.8A(1)						M
FB35.8A(1)	FB35.8A(1)					30 A D H H W WW
FB35.5	$\frac{3.0A(1)}{2}$					FF LAROCHE S. CO 80488 303 303 4 UR PHO PHO ELEVATION
12 FB35.8B(1)	<u> </u>	F	<sup>EB35.8B(1)</sup>	4" 12		E. CASEY BOGENHAGEN/JEFF LAROCHE OW: TBD, STEAMBOAT SPRINGS, CO 80488 TROUTT SE. AGRICULTURAL LE AGRICULTURAL TO JUAROCHE@E3CHOPHOUSE.COM S.   S.   S.   S.   S.   S.   S.   S.
FB50.9 FB50.9 SD115	/	<u></u>		2'-1 =		CO (SO )
$\frac{2^{1}-15/16}{100}$ (SD115) $\frac{2}{100}$ (SD115) $\frac{2}{100}$ (SD115)	RF2-3		FB43.3B(1)	15/16"		NGS.   C   C   C   C   C   C   C   C   C
FB3127 I	~			9" 12-2"	- " >	
		4		4 @ 2'-10 1/4"	2" 12	BOGENHAGENA, E. LLC EAMBOAT SPRIN LTURAL O.393.0846 HEGE3CHOPHC NEGE3CHOPHC PRAME
				EB-3	4 4	OGEN AMBO D.3933 B.3934 EE GE.3 P.T.Y.
5 EB-3				FB30A(1) FB30A(1)	3,-0,,	STEA STEA SOULT SO
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FB42.3B(1) SP-				RF2-101	_	PROJECTON PROJEC
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18/2 - 1/2 + +			15'-4 3/8" CLEAR +/- RF2-4	l i		FO Jap T S
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10 1/4" 42'-8"				_	10 1/2"	REVIEWED
10 1/4" 42'-8" 2'-10 3/4" CLEAR +/-			2'-10			By Richard T Smith at 2:43 pm, Aug 10, 2020
2'-0" 50'-0" OUT-TO-OUT OF STEEL				2'-0" 12'-0" OUT-TO-OUT OF	STEEL	THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN
				(B)		BUILDINGS. THE DRAWINGS AND THE METAL
				_		BUILDING WHICH THEY REPRESENT ARE THE
RIGID FRAME ELEV	/ATION: FRAM	ME LINE :	7			PRODUCT OF GREAT WESTERN BUILDINGS.  THE REGISTERED PROFESSIONAL ENGINEER WHOSE
(E)	VALION. LIVAI	VIL LIINL Z	_		(A)	SEAL AND SIGNATURE APPEARS ON THESE
						DRAWINGS IS EMPLOYED BY GREAT WESTERN
						BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF
						RECORD AND SHALL NOT BE CONSTRUED AS
						SUCH.





ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 GA. PBR - NEED STD. COLOR

TRIM FRAME	TABLE E LINE 1								ENG.	RTS
◇ID	QUAN	PART		ENGTH			DETA	AIL	Ę.	MEZ
1 2 3 4 5 6 7	6 2 2 1 2 2	FL-15 FL-601E FL-600L FL-17 FL-600R FL-15 FL-78	5 1 5 6	0'-0" 1/8" 5/8" -4" 5/8" -8" 2'-2"			TD36 TD85 TD13 TD13 TD25 TD25	5 3 5	DATE DWN. CH	MEZ
8	2	FL-601C	7	 13/16"			TD8:			7
F	BOLT TAI	INE 1	<b>'</b>		T) (D.E.	1			ISSUE	APPROVAL
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		ER TABLE E LINE 1								/
	QUAN	MARK	P	ART		LENGT			<u></u> ∐∠	<u>/_</u>
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		G-9 G-10	8	X25Z16 X25Z16		12'-1' 10'-1	19/	16"	AROCHE	
		G-10		CONNE	ECTION LINE QUAN 1 1 8		ES (	16	CUSTOMER NAME: CASEY BOGENHAGEN /JEFF LAROCHE	PROJECT NAME: EL JEFE LLC

# REVIEWED

By Richard T Smith at 2:43 pm, Aug 10, 2020

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FRAMING

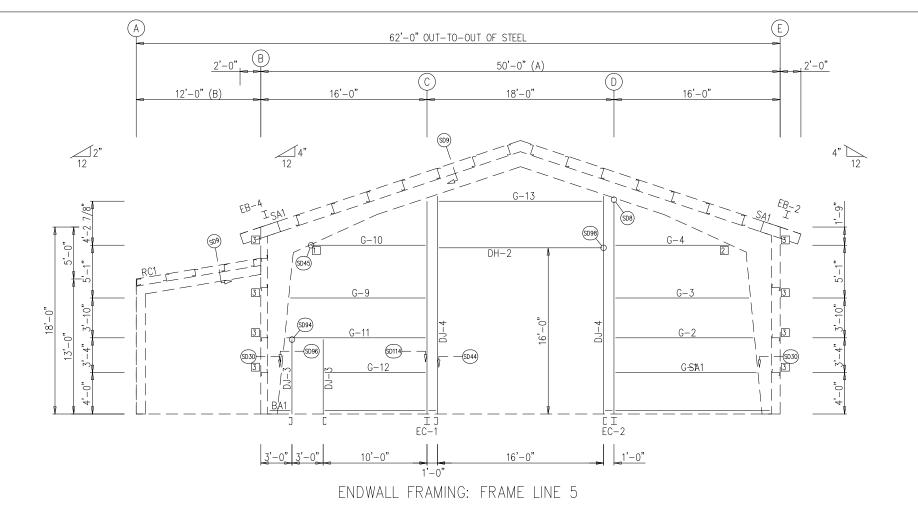
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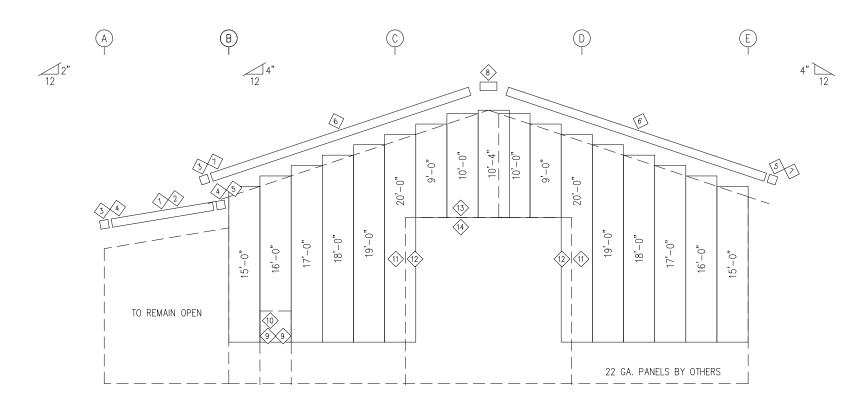
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ENDWALL SHEETING & TRIM: FRAME LINE 5
PANELS: 26 GA. PBR - NEED STD. COLOR

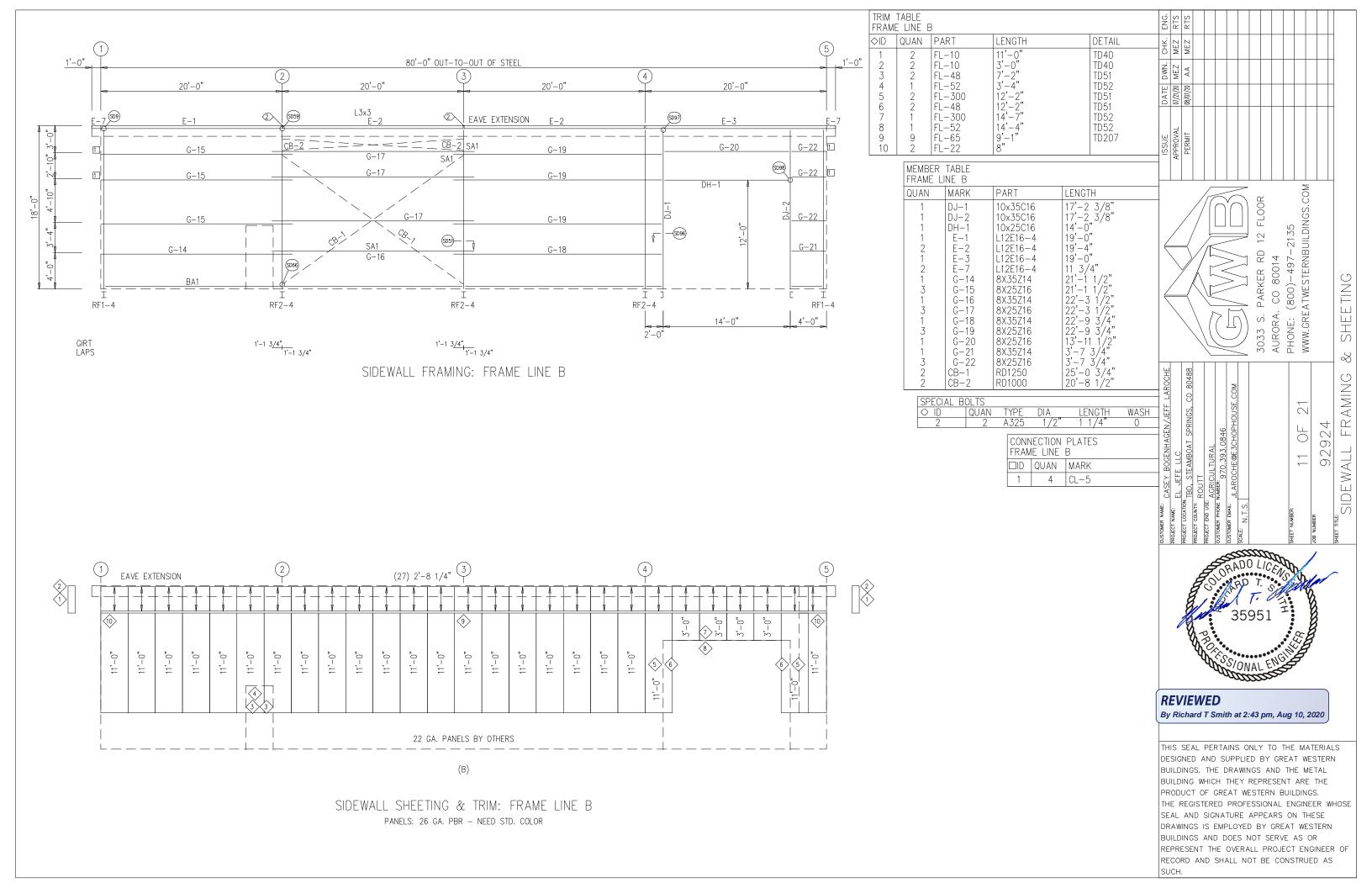
TRIM FRAMI	TABLE E LINE 5	- -					ENG. RTS	RTS								
♦ID	QUAN	PART	LENGTH		DETAIL					+						
1 2 3 4 5 6 7	2 1 2 2 2 6	2 FL-15 6'-8" TI 1 FL-78 12'-2" TI 2 FL-600L 5 5/8" TI 2 FL-601C 7 13/16" TI 2 FL-600R 5 5/8" TI 6 FL-15 10'-0" TI 2 FL-601E 8 1/8" TI 1 FL-17 1'-4"					DATE DWN. CHK.	A A								
8 9 10 11 12 13	1 2 1 2 2 2 1	FL-17 FL-48 FL-52 FL-55 FL-48 FL-55 FL-52	1'-4" 7'-2" 3'-4" 16'-2" 16'-2" 16'-7" 16'-4"		TD51 TD52 TD51 TD51 TD51 TD52 TD52	_	ISSUE	PERMIT					M			
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	FRAMI	ER TABLE E LINE 5	DADT	LENC	TIL						S. PAR	2A. CO	:: (800 :REATWE		EETIN	
	QUAN 1 1 1	EB-2 EB-4 EC-1	PART W12541 W12541 W8X10	LENG 4'-5 4'-5 20'-	3/8" 3/8" 3/8" 8 15/16"						3033	AURORA.	PHONE:		& SH[	
	1 2 2 1 1 1 1 1 1 1 1 1 1 1	EC-2 DJ-3 DJ-4 DH-2 G-1 G-2 G-3 G-4 G-9 G-10 G-11 G-12 G-13	W8X10 8x25C16 8x25C16 8x25C16 8X35Z14 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16	13 - 1 13' - 1 12' - 1 10' - 1 10' - 1 13' - 4	3/8" 3/8" 8 15/16" 8 15/16" 0 15/16" 0 3/16" 2 1/4" 9 9/16" 11 1/4" 11 1/16" 4 1/4"		CASEY BOGENHAGEN/JEFF LAROCHE			JLAR			10 OF 21	92924	NDWALL FRAMING	
			FRAME	CTION PLATINE 5  QUAN MAR  1 b1 1 b2 8 4X2			PROJECT NAME:	PROJECT LOCATION: PROJECT COUNTY:	PROJECT END USE CUSTOMER PHONE	CUSTOMER EMAIL:	ORA!	500 E 00 E 7	ICENS	JOB NUMBER	SHET THE	
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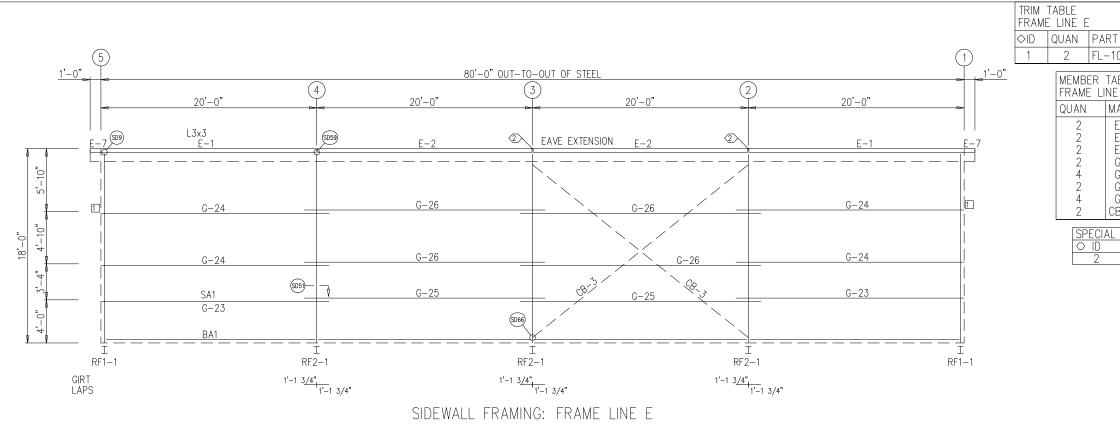
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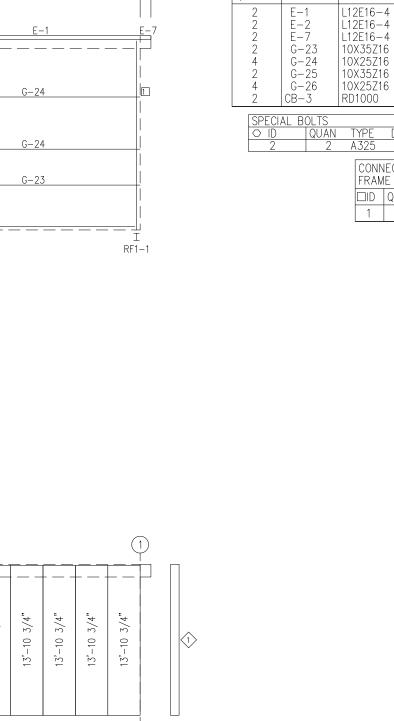
THIS SEAL PERTAINS ONLY TO THE MATERIALS

By Richard T Smith at 2:43 pm, Aug 10, 2020

**REVIEWED** 







LENGTH

14'-0"

PART

2 FL-10

MEMBER TABLE FRAME LINE E

MARK

QUAN

DETAIL

TD40

LENGTH

19'-0" 19'-4"

CONNECTION PLATES FRAME LINE E

□ID QUAN MARK

1 2 CL-5

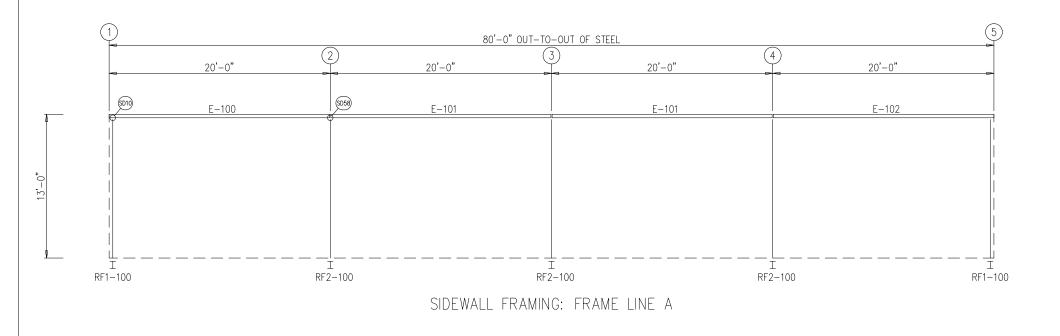
19 - 4 11 3/4" 21'-1 1/2" 21'-1 1/2" 22'-3 1/2" 22'-3 1/2" 26'-5 3/4"

EAVE EXTENSION 13'-10 3/4" 13'-10 3/4" 13'-10 3/4" 13'-10 3/4" 13,-10 13'-10 13'-10 13'-10 22 GA. PANELS BY OTHERS

> SIDEWALL SHEETING & TRIM: FRAME LINE E PANELS: 26 GA. PBR - NEED STD. COLOR

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	CASEY BOGENHAGEN/JEFF LAROCHE		SUECT LOCATION: TBD, STEAMBOAT SPRINGS, CO 80488				    ≥									SIDEWALL FRAMING & SHEETING
	F LAF		, co				JLAROCHE@E3CHOPHOUSE.COM						7	- I		
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	BOGE	-E LL(	TEAMB		ULTUR	70.39	CHE						-	- 1	6	74
	CASEY	EL JEFE LLC	TBD, S	ROUTT	AGRIC	JUMBER:	JLARO									)EV
	6ii	IAME:	OCATION:	JECT COUNTY:	OJECT END USE: AGRICULTURAL	STOMER PHONE NUMBER: 970.393.0846	TOMER EMAIL:	N.T.S.				ABER:		84		
	CUSTOMER	PROJECT NAME:	PROJECT L	PROJECT O	PROJECT E	CUSTOMER	SUSTOMER	SCALE:				SHEET NUMBER:		JOB NUMBER		SHEET TITLE:
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		TO REMAIN OPEN		

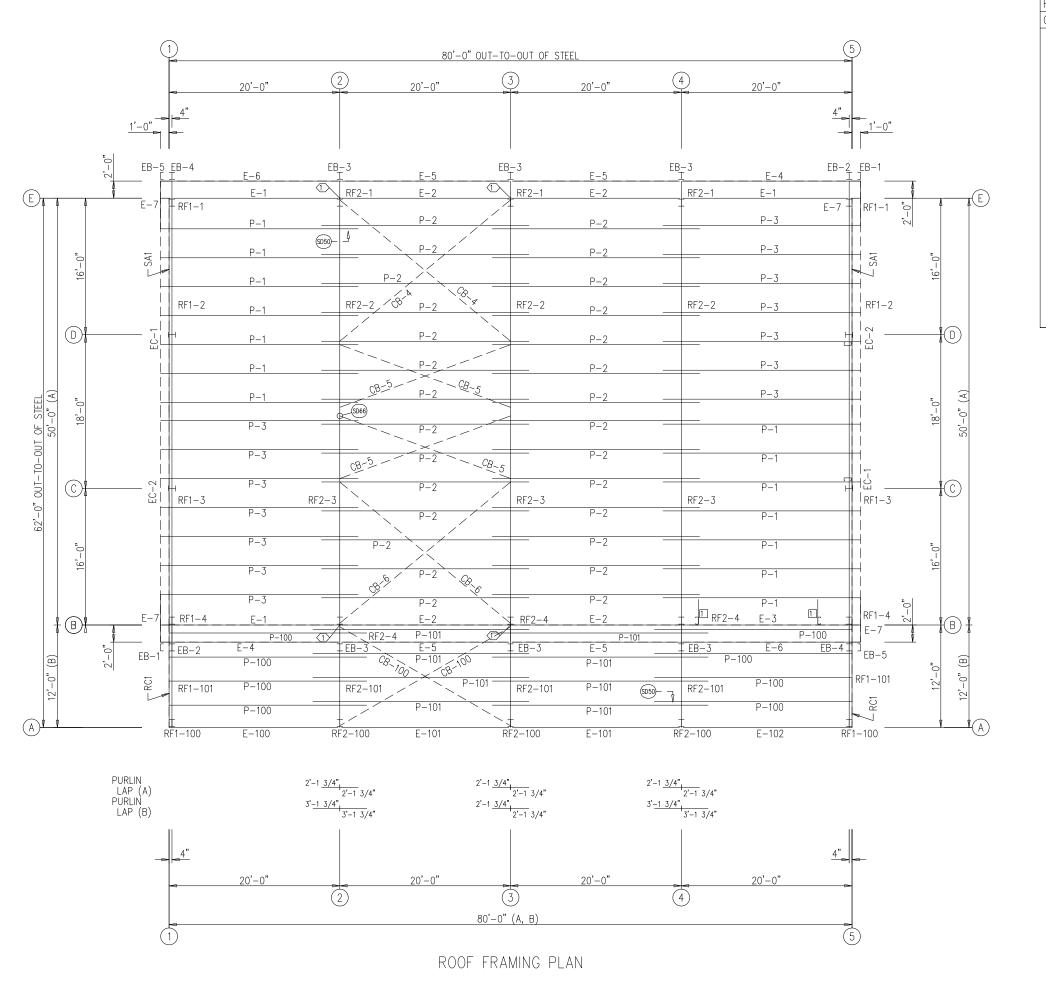
SIDEWALL SHEETING & TRIM: FRAME LINE A

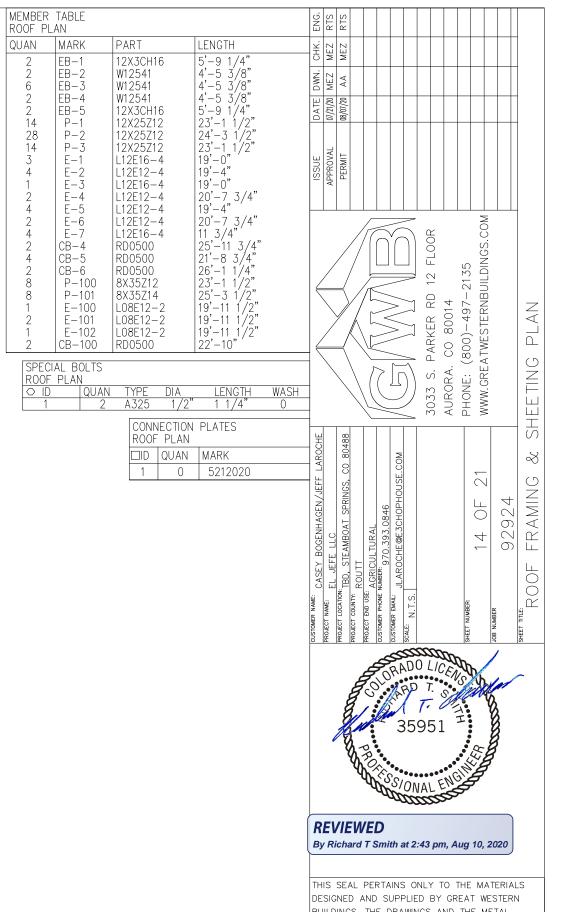
TRIM TABLE	RATS RTS S
FRAME LINE A ◇ID QUAN PART LENGTH DETAIL	
1 7 FL-214 11'-7" TD21 2 7 FL-80 11'-7" TD21	DWN. CHK. AAA MEZ AAA MEZ
MEMBER TABLE FRAME LINE A	0/12/20 MI 80/01/20 MI 80/01/2
QUAN MARK PART LENGTH	
1 E-100 L08E12-2 19'-11 1/2" 2 E-101 L08E12-2 19'-11 1/2" 1 E-102 L08E12-2 19'-11 1/2"	ISSUE APPROVAL PERMIT
1 2 102 200212 2 10 11 1/2	
	Σ ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο
	3033 S. PARKER RD 12 FLOOR AURORA. CO 80014 PHONE: (800)-497-2135 www.greatwesternbuildings.com
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	33 S. PARKER RI JRORA. CO 80014 HONE: (800)-497. W.GREATWESTERNI SHEETING
	S. S. B. B. C.
	3033 S. AURORA. PHONE: WWW.GRE
	880488 NG & Y
	CASEY BOGENHAGEN/JEFF LAROCHE EL JEFE LLC TROD. TI AGRICULTURAL AGRICULTURAL JUAROCHE©E 3CHOPHOUSE.COM  13 OF 21  92924  DEWALL FRAMING
	CASEY BOGENHAGEN/JEFF LARGEL JEFE LLC TBD. STEAMBOAT SPRINGS, CO 80 ROUTT AGRICULTURAL AGRICULTURAL JUAROCHE@E3CHOPHOUSE.COM 13 OF 21 92924  92924  DEWALL FRAMIN
	Y BOGENHAGENZ FFE LLC STEAMBOAT SPRII T T SULTURAL 970.393.0846 OCHE©E3CHOPH 13 OF 92924 WALL FR
	CASEY BOGENHA EL JEFE LLC TBD, STEAMBOAT ROUTT AGRICULTURAL WMER: 970.393.06 JLAROCHE©E.3C1 929 929
	OUSTOWER NAME: PROJECT LOCATION PROJECT COUNTY: PROJECT END USE: CUSTOMER PHONE T CUSTOMER EMAIL: SCALE: N.T.S. JOB NUMBER: SHEET TITLE: SHEET TITLE
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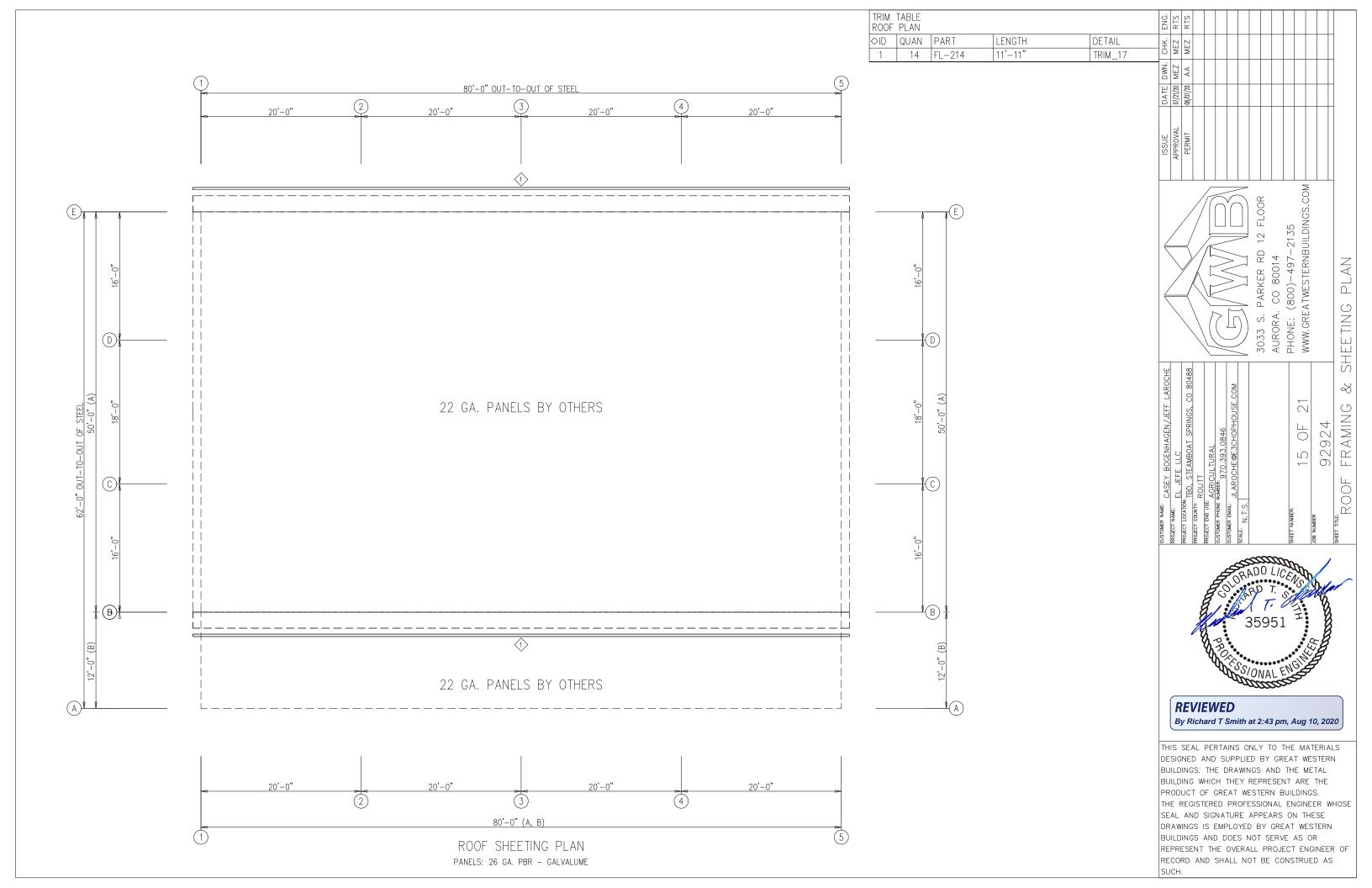
By Richard T Smith at 2:43 pm, Aug 10, 2020

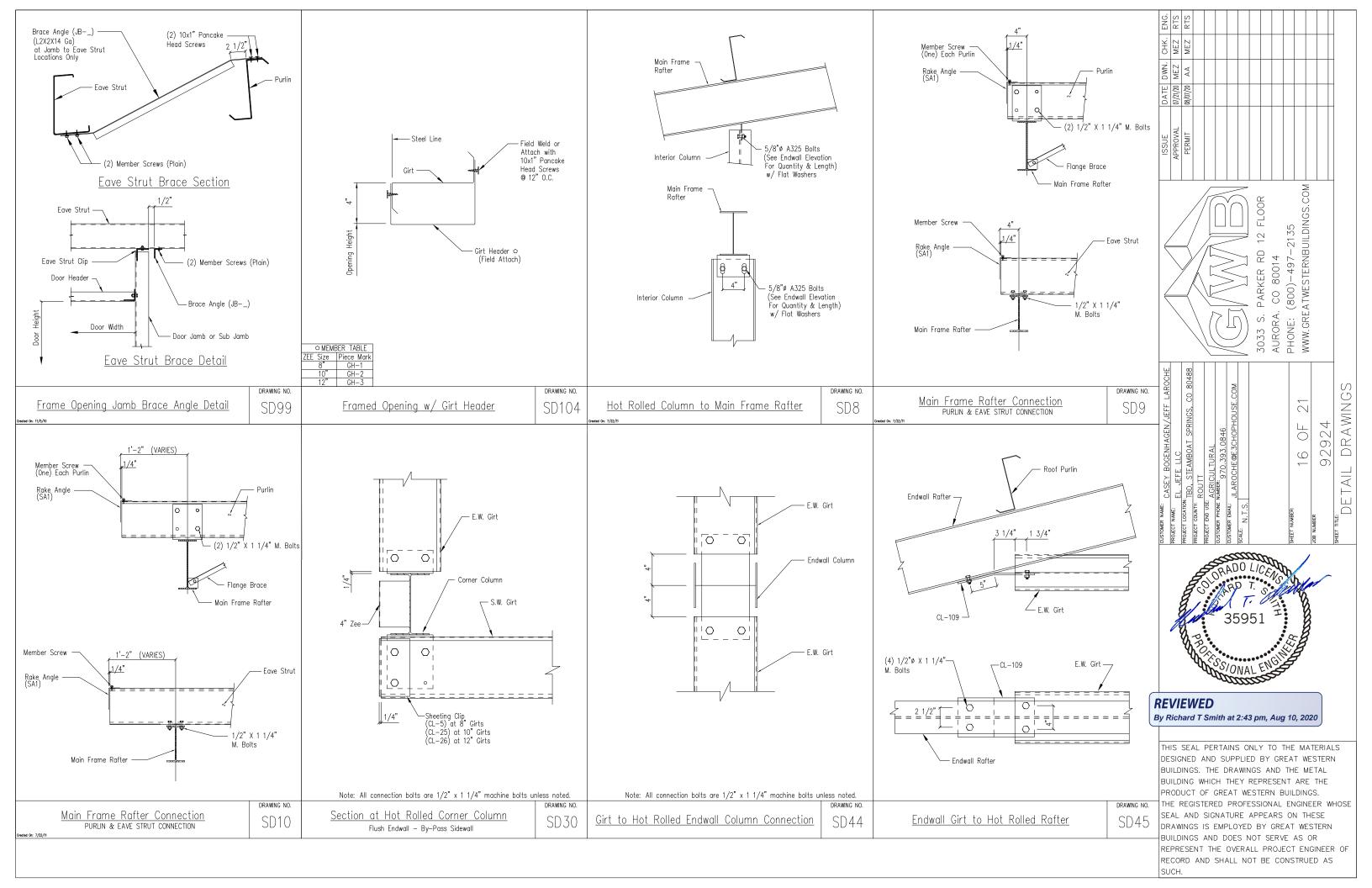
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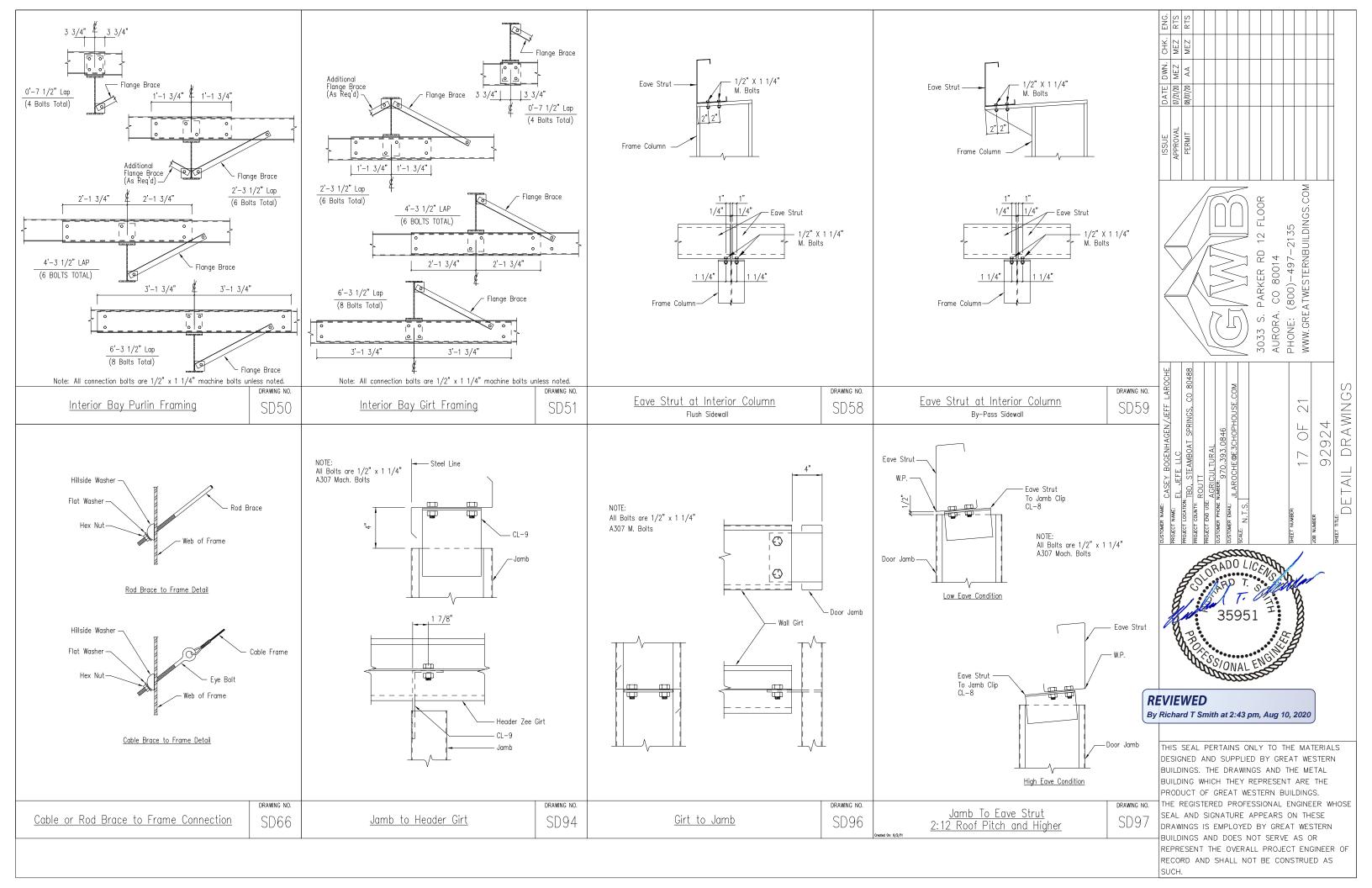


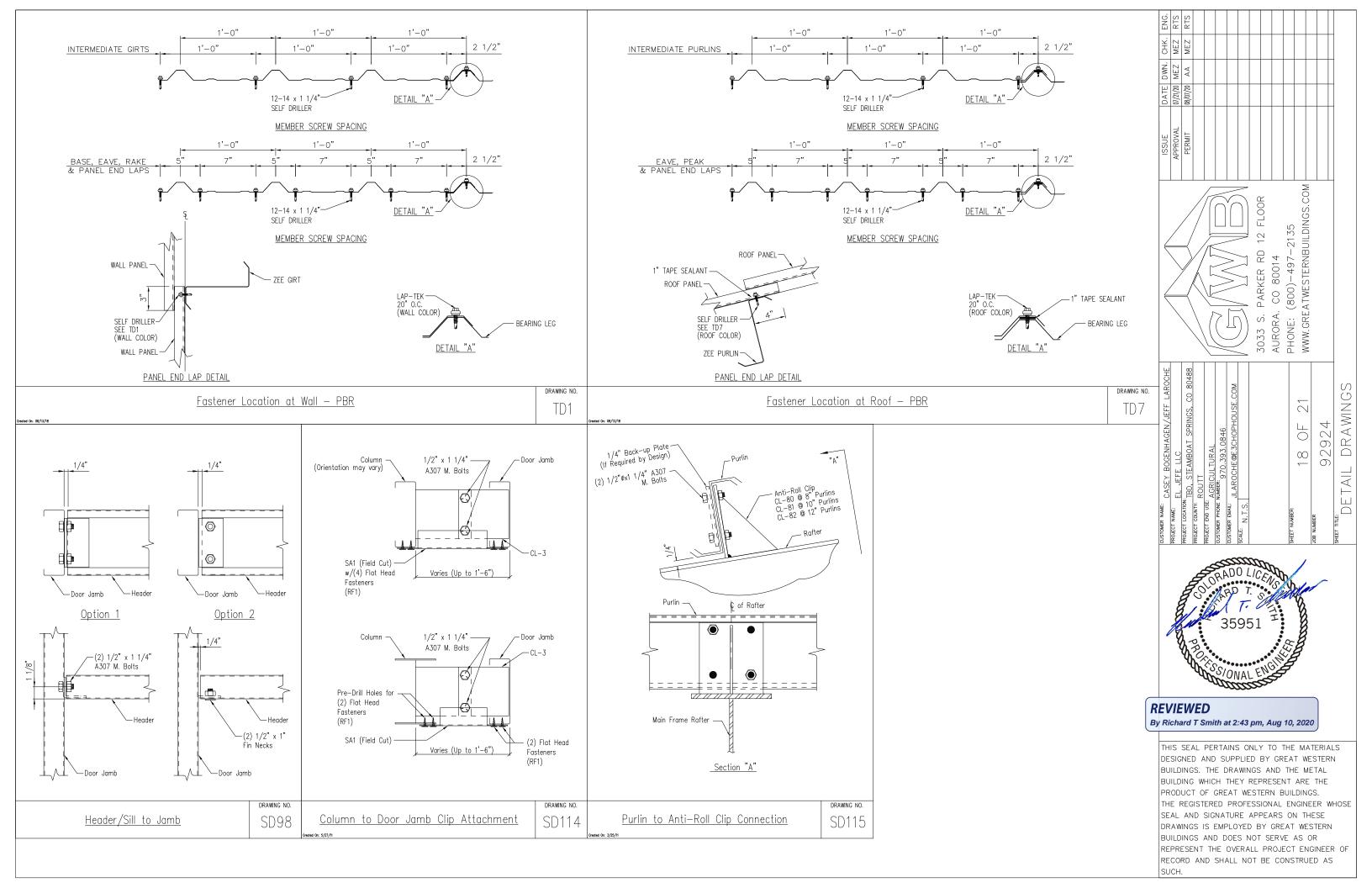


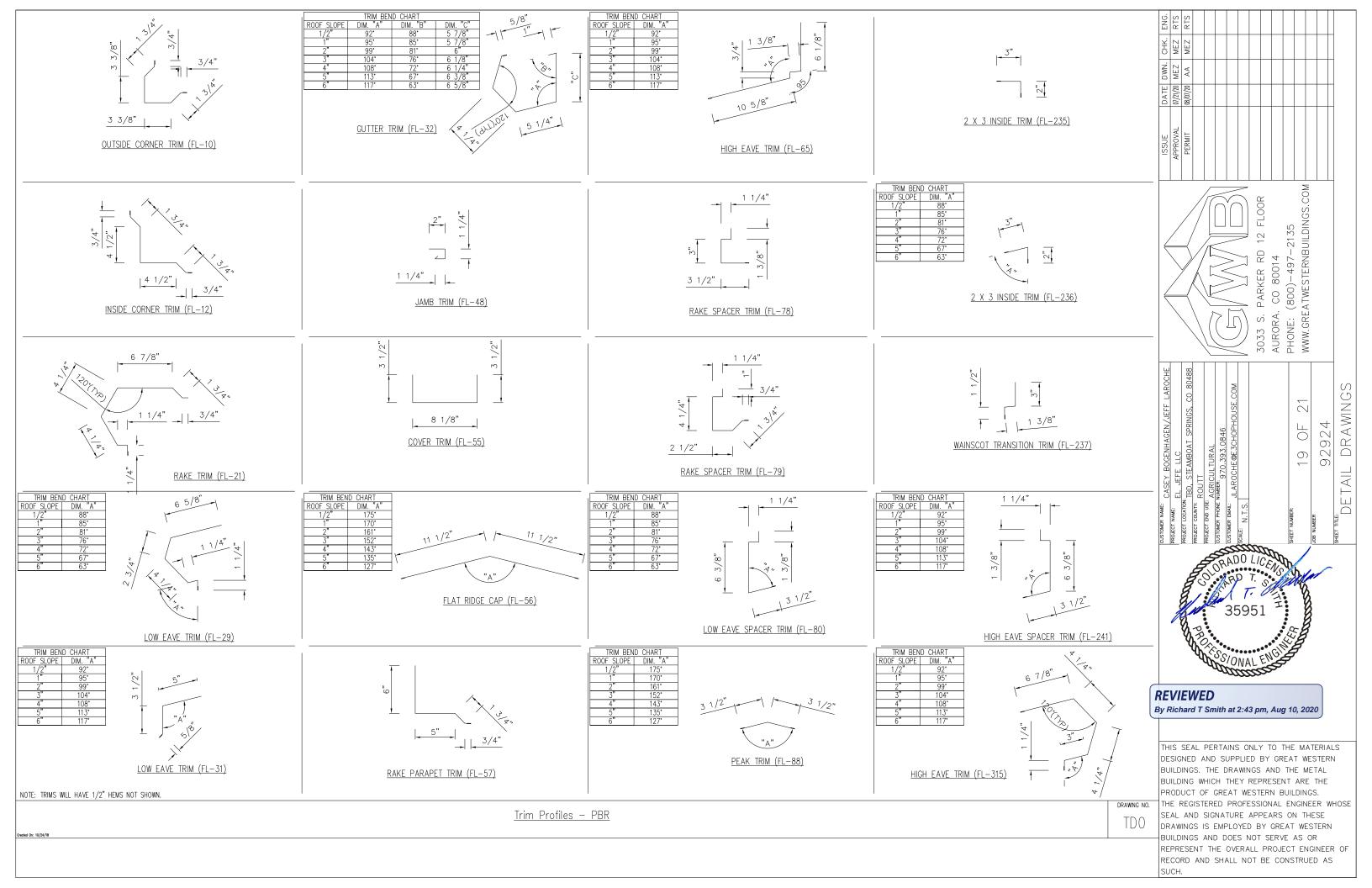
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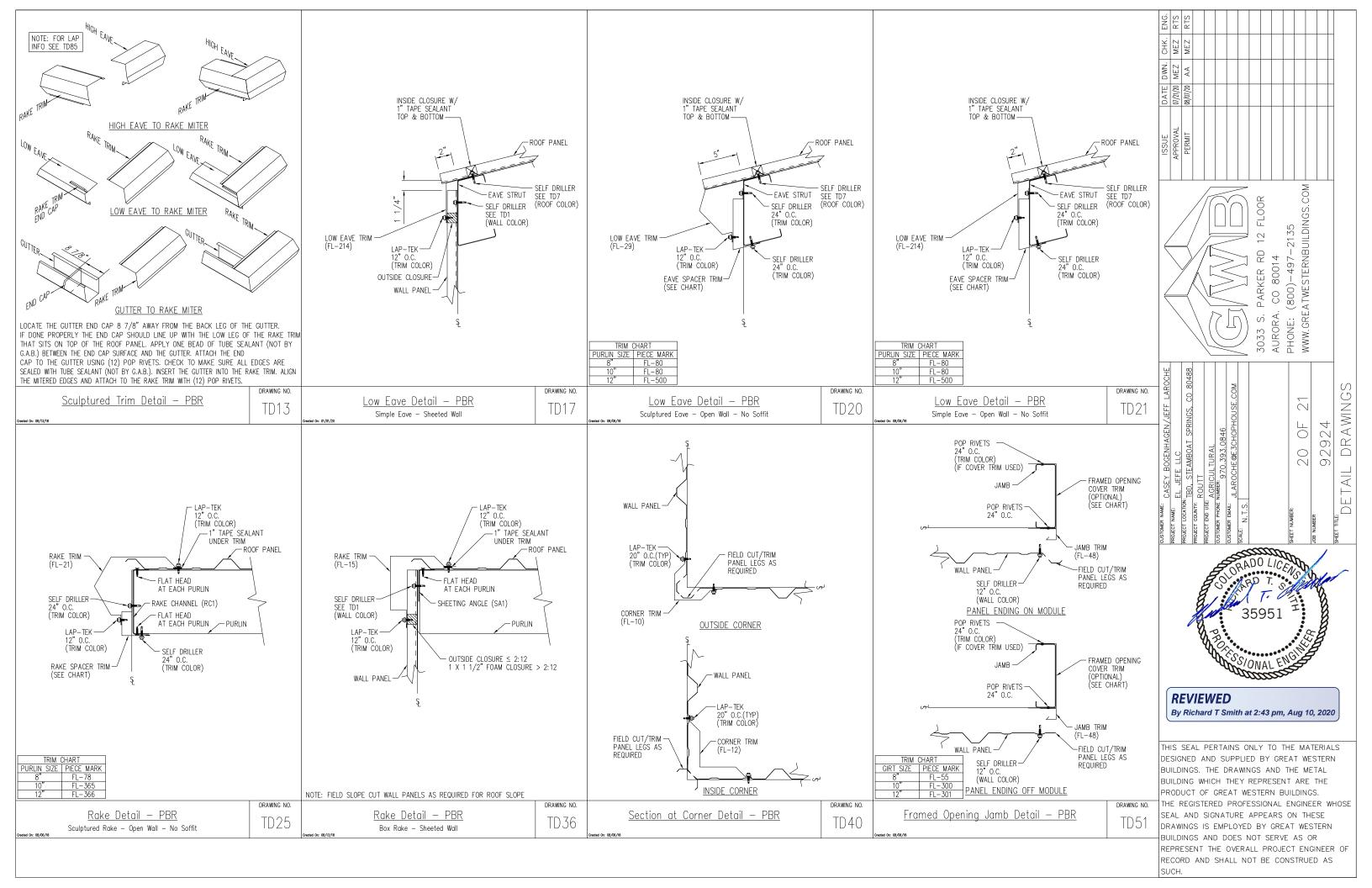


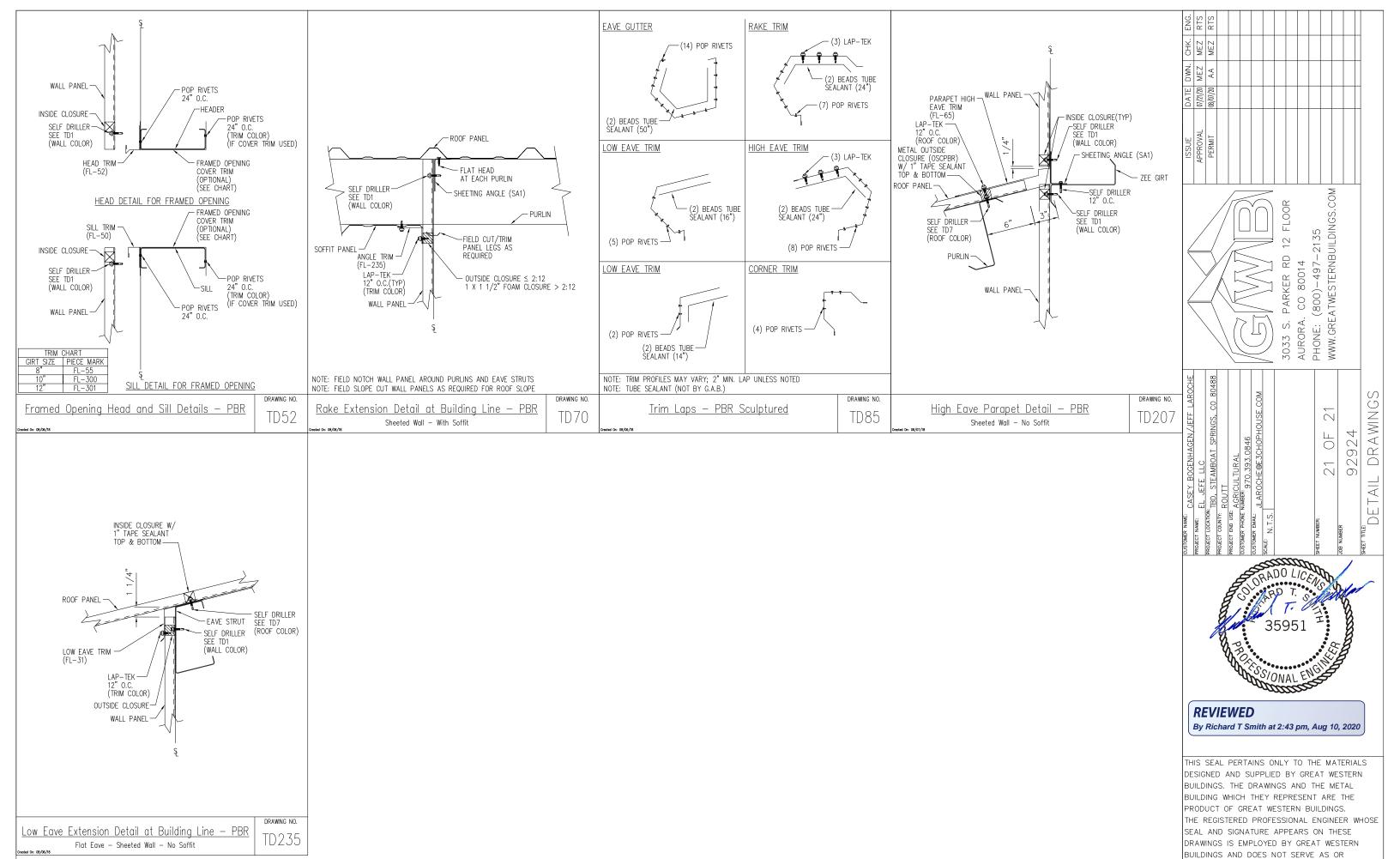












REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS