ERECTION NOTES

- 1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303-16; Section 7.10).
- 2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
- 3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16; Section 7.14).
- 4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
- 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code Steel or AWS D1.3 Structural Welding Code — Sheet as applicable, for the processes, positions, and materials involved.
- All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not pregualified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency. 6. All documentation and records shall be the responsibility of the customer.
- 7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
- 8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
- 9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16, Section 7.15). 10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
- WARNING SOME PANÉLS AND TRIM PARTS ARE FURNISHÉD WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION, FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

- 1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
- 2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
- 3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
- 4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
- 5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing, it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
- 6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3).
- 7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
- 8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
- 9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance
- 10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

- 1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
- Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefor is not a cause for rejection.
- The Metal Building Provider's red-oxide and gray-oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water-holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de-icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
- 4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
- 5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
- 6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the
- 7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket

BUILDING DESIGN CODES Building Code:	IBC 18
Hot-rolled version:	AISC 360-16
Cold-formed version:	AISI S100-16
GENERAL LOADS	
Dead Load:	3.10_ psf
Roof Collateral Load:	1.00_ psf (Misc.)
Sprinkler Load:	0.00_ psf
Roof Live Load:	20.00_ psf
Tributary Live Load Reduction:	NO
Rainfall Intensity:	4.00 in/hr (5-minute duration 5-year recurrence)
WIND LOAD	5 yeur recurrence)
Wind Load (3—sec gust) Vult:	115_ mph
Vasd:	89 mph
V service:	77 mph
Exposure Factor:	C
Wind Condition:	Enclosed
Internal Pressure Coefficient :	+/- 0.18
Edge Zone Width:	6.50 Ft
SNOW LOAD	
Ground Snow Load :	80.00 psf
Roof Snow Load :	60.00 psf
Importance Factor:	1.00
Exposure Factor:	1.00
Thermal Factor:	1.00
Slope Factor:	1.00
DEFLECTION CRITERIA	
Main Frames Horizontal: H/60	Roof Panels: <u>L/60</u>
Main Frames Vertical: <u>L/180</u>	Purlins: <u>L/180</u>
Bearing Frame Rafter: <u>L/180</u> Endwall Columns: L/120	Wall Panels: <u>L/60</u> Girts: L/90
Wind Frame Horizontal : H/60	<u>L/ 30</u>
	laskings involving wind one based on
For components, claddings and MWFRS, defl 10 year serviceability wind pressures.	lections involving wind are based on
SEISMIC LOAD	
Risk Category:	II — Normal
Seismic Importance Factor :	1.0000
Structural Response Acceleration (Ss):	0.4190
Structural Response Acceleration (33):	0.0862
Site Class:	<u></u>
Design Spectral Response (Sds):	0.4092
Design Spectral Response (Sd1):	0.1376
Seismic Design Category:	<u>C</u>
Framing Direction:	Lateral Longitudinal
Structural Syst:	'Structural Steel Systems Not Specif
· · · · · · · · · · · · · · · · · · ·	Detailed for Seismic Resistance'
Response Modification Factor(s) :	3.03.0
Deflection Amplification:	3.0
Sesimic Response Coefficient(s) (Cs):	0.1365 0.1365
Design Base Shear V :	14.14 (kips)14.12 (kips)
	Equivalent Lateral Force
Analysis Procedure:	
•	
ROOF PANEL	· 26 Colors SMD Poval Plus
ROOF PANEL Profile: Super Span X Gauge	: <u>26</u> Color: <u>SMP Royal Blue</u>
ROOF PANEL Profile: Super Span X Gauge UL580 Class 90: Yes	: <u>26</u> Color: <u>SMP Royal Blue</u>
ROOF PANEL Profile: Super Span X Gauge UL580 Class 90: Yes Clip Type if Standing Seam: NO	: <u>26</u> Color: <u>SMP Royal Blue</u>
ROOF PANEL Profile: Super Span X Gauge UL580 Class 90: Yes Clip Type if Standing Seam: NO WALL PANEL	: <u>26</u> Color: <u>SMP Royal Blue</u> : <u>26</u> Color: <u>SMP Steel Gray</u>

Profile: Super Span X Gauge: <u>26</u> Color: <u>SMP Steel Gray</u>

PRIMARY FRAMING

Built-Up & Hot-Rolled: Gray Oxide Primer

SECONDARY FRAMING

These drawings, being for approval, are by definition not final and are

proper interpretation of the project documents. Only drawings issued "F

for conceptual representation only. Their purpose is to confirm the

These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

Erector Installation" can be considered complete.

FOR APPROVAL:

FOR CONSTRUCTION PERMIT:

FOR ERECTOR INSTALLATION:
Final drawings for construction

Purlins, Eave Struts: Pre-Galvanized Girts, Light Gage Columns: Pre-Galvanized Light Gage Jambs & Headers: Pre-Galvanized Base Angle Finish: Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification. Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-90 specification.

APPROVAL SPECIFICATIONS

- 1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
- 2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
- 3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, enaineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping
- 4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
- 5. It is imperative that any changes to these drawings:
- 5.1. Be made in contrasting ink. 5.2. Be legible and unambiguous.
- 5.3. Have all instances of changes clearly indicated.
- 6. A dated signature, in the designated greas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
- 7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
- 8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
- 9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING SCHEDULE

DWG NO.	ISSUE	DATE	DESCRIPTION
C1	0	08.02.23	COVER SHEET
F1	1	07.20.23	ANCHOR BOLT PLAN
F2	1	07.20.23	ANCHOR BOLT DETAILS
F3	1	07.20.23	ANCHOR BOLT REACTIONS
F4	1	07.20.23	ANCHOR BOLT REACTIONS
P1	0	08.02.23	RIGID FRAME ELEVATION
E1	0	08.02.23	ROOF FRAMING PLAN
E2	0	08.02.23	ROOF SHEETING PLAN
E3	0	08.02.23	ENDWALL FRAME & SHEETING ELEVATION
E4	0	08.02.23	ENDWALL FRAME & SHEETING ELEVATION
E5	0	08.02.23	SIDEWALL FRAME & SHEETING ELEVATION
E6	0	08.02.23	SIDEWALL FRAME & SHEETING ELEVATION
E7	0	08.02.23	PARTITION FRAME & SHEETING ELEVATION
E8	0	08.02.23	BUILDING SECTIONS
E9	0	08.02.23	BUILDING SECTIONS
D1	0	08.02.23	STANDARD DETAILS PAGE
D2	0	08.02.23	STANDARD DETAILS PAGE
D3	0	08.02.23	STANDARD DETAILS PAGE
D4	0	08.02.23	STANDARD DETAILS PAGE

TRIM COLOR:		
Shadow Rake:	SMP Royal Blue	GAUGE: 26_
	SMP Royal Blue	GAUGE: 26
CORNER:	SMP Royal Blue	GAUGE: 26
ACCESSORY:	SMP Royal Blue	GAUGE: 26
SOFFIT TRIM:	SMP Royal Blue	GAUGE: <u>26</u>
PARTITION TRIM:	SMP Royal Blue	GAUGE: <u>26</u>
BASE Trim:	SMP Royal Blue	GAUGE: <u>26</u>

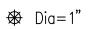
The Engineer whose seal and signature appear on these documents represents Whirlwind Stee Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, I

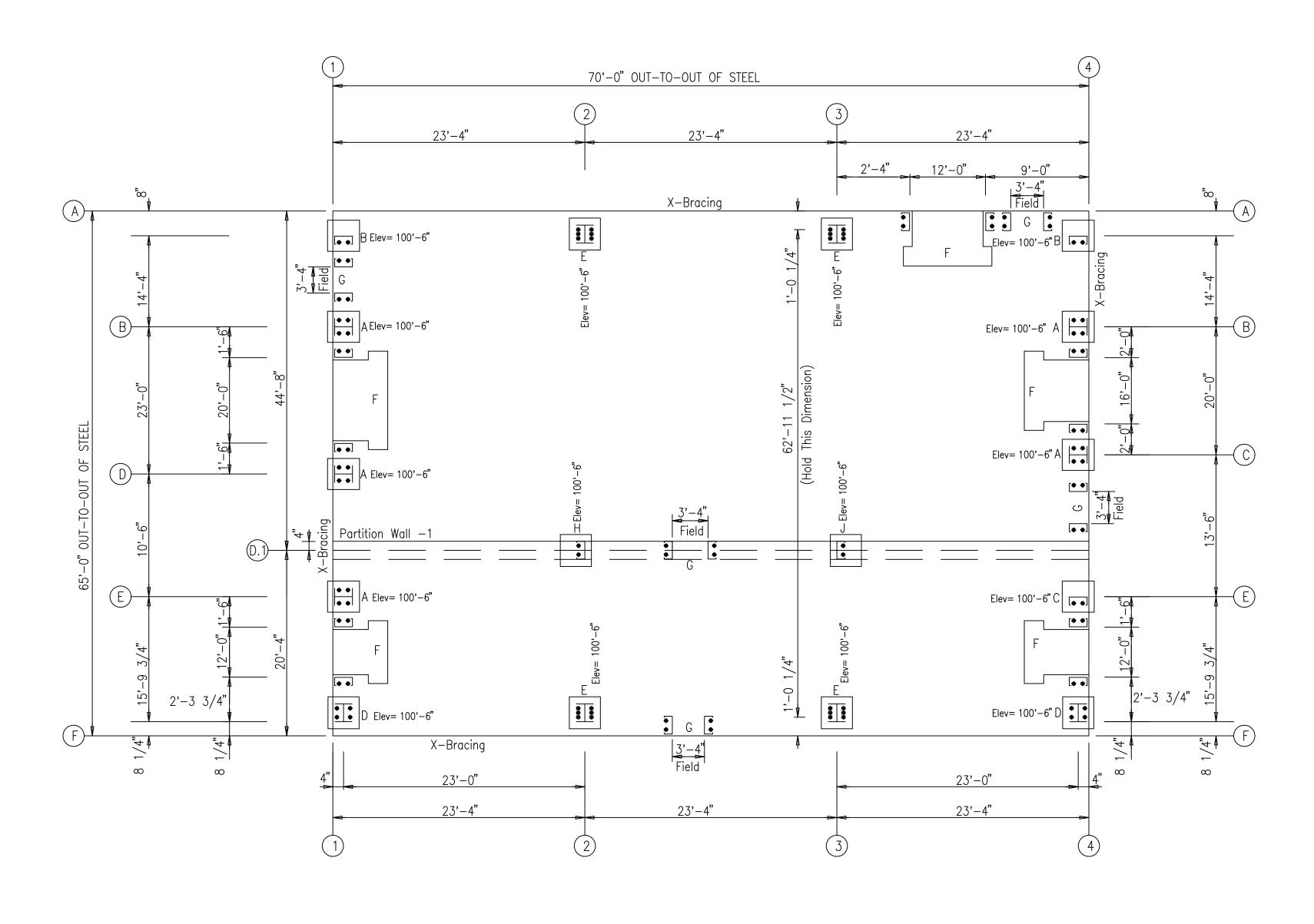
08.02.23 KMO 11217-32005

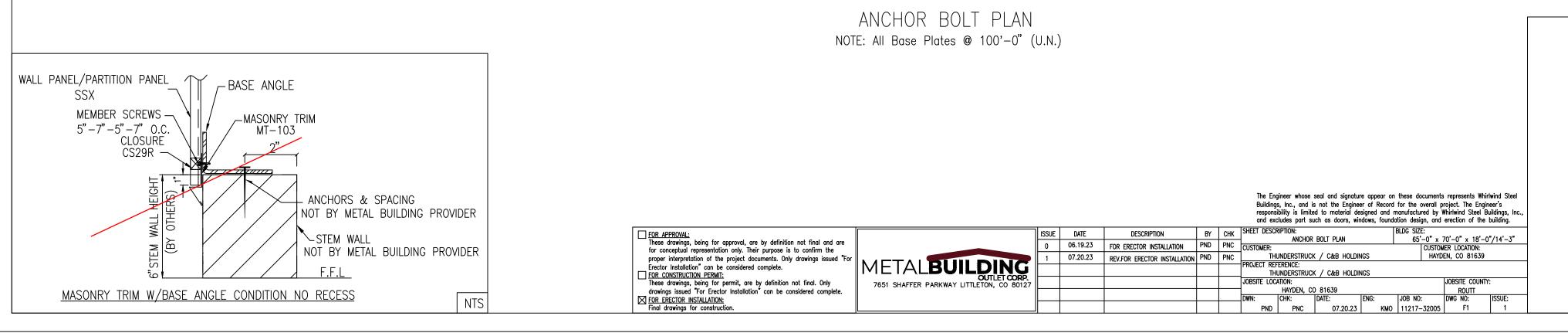
C1

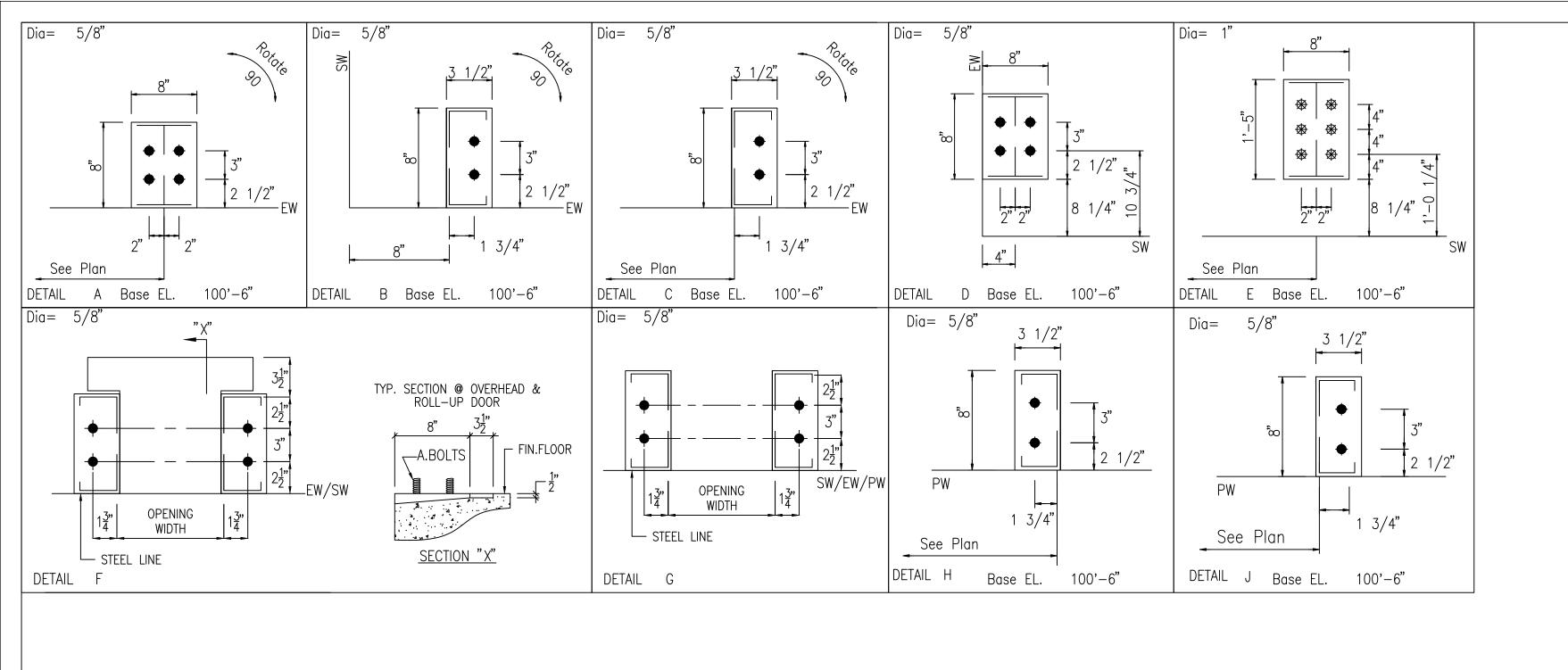
PNC

and excludes part such as doors, windows, foundation design, and erection of the building. SHEET DESCRIPTION: DATE DESCRIPTION 65'-0" x 70'-0" x 18'-0"/14'-3" 06.19.23 FOR CONSTRUCTION PERMIT PND PNC CUSTOMER: THUNDERSTRUCK / C&B HOLDINGS HAYDEN, CO 81639 P2 07.20.23 REV.FOR CONSTRUCTION PERMIT PND PNC METALBUILDING PROJECT REFERENCE: 08.02.23 FOR ERECTOR INSTALLATION PND PNC THUNDERSTRUCK / C&B HOLDINGS 7651 SHAFFER PARKWAY LITTLETON, CO 8012 HAYDEN, CO 81639 LIOR NO:









The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For

proper interpretation or the project documents. Unly drawings issued in Erector Installation" can be considered complete.

FOR CONSTRUCTION PERMIT:

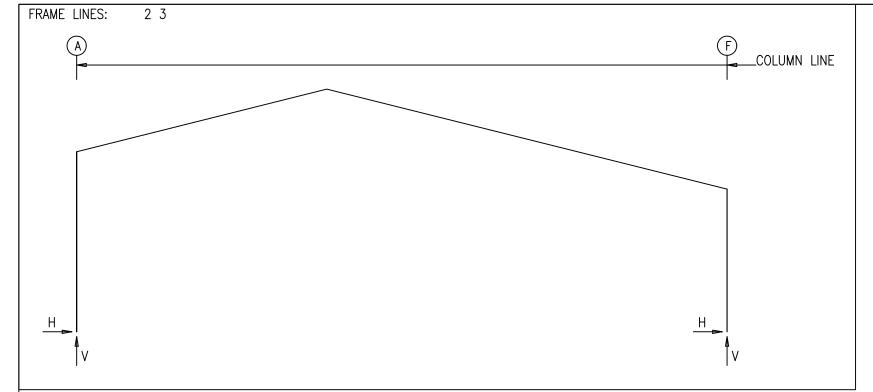
These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION:

Final drawings for construction.

	ISSUE	DATE	DESCRIPTION	
	0	06.19.23	FOR ERECTOR INSTALLATION	PI
ACTAL BUILDING	1	07.20.23	REV.FOR ERECTOR INSTALLATION	PI
METALBUILDING				
7651 SHAFFER PARKWAY LITTLETON, CO 80127				

								··· · · · · · · · · · · · · · · · ·		
CHK	SHEET DESC	RIPTION:	-		BLE	OG SIZE:	•	-		
		ANCHOR	BOLT DETAILS			65'-0" x 7	70'-0" x 18	'-0"/14'-3"		
PNC	CUSTOMER:	THUNDERSTRUCK / C&B HOLDIN OJECT REFERENCE: THUNDERSTRUCK / C&B HOLDIN BSITE LOCATION: HAYDEN, CO 81639			-	CUSTOMER LOCATION:				
PNC	TH	UNDERSTRUCK	/ C&B HOLDIN	NGS		HAYDI	EN, CO 8163	59		
	PROJECT REI	ERENCE:				•				
	TH	ANCHOR BOLT DETAILS 65'-0" x 70'-0" x 18'-0"/14'-3" STOMER: THUNDERSTRUCK / C&B HOLDINGS NECT REFERENCE: THUNDERSTRUCK / C&B HOLDINGS SITE LOCATION: HAYDEN, CO 81639 HAYDEN, CO 81639 ROUTT HEYDEN, CO 81639 ROUTT SITE COUNTY: ROUTT HOUSE JOB NO: DWG NO: ISSUE:								
	JOBSITE LOC	ATION:	•				JOBSITE COU	NTY:		
	i	HAYDEN, CO 81639 ROUTT								
	DWN:	CHK:	DATE:	ENG:	J	OB NO:	DWG NO:	ISSUE:		
	PND	PNC	07.20.23	KN	10 1	1217-32005	F2	1		
	CHK PNC PNC	PNC CUSTOMER: PNC THI PROJECT REF JOBSITE LOC DWN:	PNC CUSTOMER: THUNDERSTRUCK PROJECT REFERENCE: THUNDERSTRUCK JOBSITE LOCATION: HAYDEN, CO DWN: CHK:	PNC CUSTOMER: THUNDERSTRUCK / C&B HOLDIN PROJECT REFERENCE: THUNDERSTRUCK / C&B HOLDIN JOBSITE LOCATION: HAYDEN, CO 81639 DWN: CHK: DATE:	PNC CUSTOMER: PNC THUNDERSTRUCK / C&B HOLDINGS PROJECT REFERENCE: THUNDERSTRUCK / C&B HOLDINGS JOBSITE LOCATION: HAYDEN, CO 81639 DWN: CHK: DATE: ENG:	PNC CUSTOMER: THUNDERSTRUCK / C&B HOLDINGS PROJECT REFERENCE: THUNDERSTRUCK / C&B HOLDINGS JOBSITE LOCATION: HAYDEN, CO 81639 DWN: CHK: DATE: ENG: J	PNC CUSTOMER: CUSTOM PNC THUNDERSTRUCK / C&B HOLDINGS HAYD PROJECT REFERENCE: THUNDERSTRUCK / C&B HOLDINGS JOBSITE LOCATION: HAYDEN, CO 81639 DWN: CHK: DATE: ENG: JOB NO:	ANCHOR BOLT DETAILS 65'-0" x 70'-0" x 18		



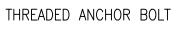
RIGID	FRAME:		MAXIMUM	REACTION	S, ANCH	OR BOLTS	, & BASE	PLATE	S				
Frm Line	Col Line	Load Id	Colu Hmax H	umn_React V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
2*	А	1	30.3	53.7	2 4	-5.7 -1.1	-8.5 -9.2	6	1.000	8.000	17.00	0.500	6.0
2*	F	3 7	5.6 -28.3	-8.9 53.6	1 5	-30.3 1.6	53.1 -9.3	6	1.000	8.000	17.00	0.500	6.0
2*	Frame lir	nes:	2 3										

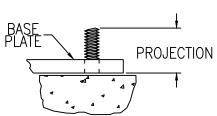
RIGID F	FRAME:	BAS	C COLUM	n reacti	ONS (k)							
Frame Co Line Lin 2* A 2* F		Dead Vert 4.4 4.2	-—-Collo Horiz 0.5 -0.5	oteral— Vert 0.8 0.8	 Horiz 9.2 -9.2	-Live Vert 16.2 16.0	 Horiz 27.7 -27.7	-Snow Vert 48.5 48.1	-—-Wind Horiz -11.7 3.4	_Left1- Vert -18.6 -12.8	-Wind_f Horiz -4.7 11.5	Right1- Vert -14.0 -19.1
Frame Co Line Lin 2* A 2* F			−Wind_ Horiz −2.7 9.5	Right2- Vert -7.0 -12.0	Wind Horiz -4.0 5.4	_Long1- Vert -19.7 -15.8	Wind Horiz -6.7 4.8	L_Long2- Vert -19.0 -19.6	-Seism Horiz -1.8 -2.5	ic_Left Vert -0.9 0.9	Seismic Horiz 1.8 2.5	_Right Vert 0.9 –0.9
Frame Co Line Lin 2* A 2* F			-MIN_S Horiz 9.2 -9.2	NOW Vert 16.1 16.0	F1UNB_ Horiz 20.0 –20.0	SL_L- Vert 48.0 22.6	F1UNB_ Horiz 25.7 –25.7	SL_R- Vert 31.3 48.6				
2* Fran	ne lines:	2 3										

GENERAL NOTES

- 1. All anchor bolts (by others) to have nuts and flat washers.
- 2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- 3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)

 4. The projection is based from the bottom of the base plate. Adjustments
- 4. The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.





NOTE: PROJECTION BASED
FROM BOTTOM OF BASE
PLATE. ADJUSTMENTS
SHOULD BE MADE FOR
GROUT AND/OR LEVELING
PLATES.

	4	4	1	1	1	1	1	Frm Line	ENDW	Frm Line 4 4 4 4 4	Frm Line 4 4 4 4	Frm Line 4 4 4 4	Frm Line 1 1 1 1	Frm Line 1 1 1 1	Frm Line 1 1 1 1	ENDW
	E	F	F	E		E	A		VALL	Col Line F E C B A	Col Line F E C B A	Col Line F E C B A	Col Line A B D E F	Col Line A B D E F	Col Line A B D E F	VALL
2	<u>.</u>			_)	3	4		COLU	E2UNB Horz 0.0 0.0 0.0 0.0 0.0	Wind Suct Horz 0.0 3.2 4.4 4.5	Dead Vert 0.7 1.0 1.2 1.4 0.5	-MIN_ Horz 0.0 0.0 0.0 0.0 0.0	Wind_F Horz 0.0 -4.4 -3.8 -2.6 -6.4	Dead Vert 0.5 1.5 1.3 0.9	COLU
		15	13			9				Vert 8.2 12.6 18.1 7.6 1.4	Wind Horz 0.0 0.0 0.0 0.0 2.4	Colle Vert 0.1 0.2 0.2 0.3 0.1	SNOW Vert 2.0 6.1 5.1 3.4 2.7	Press Vert 0.0 0.0 0.0 0.0 -3.2	Colle Vert 0.1 0.3 0.3 0.2 0.1	MN:
0.0	1.9 0.0 2.6	0.0	0.0 0.0	1.7 0.0	2.5 0.0	2.9 0.0	0.0 0.0	— Colu nax H		E2U Hor 0.0 0.0 0.0 0.0	- - -		E1U Hor 0.0 0.0 0.0 0.0	Wi Hor 0.0 4.9 4.2 2.9 0.0		
19.5 -4.2	-4.1 14.0 -2.8	-2.1 9.0	-2.1 9.3	-5.5 11.3	-5.9 21.0	-4.3 25.3	-1.8 7.4	mn_Reactio V Vmax	MAXIMUM F	2.4 3.0 7.1 22.6	1 Wi /ert Ho 3.5 0 7.9 0 5.4 0 1.4 -0 5.3 0	Live Vert 2.6 4.3 4.8 5.5 2.1	6.8 23.5 7.4 1.9	0.0 9 0.0 2 0.0 9 0.0	Live Vert 2.0 6.1 5.1 3.4 2.7	BASIC COL
9	16	2	12	10 14	12 11	10 9	2	Loàd H	EACTIONS		.0 -2 .0 -4 .0 -3 .6 -8	Snow Vert 7.8 12.8 14.3 16.5 6.3	E1UI Horz 0.0 0.0 0.0 0.0 0.0	Wind Horz 0.0 0.0 0.0 0.6 0.0	Snow Vert 5.9 18.2 15.4 10.3 8.0	.UMN REA
2.6 -2.5	-1.7 1.9 -2.4	0.0	-4.9 -3.9	-1.6 1.7	-2.3 2.5	-2.7 2.9	0.0	— min H	, ANCHOF		ert F 2.2 4.3 5.5 	Win Hor -1.83 0.0 0.0 -1.9	NB_SL_R- 2 Ver 0.8 9.7 19.5 9.3	LLong1 - Ver -3.: -7.: -2.: -4.6	Win Hor 0.0 0.0 -2.8 0.0	CTIONS (
-2.8 -4.2	-4.1 -4.1 -2.5	-2.1	-2.1 -3.7	-2.4 -5.5	-5.9 -5.9	-3.7 -4.3	-1.8	V Vmin	R BOLTS			3 -4) -5) -5 9 -7	t 3 7 5 3	t H 1 C 7 C 7 -2 8 C) -3) -8 8 -10) 1	k)
	2	4	4	4	4	4	2	- Bo Qty	, & BAS		ft Vert 0.1 0.3 -0.4 -4.5 4.5	ert 4.2 7.5 5.8 7.6).1).0 2.4 –).0	ert 3.6 3.6).2 .9	
0.625	0.625 0.625	0.625	0.625	0.625	0.625	0.625	0.625	lt(in) Dia	SE PLATE		Seis_I Horz 0.0 0.0 0.0 0.0 2.8	Wind_Ric Horz 2.12 0.0 0.0 0.0 2.8		g2 Vert -1.7 -5.4 -11.0 -1.6 -3.6	Wind_Rig Horz 0.0 0.0 0.0 1.9 2.12	
	3.500 8.000	8.000	8.000	8.000	8.000	8.000	3.500	Ba Width —	rS		Right Vert -0.1 -0.3 0.4 3.8 -3.8	Vert		Seis_Le Horz 0.0 0.0 -2.8 0.0 0.0	Vert -2.9 -5.0 -2.9 -10.0	
8 000				8.000	8.000	8.000	8.000	se_Plate(i Length 			Seis Long Horz 0.0 0.0 0.1 0.1	Wind_Le Horz -1.83 0.0 0.0 -2.0 0.0		eft Vert 0.1 -0.2 -5.5 5.6 -0.1	Wind_Le Horz 0.0 0.0 -2.5 0.0 -10.88	
	0.25	0.37	0.37	0.37	0.37	0.37	0.250				-M Ho: 0.0 0.0 0.0 0.0	ft2 Vert -3.0 -5.1 -4.3 -5.7 1.5		Seis_Rig Horz 0.0 0.0 0.0 2.8 0.0	ft2 Vert -2.5 -6.2 -7.8 3.3 -1.9	
					5 6.	5 6.	0 6.	Ele (in			0 2 0 4 0 4 0 5	Wind_R Horz 2.12 0.0 0.0 0.0 2.5		yht Vert -0.1 0.2 4.8 -4.9 0.1	Wind_R Horz 0.0 0.0 0.0 2.0 2.12	
0	.0	.0	.0	.0	.0	.0	.0				7—— ert .6 .3 .8 .5	Right2 Vert -1.8 -2.5 -2.2 -2.2 -6.2		Seis_ Horz 0.0 0.1 0.1 0.0 -7.0	Vert Vert -1.7 -2.8 -1.0 -8.0 -3.0	
												Wind Press Horz 0.0 -2.9 -4.0 -4.1 0.0		Long Vert 0.0 0.0 0.0 0.0 -3.5		

BUILDING BRACING REACTIONS Panel_Shear ± Reactions(k) (lb/ft) — Wind — Seismic — Horz Vert Horz Vert Wind Seis Line Line __EW 5.0 D,E 2.8 2.8 4.9 3.2 3.8 3.5 3.8 F_SW 1,2 6.5 7.0 2.8 R_EW B,A 3,2 2.8 B_SW 7.2 4.8 7.1 4.7 Reactions for seismic represent shear force, Eh

ANCHO	R BOLT	SUMM	ARY ((GRADE	36)
Qty	Locate	_	Dia (in)	Туре	Proj (in)
◆ 36 ◆ 34 ♦ 24 ◆ 4 ◆ 4	Jamb Endwall Frame Partition C Partition Jo		5/8" 5/8" 1" 5/8" 5/8"	F1554 F1554 F1554 F1554 F1554	2.50 2.50 3.50 2.50 2.50

Partition column	reactio	ns	
Frame Column Line	Dead Load	Wind Press Horz.	Wind Suct Horz.
2 H	0.64	-3.43	3.43
3 J	0.64	-3.43	3.43

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В

4

FOR ERECTOR INSTALLATION:

Final drawings for construction.

2.7 0.0

0.0

0.0

17

3 17 12 11

3

-4.2

24.2

-4.3

7.7

-2.5 2.7

0.0

-4.2

-4.2

-4.3



	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET
	0	06.19.23	FOR ERECTOR INSTALLATION	PND	PNC	CUSTO
	1	07.20.23	REV.FOR ERECTOR INSTALLATION	PND	PNC	
G						PROJE
P. 0127						JOBSI'
						DWN:
						Dilli.

4 0.625 8.000 8.000 0.375

2 0.625 3.500 8.000 0.250

6.0

14 13 14

	10	9	10	<u>) </u>
6				,
5	2	1	2	
7	4	3	4	-
7	4	3	4	-
8	2	1	2	
미				

13

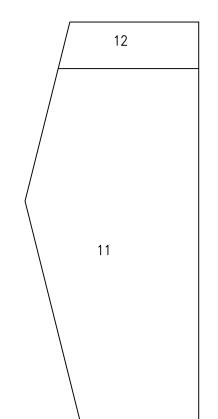
12

11

12

14

			5 5 p 5 5 to		· 3	
Zone	Width (ft)	Length (ft)	Pressure(psf Member) Panel	Suction(psf Member) Panel
1 2 3 4 5 6 7 8 9 10 11 12 13	6.50 6.50 14.83 4.44 6.50 30.29 2.06 2.06 6.50	4.50 4.50 2.00 2.00 2.00 2.00 61.00 6.50	16.00 16.00 16.00 16.00 16.00 16.00 16.00 16.00 24.65 24.70 24.70	18.56 18.56 18.56 18.56 18.56 18.56 18.56 16.00 16.00 30.61 30.60 30.60	-17.64 -34.22 -34.22 -51.33 -82.38 -91.90 -103.2 -82.38 -38.92 -43.59 -27.25 -29.16 -27.20 -29.10	-56.56 -82.58 -82.58 -97.85 -91.06 -106.3 -122.2 -91.06 -64.86 -106.4 -33.21 -40.85 -33.20 -40.84
				ards surf		



12

14

Components & Cladding

NOTES FOR REACTIONS

opposite directions.

Width (ft)
Length (ft)
Eave Height (ft)
Roof Slope (rise/12)
Dead Load (psf)
Collateral Load (psf)

Live Load (psf Snow Load (psf) Wind Speed (mph)

Importance Wind

Seismic Zone

5. Loading conditions are:

Importance Seismic

Seismic Coeff (Fa*Ss)

0.6Dead+0.6Wind_Left1

0.6Dead+0.6Wind_Right1 0.6Dead+0.6Wind_Long1L 0.6Dead+0.6Wind_Long2L

0.54Dead+0.7Seismic_LongL Dead+Collateral+F1UNB_SL_R

Dead+Collateral+E1UNB_SL_L 0.6Dead+0.6Wind_Left1+0.6Wind_Suction

10 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L 11 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L 12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L

13 Dead+Collateral+E1UNB_SL_R
14 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
15 Dead+Collateral+E2UNB_SL_L

16 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L 17 Dead+Collateral+E2UNB_SL_R

Dead+Collateral+Snow+Slide_Snow

Wind Code

Exposure

Closure

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.

= 65.0

= 1.0 = 20.0

= 60.0 = 115.0

= C

= 1.00

= 1.00

= 0.61

= C

= IBC 18

= Enclosed

= 70.0 = 18.0/14.3 = 3.0/3.0 = 3.1

2. Positive reactions are as shown in the sketch. Foundation loads are in

3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.

4. Building reactions are based on the following building data:

(-) wind away from surface

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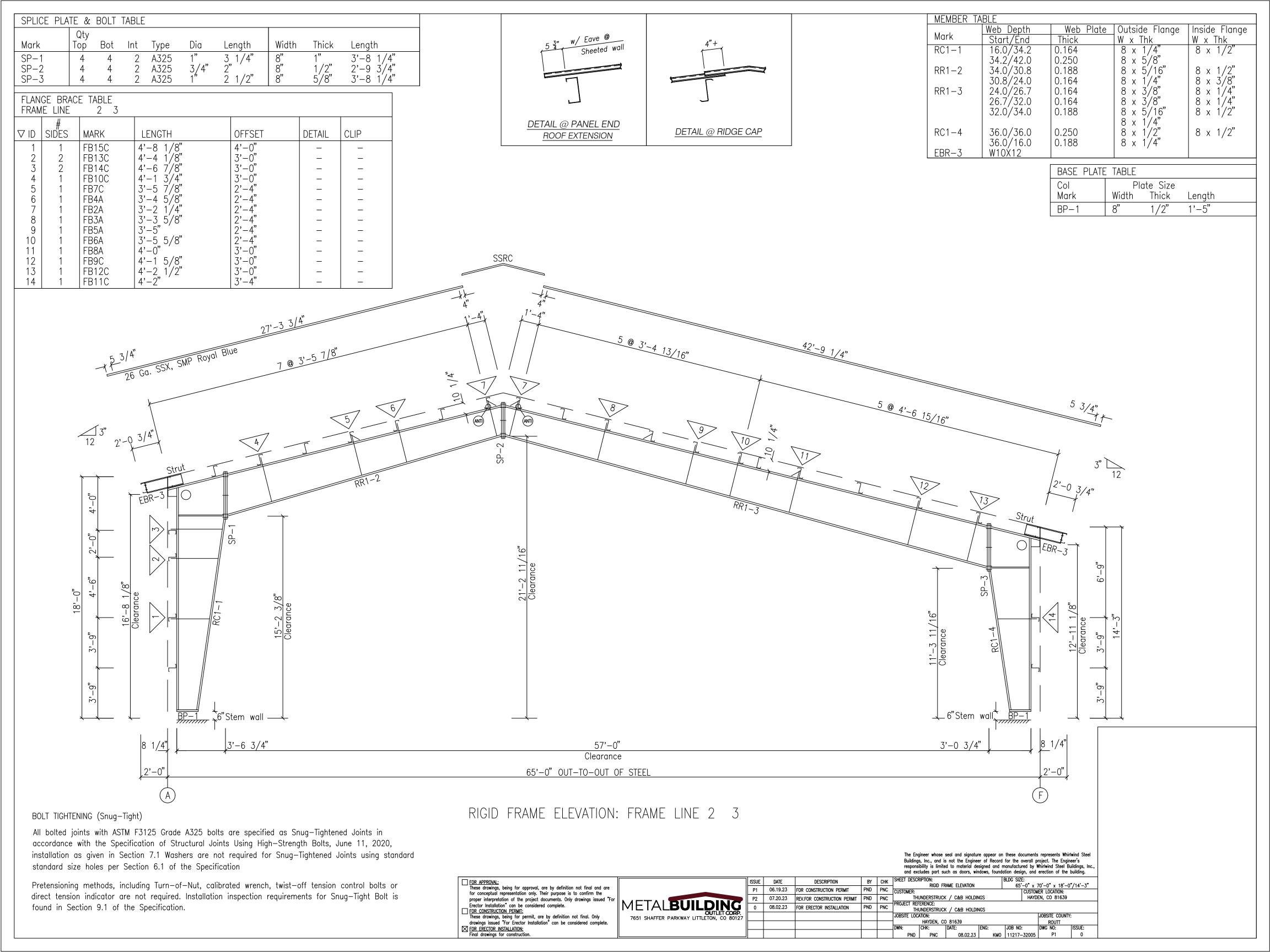
FOR APPROVAL:
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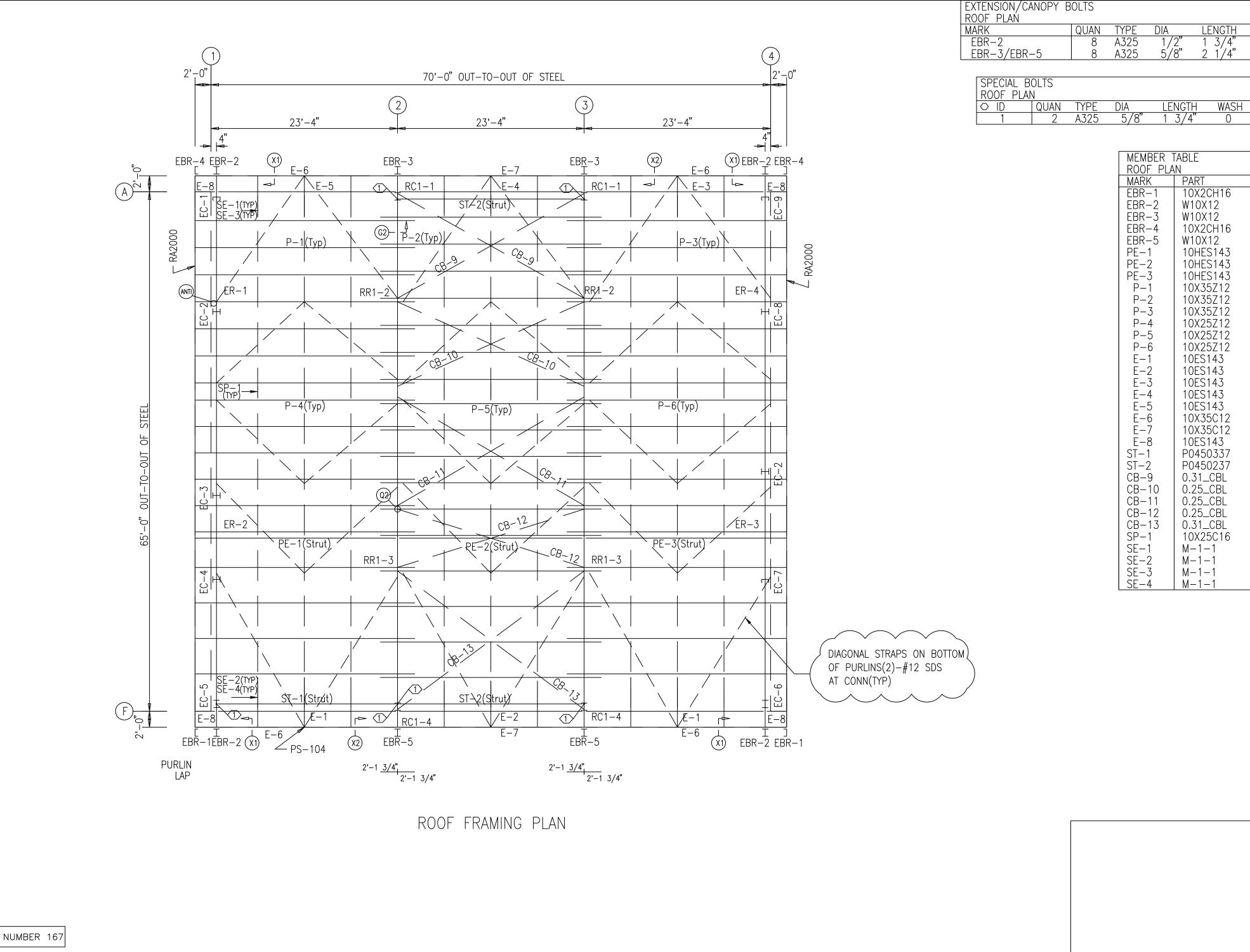
| FOR CONSTRUCTION PERMIT: | These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION:
Final drawings for construction.



							<u>*</u>	·	.		,
SSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCR		BOLT REACTIONS		BLDG SIZE:	70'–0" x 18'–0'	"/14'_3"
0	06.19.23	FOR ERECTOR INSTALLATION	PND	PNC	CUSTOMER:	Autorion	BOLI NENOHONO			IER LOCATION:	/14 3
1	07.20.23	REV.FOR ERECTOR INSTALLATION	PND	PNC		JNDERSTRUCK	C / C&B HOLDIN	NGS	HAYD	EN, CO 81639	
					PROJECT REF						
					TH	understruci	K / C&B HOLDIN	NGS			
					JOBSITE LOCA					JOBSITE COUNTY:	;
						HAYDEN, CO	D 81639			ROUTT	
					DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
					PND	PNC	07.20.23	KMO	11217-32005	F4	1





UL580, CLASS 90 CONST. NUMBER 167

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FOR APPROVAL:
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FOR CONSTRUCTION PERMIT:

These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION:

Final drawings for construction.

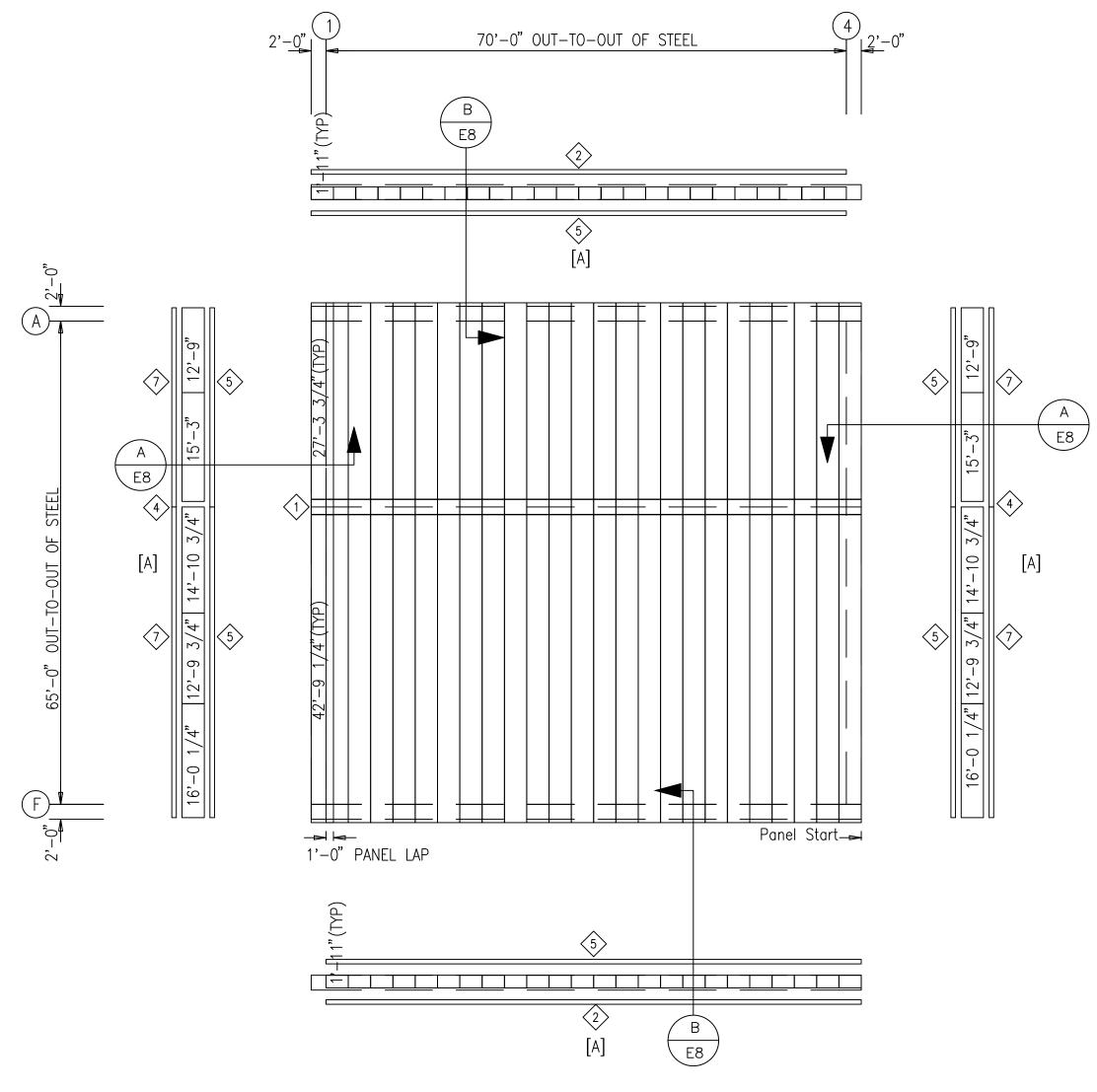
METALBUILDING
OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

| ISSUE | DATE | DESCRIPTION | BY CHK | SHEEL DESCRIPTION | P1 | O6.19.23 | FOR CONSTRUCTION PERMIT | PND | PNC | CUSTOMER: P2 07.20.23 REV.FOR CONSTRUCTION PERMIT PND PNC THUNDERSTRU
0 08.02.23 FOR ERECTOR INSTALLATION PND PNC THUNDERSTRU
THUNDERSTRU
THUNDERSTRU

BY CHK SHEET DESCRIPTION: BLDG SIZE: 65'-0" x 70'-0" x 18'-0"/14'-3"

CUSTOMER LOCATION: ROOF FRAMING PLAN THUNDERSTRUCK / C&B HOLDINGS HAYDEN, CO 81639 THUNDERSTRUCK / C&B HOLDINGS JOBSITE LOCATION: JOB NO: DWG NO:

ROOF	SHEETING	TRIM TABLE
♦ID	PART	LENGTH
1	SSRC30	3'-0"
2	SF-12	15'-3"
4	CF-110	10'-3"
5	CF-116	15'-3"
7	SF-11	15'-3"



ROOF SHEETING PLAN

PANELS: 26 Ga. SSX - SMP Royal Blue

[A] SOFFIT PANELS: 26 Ga. Reverse Rolled SSX — SMP Steel Gray

| FOR APPROVAL:

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| FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.
| FOR ERECTOR INSTALLATION: Final drawings for construction.

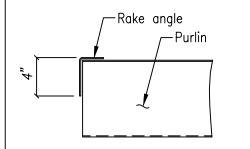
METALBUILDING
OUTLET CORT
7651 SHAFFER PARKWAY LITTLETON, CO 801:

	ISSUE	DATE	DESCRIPTION	BY	СНК	ľ
	P1	06.19.23	FOR CONSTRUCTION PERMIT	PND	PNC	ŀ
	P2	07.20.23	REV.FOR CONSTRUCTION PERMIT	PND	PNC	ļ
G	0	08.02.23	FOR ERECTOR INSTALLATION	PND	PNC	ľ
RP. 0127						ŀ
						ŀ
						1'

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65'-0" x 70'-0" x 18'-0"/14'-3"

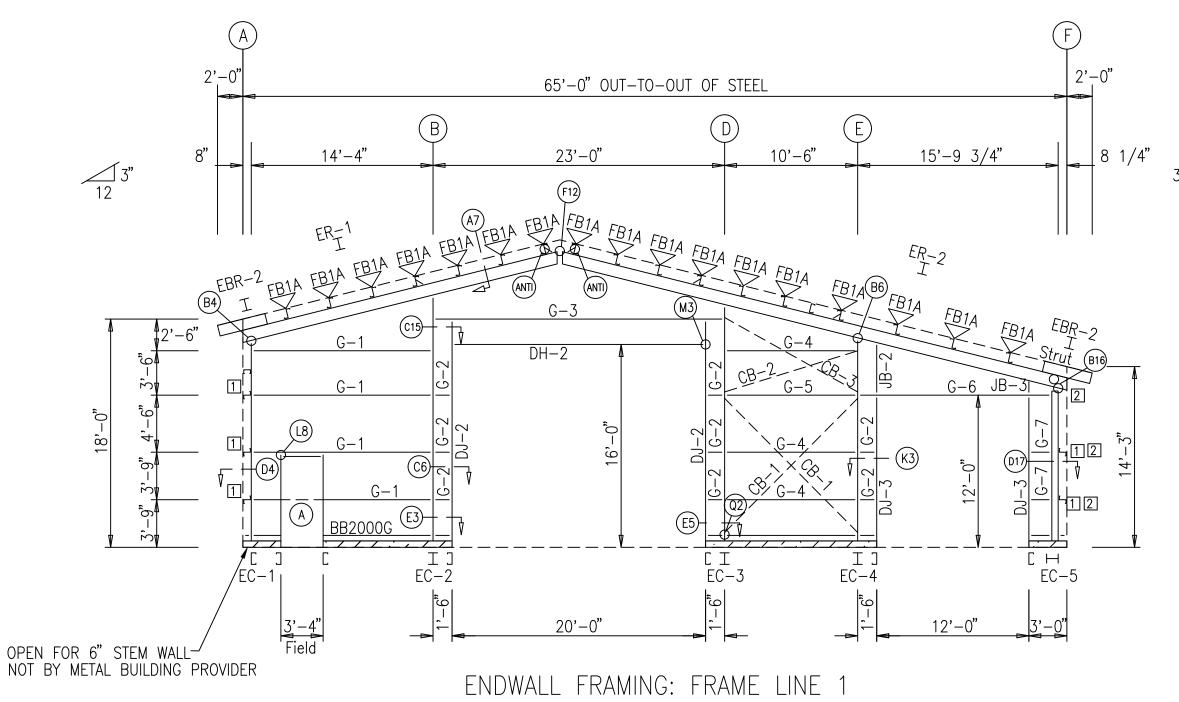
CUSTOMER LOCATION:
HAYDEN, CO 81639 SHEET DESCRIPTION: ROOF SHEETING PLAN CUSTOMER:

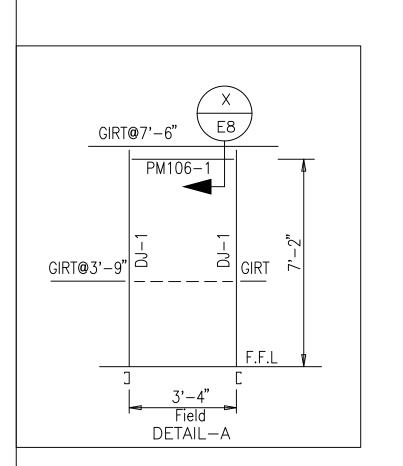
THUNDERSTRUCK / C&B HOLDINGS
PROJECT REFERENCE:

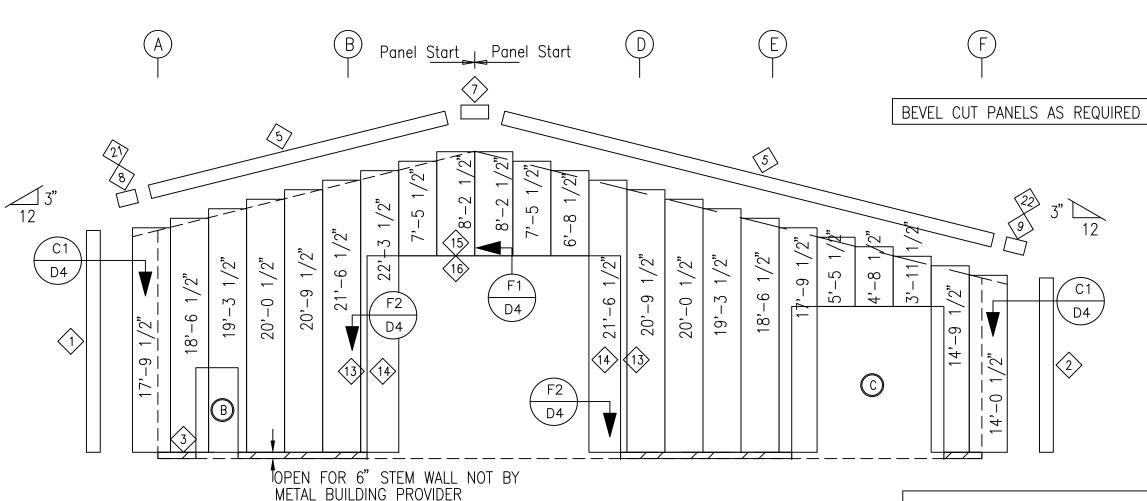


Detail at Rake Angle

	ABLE – THIS LINE –1	WALL ONLY
♦ID	PART	LENGTH
1 2 3 5 7 8	CT-102 CT-102 MT-103 RT-101 SPB SPCB-3L	18'-4" 14'-7" 20'-3" 15'-3"
9 10 12 13 14 15 16 17 18 19 20 21 22	SPCB-3R JT-101 HT-101 MT-116B JT-101 MT-116B HT-101 MT-116B JT-101 MT-116B HT-101 SF-3L SF-3R	7'-6" 3'-8" 16'-4" 15'-10" 20'-3" 20'-3" 12'-4" 11'-10" 12'-4"







5/8" 5/8" 5/8" 5/8" 5/8" A325 A325 A325 1 1/2" JB-2 & JB-3/ER-2 FLANGE BRACE TABLE FRAME LINE 1 ▽ID MARK LENGTH 2'-6 3/4" FB1A CONNECTION PLATES FRAME LINE 1 ID | MARK/PART 1 | SC-5 2 | Z-1

A325 A325

QUAN

BOLT TABLE FRAME LINE 1

LOCATION

ER-1/ER-2
EC-1/ER-1
Int_Column/Raf
EC-5/ER-2

\ D4

D4

DETAIL-C

<u>DETAIL-B</u>

D4

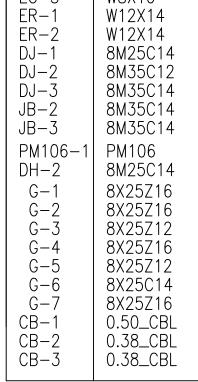
D4

D4

MEMBER 7	ΓABLE
FRAME LIN	NE 1
MARK	PART
EBR-2	W10X12
EC-1	8M35C14
EC-2	W8X10
EC-3	W8X10
EC-4	W8X10
EC-5	W8X10
1	l

LENGTH

1 1/2" 1 1/2"





GENERAL FRAMING NOTES

ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SSX - SMP Steel Gray

1. Angles are marked by their length in feet and inches. 2. Field cut or lap angles as required to fit.

3. Flange braces are marked by their length in decimal inches.

4. Outside flange of girt turns down unless noted.

5. Endwall girts and eave struts do not lap. 6. Field cut and self-tap girts at walk doors.

7. Field slot girts for brace rods or cables. 8. Field locate windows and walk doors.

9. Field weld all splices at 14 gauge valley gutters.

10. Field bolt AK400 base clip to endwall columns:

(2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd

(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd

11. Locate top of roof framed openings flush with the pan of the roof panel. 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.

13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection

manual or standard pull out for screw-down type roof for additional installation instructions. 14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For

						r
	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET
	P1	06.19.23	FOR CONSTRUCTION PERMIT	PND	PNC	CUSTO
	P2	07.20.23	REV.FOR CONSTRUCTION PERMIT	PND	PNC	DD0.15
NG	0	08.02.23	FOR ERECTOR INSTALLATION	PND	PNC	PROJE
O 80127						JOBSIT
						<u> </u>

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		•				-		-	
CHK	SHEET DESCR					BLDG SIZE:			
	ENI	DWALL FRAME	& SHEETING ELE	Vation		65' – 0"	x 70'–0" x 18'–	0"/14'–3"	
PNC	CUSTOMER:					CUST	TOMER LOCATION:		
PNC	THU	UNDERSTRUCK	(/ C&B HOLDIN	IGS		HA	YDEN, CO 81639		
PNC	PROJECT REFERENCE:								
PNC	THUNDERSTRUCK / C&B HOLDINGS								
	JOBSITE LOCATION: JOBSITE COUNTY:								
	HAYDEN, CO 81639 ROUTT								
	DWN:	CHK:	DATE:	ENG:		JOB NO:	DWG NO:	ISSUE:	
	PND	PNC	08.02.23		KMO	11217-3200	5 E3	0	

GENERAL SHEETING & TRIM NOTES

 Refer to erection drawings for rake angle locations.
 Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown. 3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.

4. Roof stitch screws are located at each member with two between members (20" max. spacing).

5. Wall stitch screws are located at each member with one between members (20" max. spacing). 6. Skylight stitch screws are at 6" o.c. 7. Start endwall panels at centerline of bldg. unless noted.

8. Gutter, rake, & eave trim lap 2". All other trims lap 1". 9. Field cut or lap panels as required to fit.

10. Field cut or top pariets as required to the.

10. Field cut panels for all openings.

11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.

12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".

13. Corner and/or peak boxes are not furnished with special rate or gutter profiles. Field miter as req'd.

14. Downspout straps are located 6" from base and at every girt location.

15. Hot-rolled or built-up members must be pre-drilled before attaching members screws. 16. Metal shavings must be swept from the roof each day to avoid surface rusting.

17. Windows and louvers must be installed before sheeting the walls.

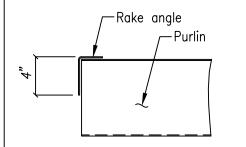
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

FOR APPROVAL:
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Erector Installation" can be considered complete. FOR CONSTRUCTION PERMIT:

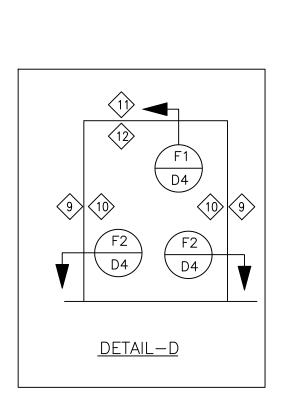
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Final drawings for construction

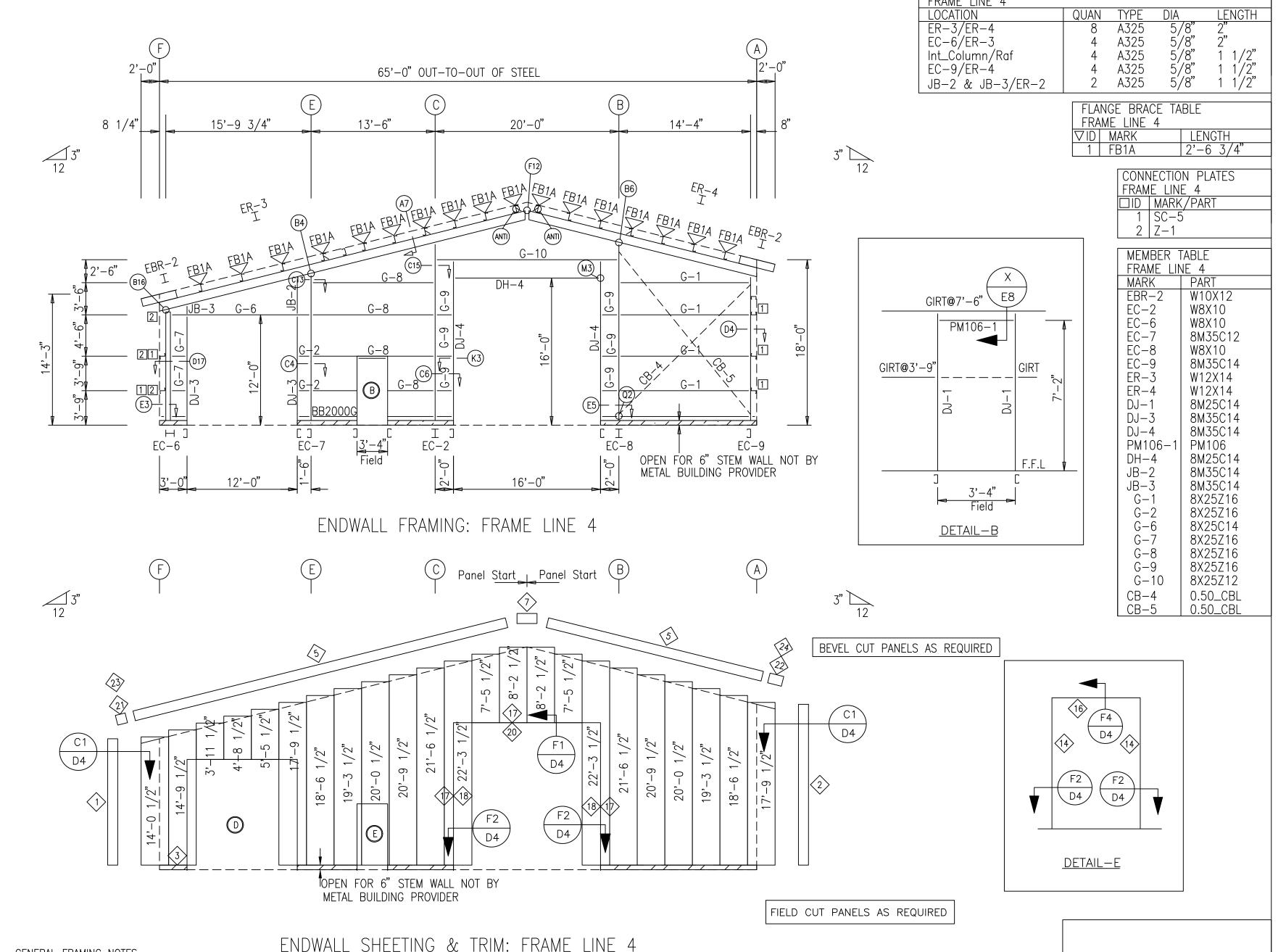
METAL**BUILDIN** 7651 SHAFFER PARKWAY LITTLETON



Detail at Rake Angle

	TABLE - THIS	WALL ONLY
FRAME	LINE – 4	
◇ID	PART	LENGTH
1 2	CT-102 CT-102	14'-7" 18'-4"
3	MT-103	20'-3"
2 3 5	RT-101	15'-3"
7 9	SPB	
	MT-116B	12'-4"
10	JT-101	11'-10"
11	MT-116B	12'-4"
12 14	HT-101 JT-101	12'-4" 7'-6"
16	HT-101	3'-8"
17	MT-116B	16'-4"
18	JT-101	15'-10"
20	HT-101	16'-4"
21	SPCB-3L	
22	SPCB-3R	
23	SF-3L	
24	SF-3R	





GENERAL SHEETING & TRIM NOTES

- Refer to erection drawings for rake angle locations.
 Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
- 3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members. 4. Roof stitch screws are located at each member with two between members (20" max. spacing).
- 5. Wall stitch screws are located at each member with one between members (20" max. spacing). 6. Skylight stitch screws are at 6" o.c.
- 7. Start endwall panels at centerline of bldg. unless noted.
- 8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
- 9. Field cut or lap panels as required to fit. 10. Field cut panels for all openings.
- 11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.

manual or standard pull out for screw-down type roof for additional installation instructions.

- 12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
 13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
- 14. Downspout straps are located 6" from base and at every girt location. 15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- 16. Metal shavings must be swept from the roof each day to avoid surface rusting.
- 17. Windows and louvers must be installed before sheeting the walls. 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection

GENERAL FRAMING NOTES

- 1. Angles are marked by their length in feet and inches.
- 2. Field cut or lap angles as required to fit. 3. Flange braces are marked by their length in decimal inches.
- 4. Outside flange of girt turns down unless noted. 5. Endwall girts and eave struts do not lap.
- 6. Field cut and self—tap girts at walk doors.
- 7. Field slot girts for brace rods or cables. 8. Field locate windows and walk doors.
- 9. Field weld all splices at 14 gauge valley gutters.
- 10. Field bolt AK400 base clip to endwall columns:
 (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 reg'd
 (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 reg'd
- 11. Locate top of roof framed openings flush with the pan of the roof panel. 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
- 13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
- 14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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METAL**BUILDIN** | FOR CONSTRUCTION PERMIT:

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| FOR ERECTOR INSTALLATION: Final drawings for construction. 7651 SHAFFER PARKWAY LITTLETON,

PANELS: 26 Ga. SSX - SMP Steel Gray

	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET D
	P1	06.19.23	FOR CONSTRUCTION PERMIT	PND	PNC	CUSTOME
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IG	0	08.02.23	FOR ERECTOR INSTALLATION	PND	PNC	PROJECT
CORP. 80127						JOBSITE
						i

Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc. and excludes part such as doors, windows, foundation design, and erection of the building. BLDG SIZE: ENDWALL FRAME & SHEETING ELEVATION 65'-0" x 70'-0" x 18'-0"/14'-3" CUSTOMER LOCATION:

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel

JOB NO:

BOLT TABLE FRAME LINE 4

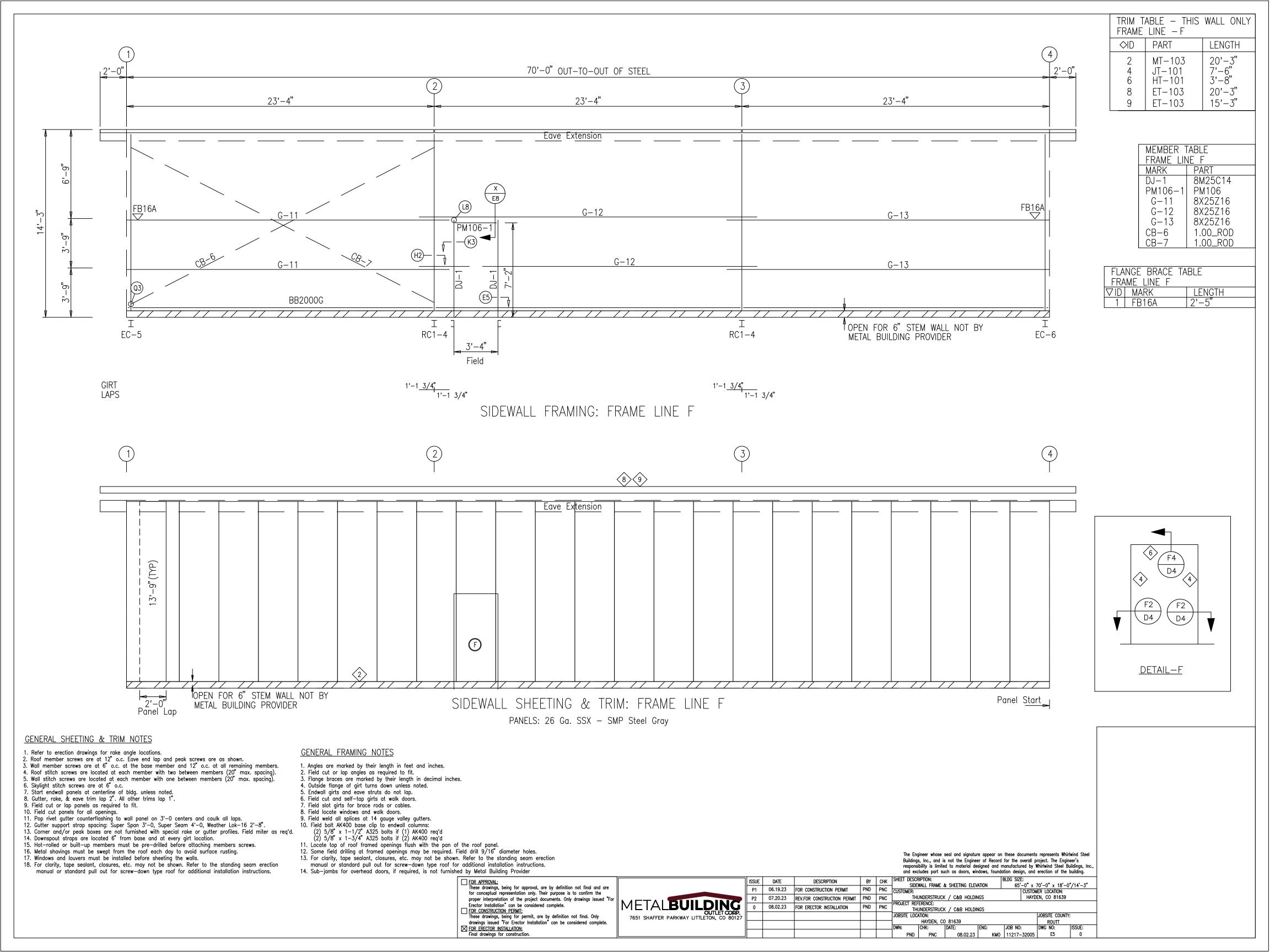
QUAN

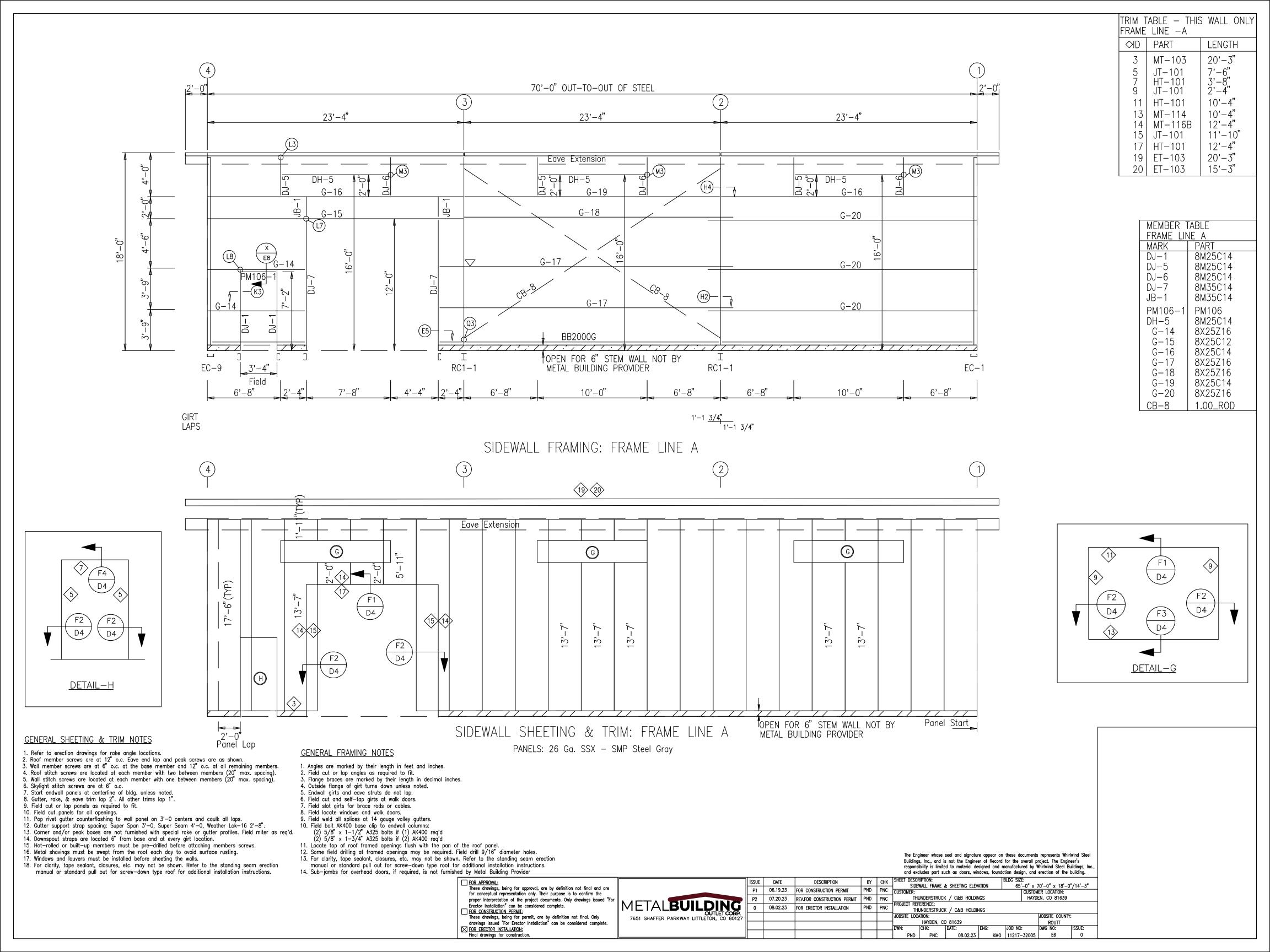
A325 A325

LENGTH

THUNDERSTRUCK / C&B HOLDINGS T REFERENCE: THUNDERSTRUCK / C&B HOLDINGS JOBSITE COUNTY LOCATION: HAYDEN, CO 81639 ROUTT DWG NO:

PND PNC 08.02.23 KMO 11217-32005



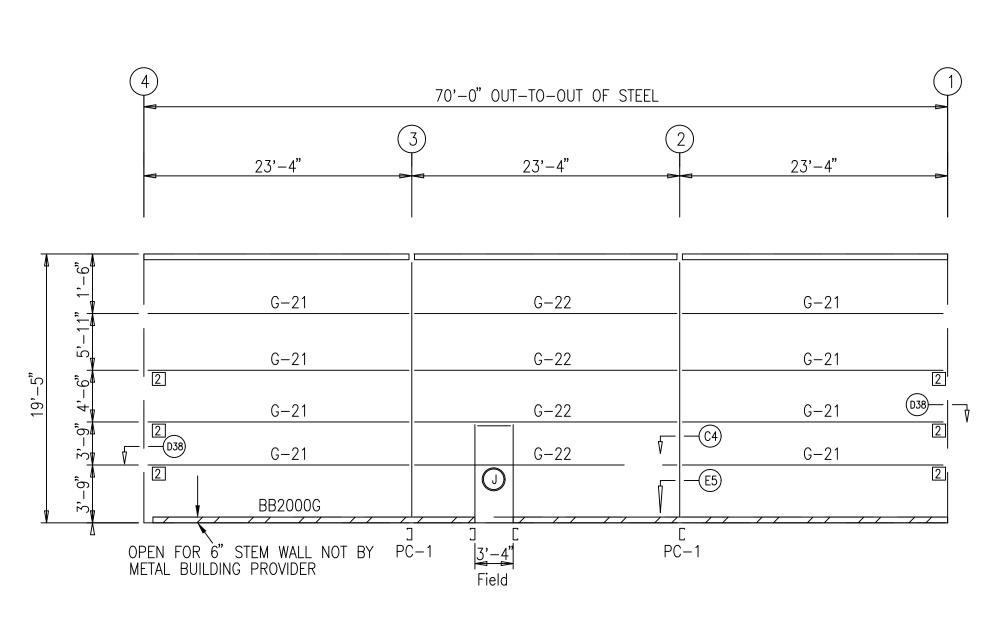


BOLT TABLE PARTITION 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
PC-1/RAFTER	2	A325	5/8"	1 1/2"

MEMBER 1	TABLE
PARTITION	1
MARK	PART
PC-1	8M35C12
DJ-1	8M25C14
PM106-1	PM106
G-21	8X25Z16
G-22	8X25Z16

TRIM PARTIT		IS WALL ONLY		
1 / ((\ 1 1 1				
◇ID	PART	LENGTH		
2	JT-101	7'-6"		
4	HT-101	3'-8"		
5	MT-103	20'-3"		
6	SF-10	20'-3"		
7	MT-106C	15'-3 <u>"</u>		
8	MT-106C	20'-3"		

CONN	IECTION PLATES					
PARTITION 1						
	MARK/PART					
2	AK244					



GIRT@7'-6" (E8)

PM106-1

3'-4" Field DETAIL-J

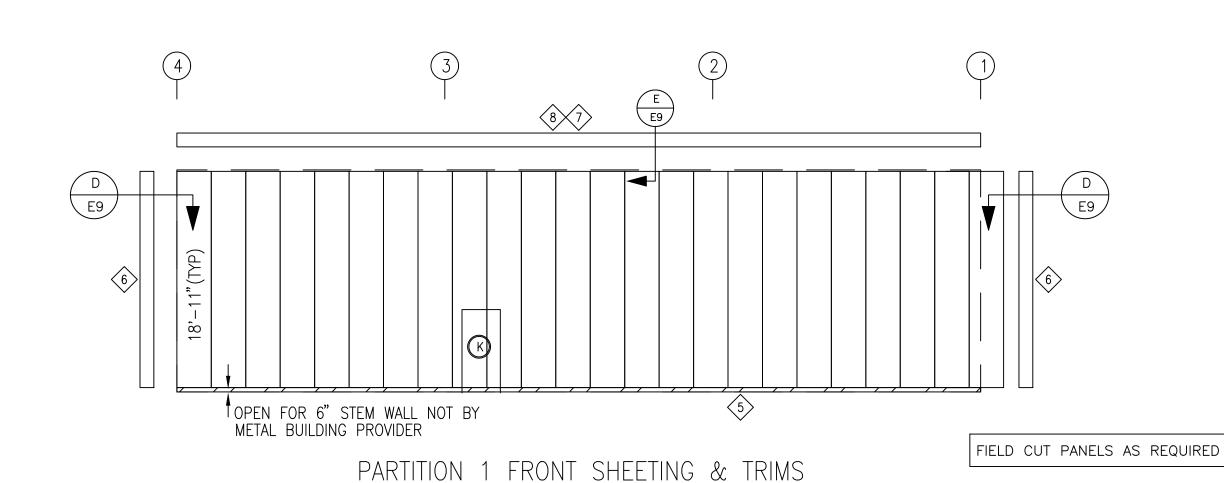
<u>DETAIL-K</u>

GIRT@3'-9"

GIRT

F.F.L

PARTITION 1 FRAMING

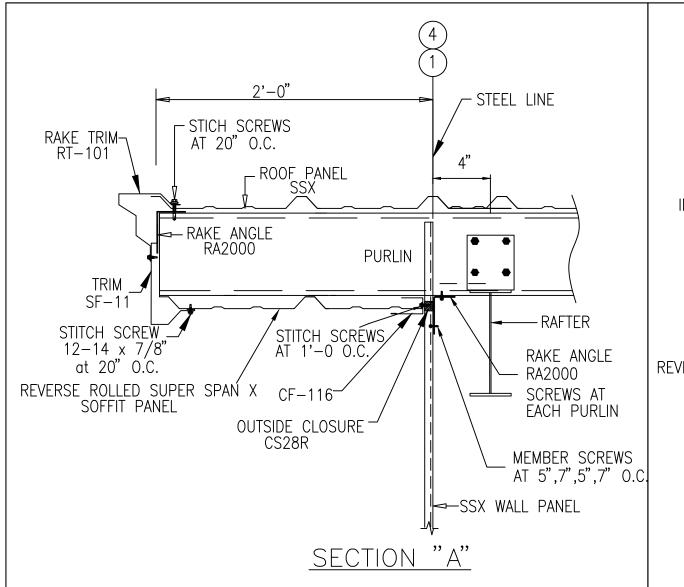


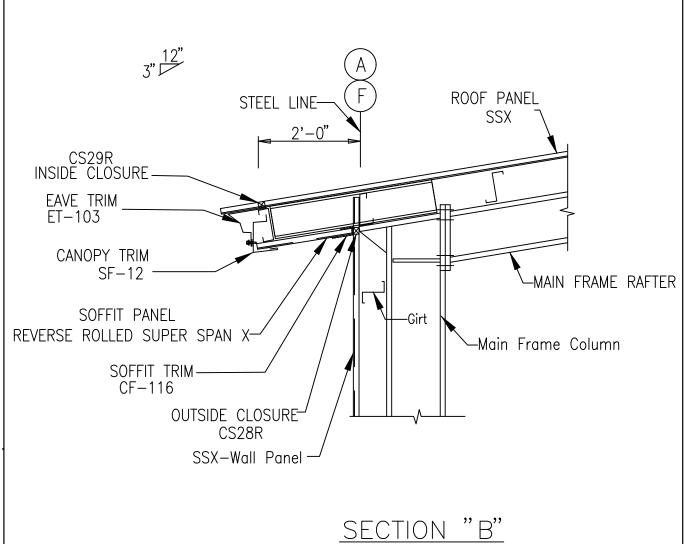
PANELS: 26 Ga. SSX — SMP Steel Gray

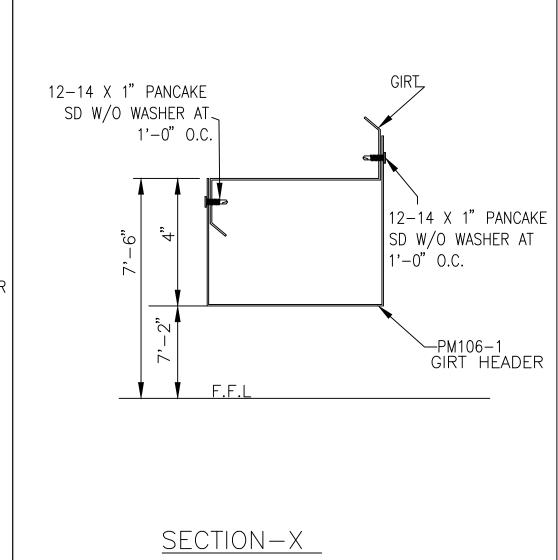
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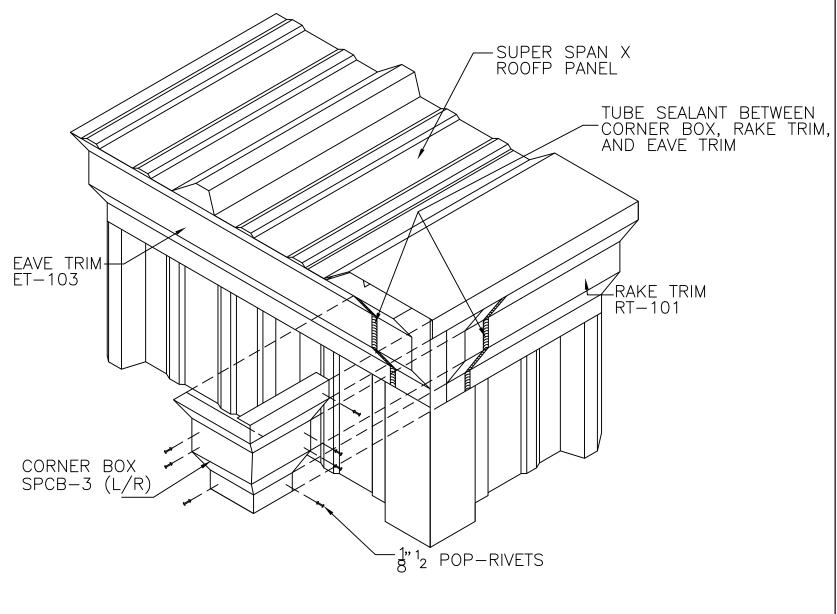
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| FOR ERECTOR INSTALLATION: Final drawings for construction.

ISSUE DATE DESCRIPTION BY CHK
P1 06.19.23 FOR CONSTRUCTION PERMIT PND PNC
P2 07.20.23 REV.FOR CONSTRUCTION PERMIT PND PNC
0 08.02.23 FOR ERECTOR INSTALLATION PND PNC
T651 SHAFFER PARKWAY LITTLETON, CO 80127

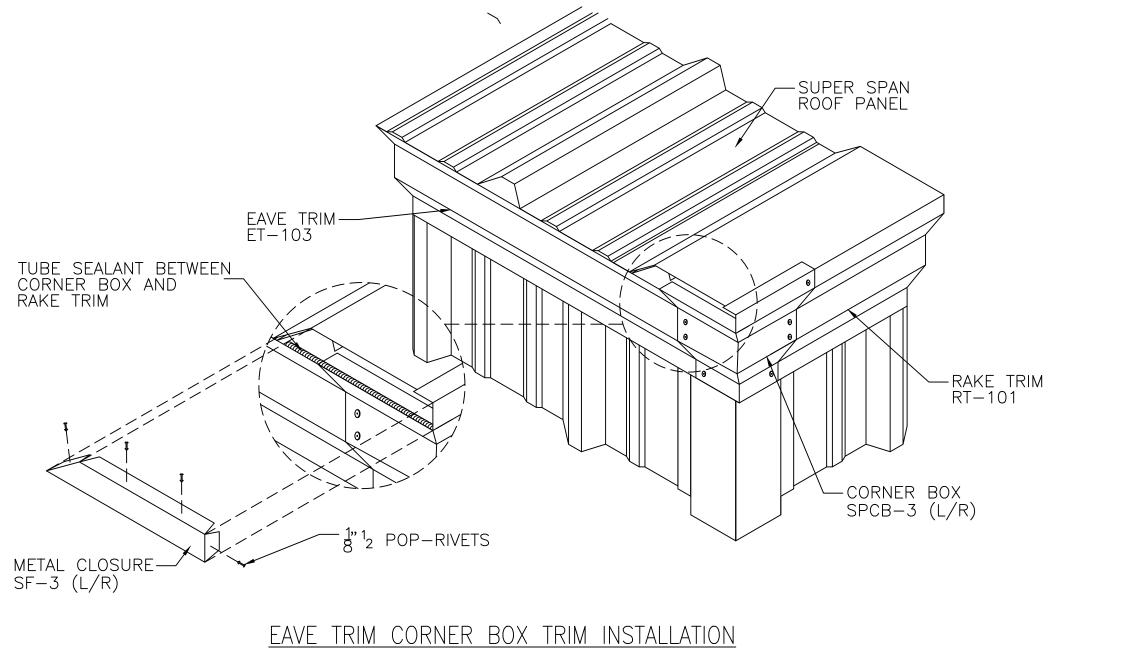








EAVE TRIM CORNER BOX TRIM INSTALLATION



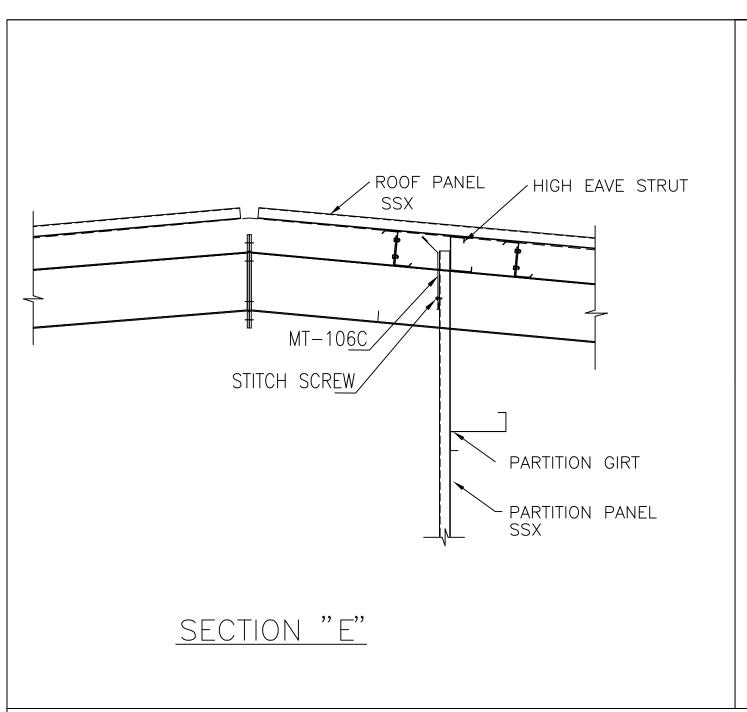
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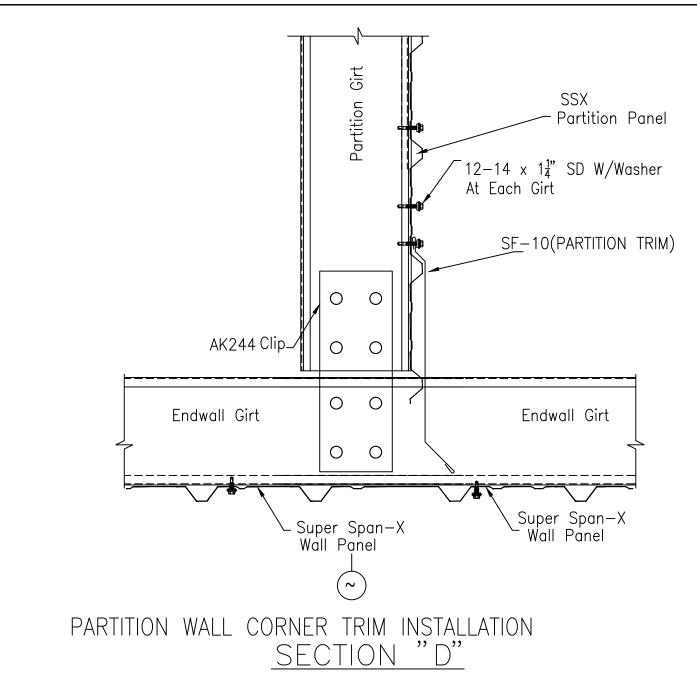
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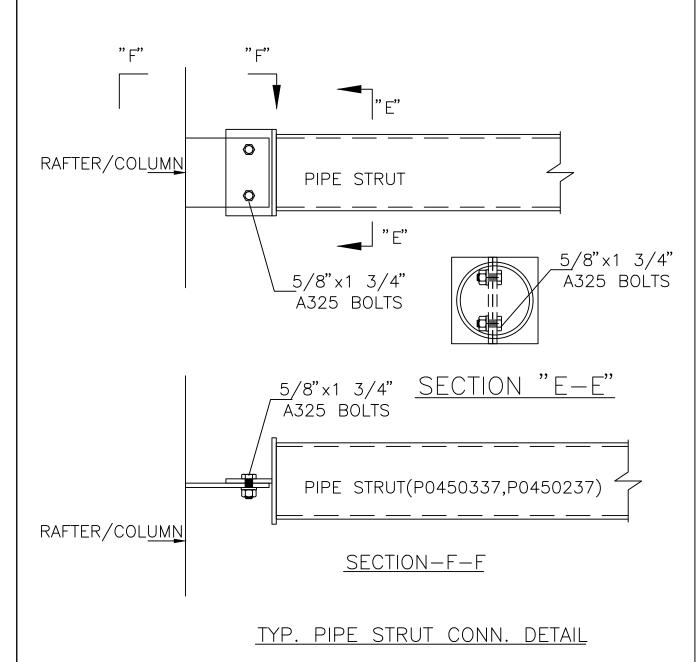
METALBUILDING OUTLET CORP. 7651 SHAFFER PARKWAY LITTLETON, CO 80127

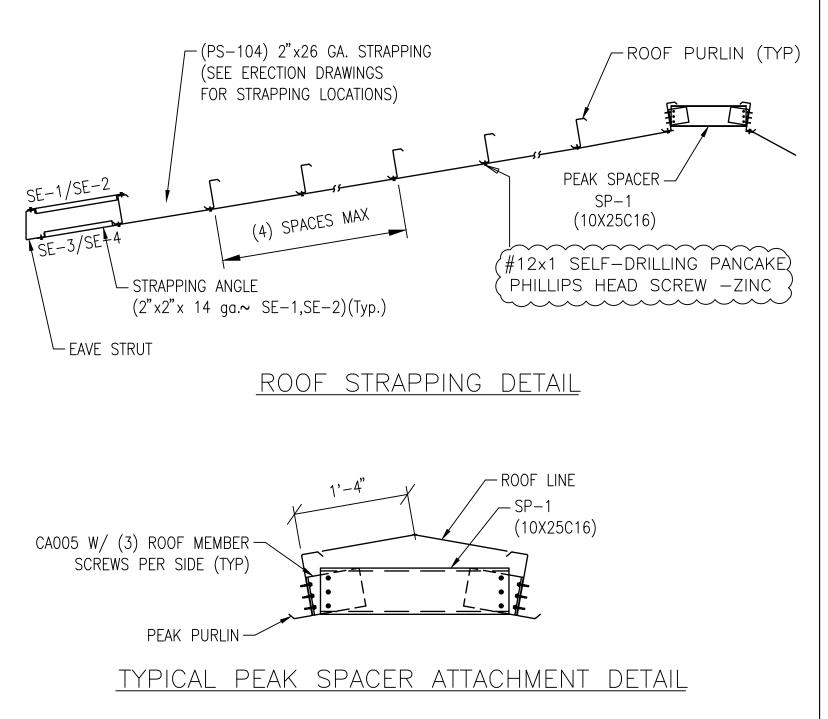
P. 127	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEE
	P1	06.19.23	FOR CONSTRUCTION PERMIT	PND	PNC	CUST
	P2	07.20.23	REV.FOR CONSTRUCTION PERMIT	PND	PNC	
	0	08.02.23	FOR ERECTOR INSTALLATION	PND	PNC	PRO.
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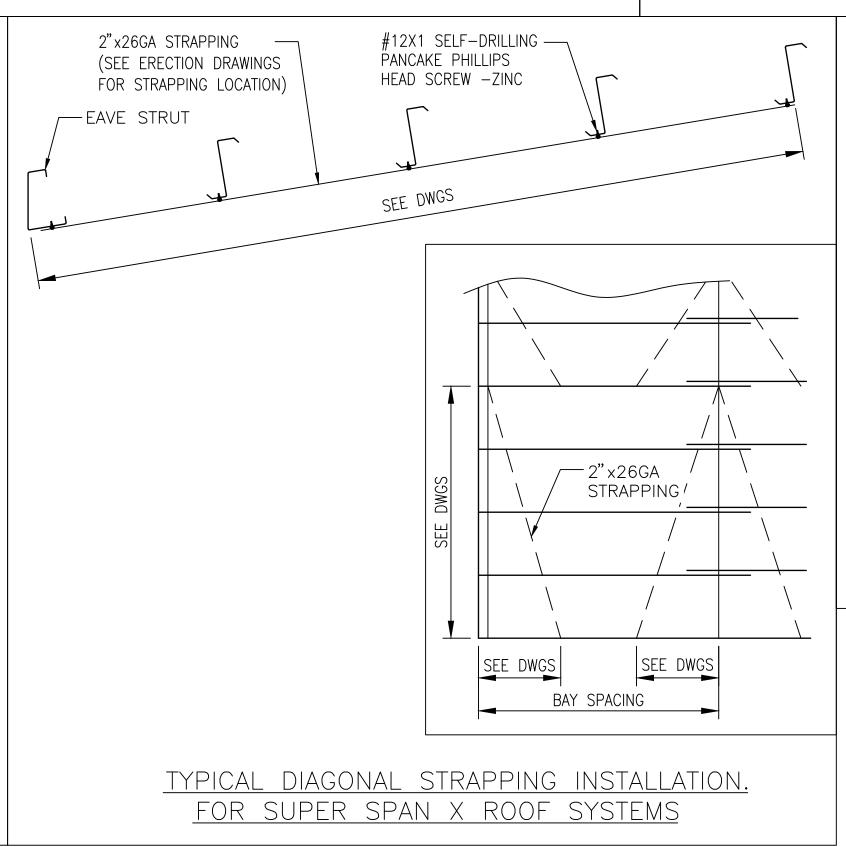
	and excludes part such as doors, windows, foundation design, and erection of the building.								
	SHEET DESCR	RIPTION:		BLDG SI	BLDG SIZE:				
_	ł	BUILDING	SECTIONS	6:	65'-0" x 70'-0" x 18'-0"/14'-3"				
	CUSTOMER:				CUSTOMER LOCATION:				
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_	PROJECT REFERENCE:								
	THUNDERSTRUCK / C&B HOLDINGS								
	JOBSITE LOCA	ATION:				JOBSITE COUN	ITY:		
		HAYDEN, CO	81639			ROUTT			
_	DWN:	CHK:	DATE:	ENG:	JOB N	0:	DWG NO:	ISSUE:	
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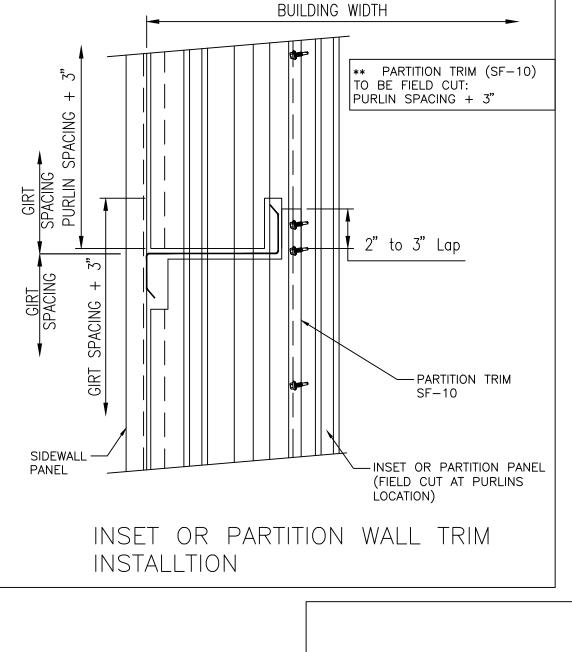












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FOR ERECTOR INSTALLATION:
Final drawings for construction

METALBUILDING OUTLET CORP. 7651 SHAFFER PARKWAY LITTLETON, CO 80127

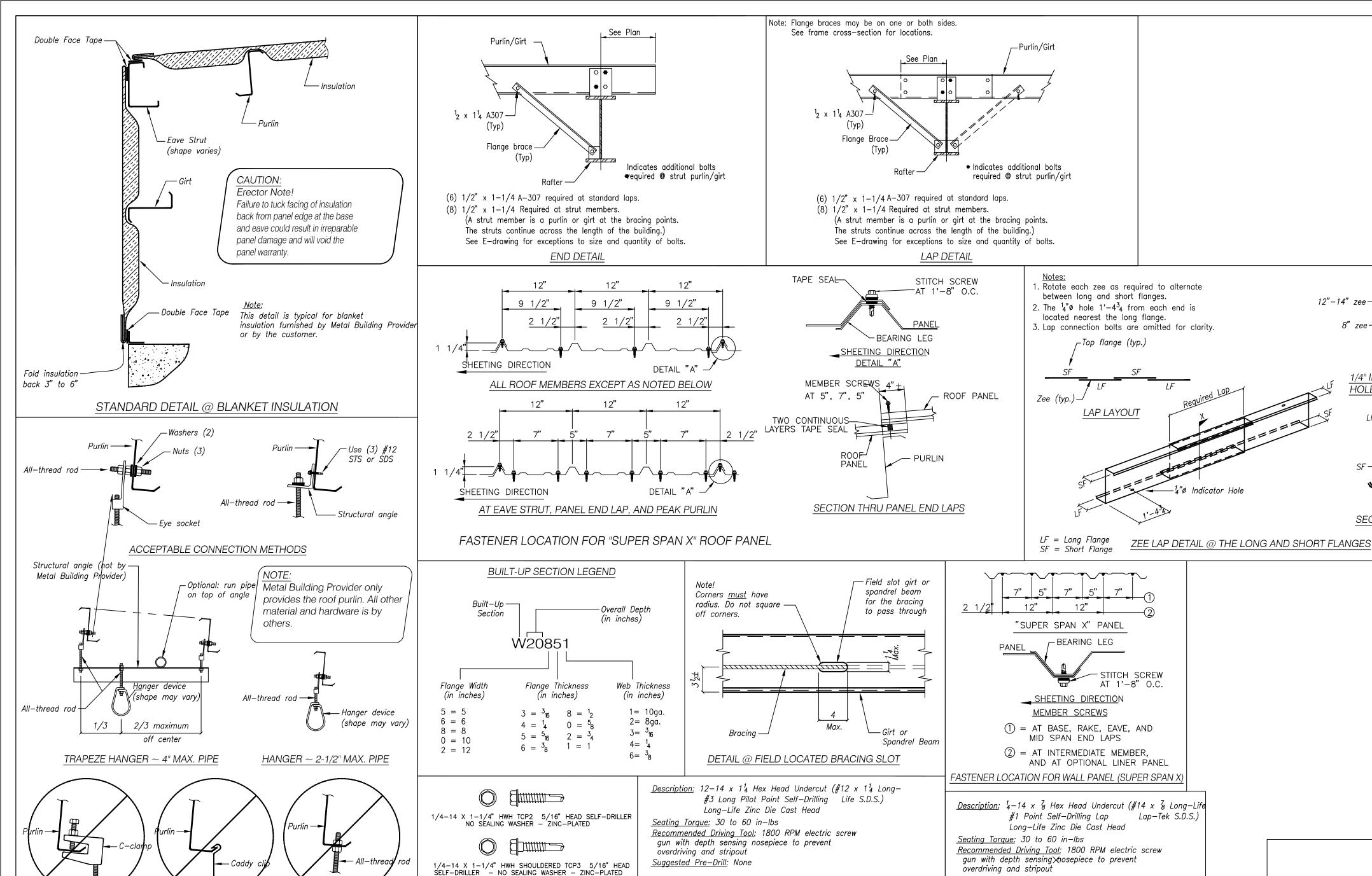
ISSUE DATE DESCRIPTION BY CHK P1 06.19.23 FOR CONSTRUCTION PERMIT PND PNC CUSTOMER: P2 07.20.23 REV.FOR CONSTRUCTION PERMIT PND PNC PND PNC 08.02.23 FOR ERECTOR INSTALLATION

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: CHK: DATE: ENG: JOB NO: DWG NO: PND PNC 08.02.23 KM0 11217-32005 E9

ROUTT DWG NO:

HAYDEN, CO 81639



(公)

Seating Torque: 30 - 60 in-lbs Recommended Driving Tool:

Flange C-Clamp is not Caddy clip attached to lip is Connection through the

ACCEPTABLE CONNECTIONS FOR ALL

COLLATERAL LOADS FOR HANGER ATTACHMENT

an acceptable connection not an acceptable connection flange is not acceptable

#12 x 1" PANCAKE HEAD SDS QUADREX DRIVE, ZINC-PLATED

STANDARD FASTENERS MISCELLANEOUS

FOR ERECTOR INSTALLATION:
Final drawings for construction

1800 RPM screw gun with depth sensing nosepiece to prevent overdriving and stripout

Suggested Pre-Drill: None Actual Size The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc.

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Actual Size

ISSUE DATE DESCRIPTION P1 06.19.23 FOR CONSTRUCTION PERMIT P2 07.20.23 REV.FOR CONSTRUCTION PERMIT PND PNC METALBUILDING PND PNC 08.02.23 FOR ERECTOR INSTALLATION 7651 SHAFFER PARKWAY LITTLETON, CO 8012

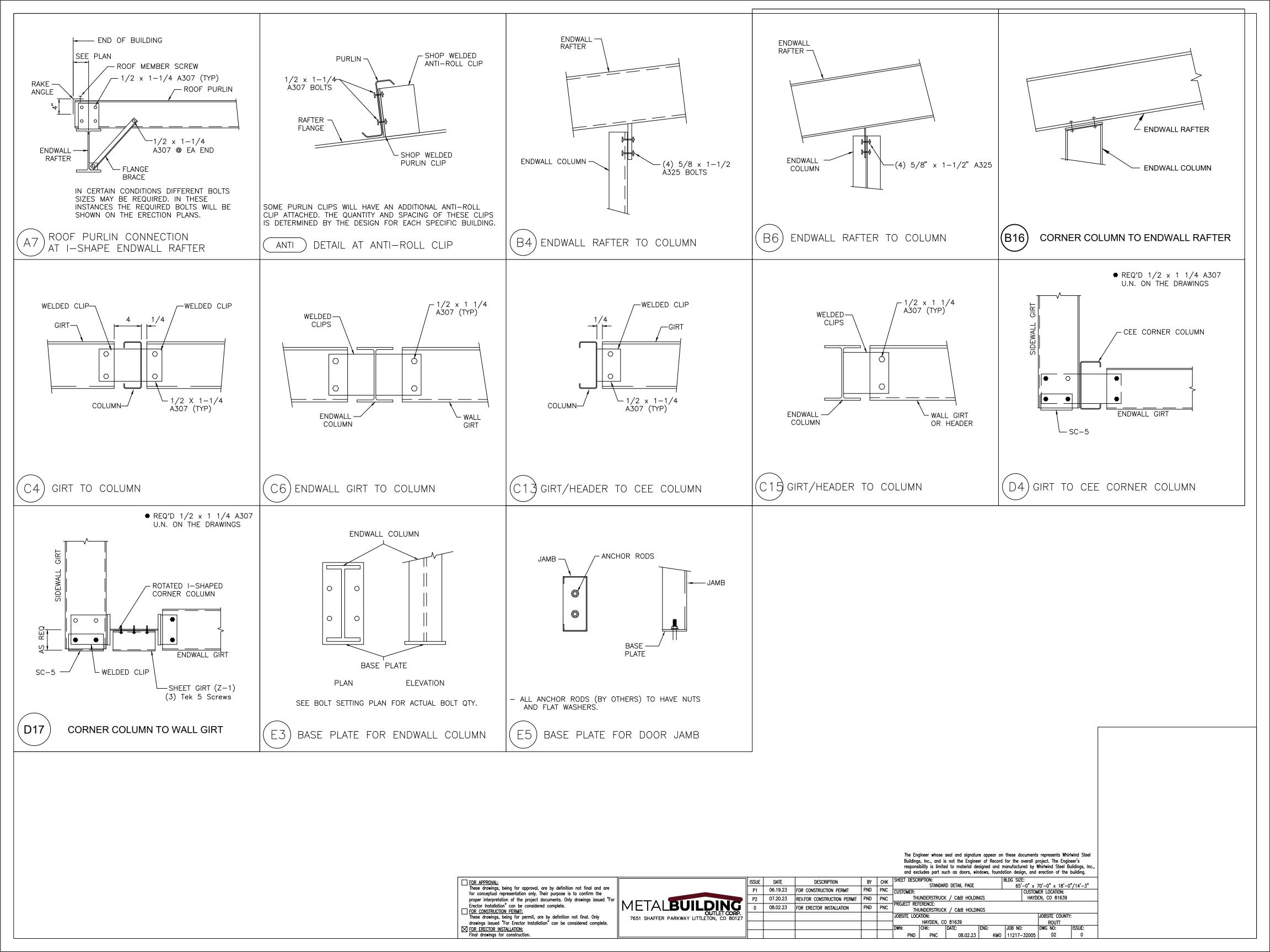
and excludes part such as doors, windows, foundation design, and erection of the building. SHEET DESCRIPTION: STANDARD DETAIL PAGE 65'-0" x 70'-0" x 18'-0"/14'-3" PND PNC CUSTOMER: THUNDERSTRUCK / C&B HOLDINGS PROJECT REFERENCE: THUNDERSTRUCK / C&B HOLDINGS JOBSITE LOCATION: HAYDEN, CO 81639 ROUTT DWG NO: JOB NO: PND PNC 08.02.23 KMO 11217-32005 D1

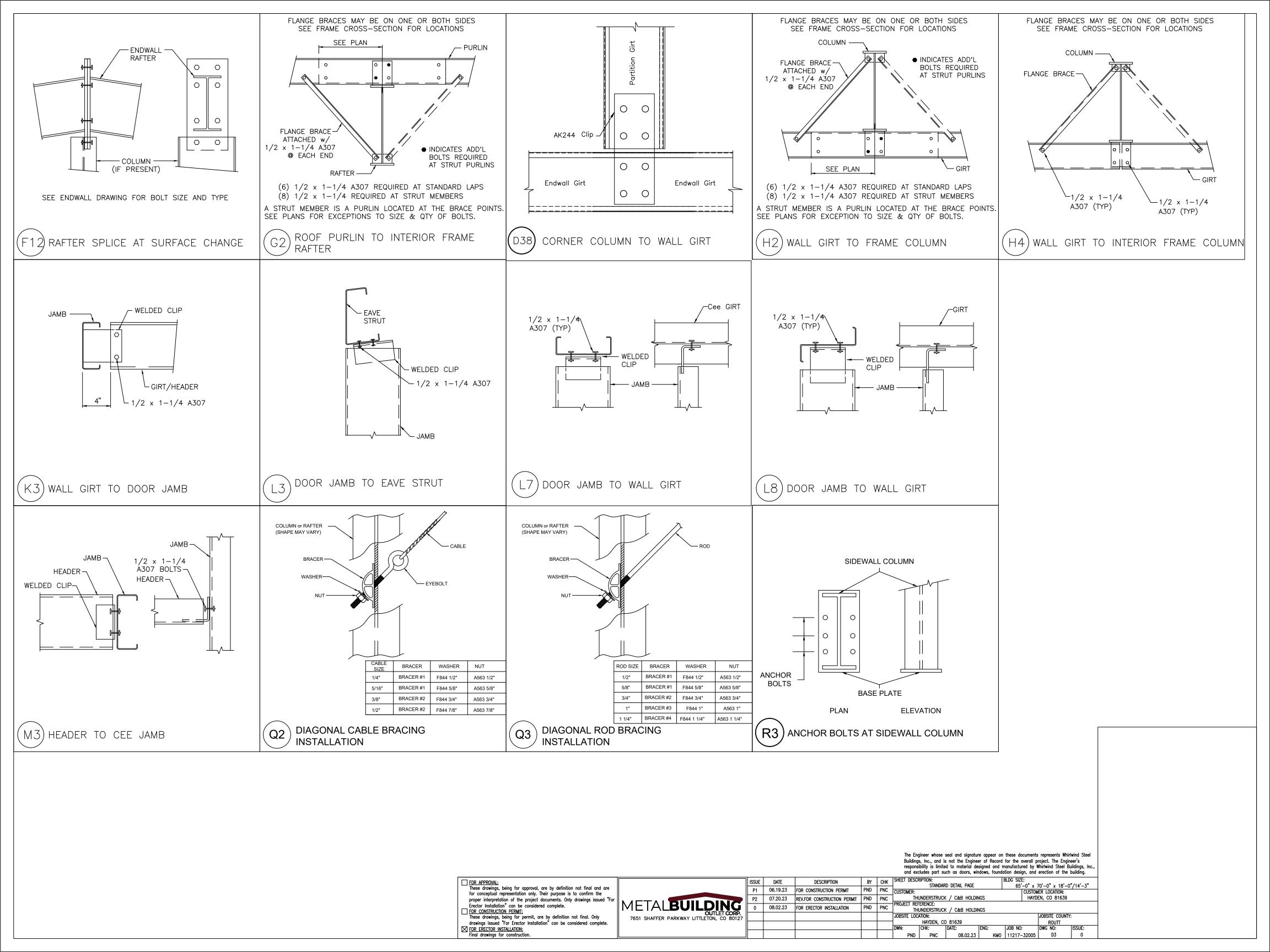
12"-14" zee

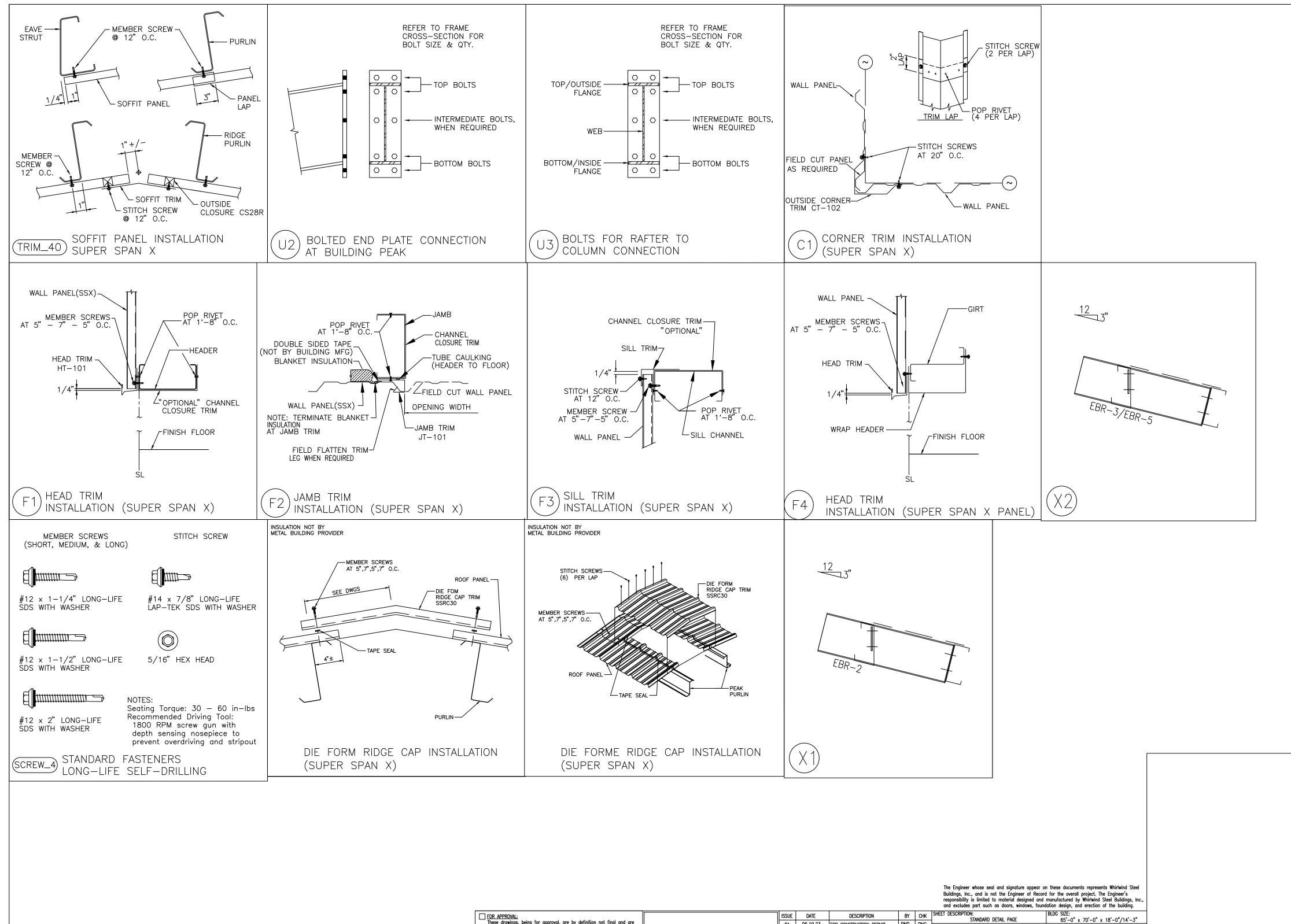
1/4" INDICATOR

SECTION "X"

HOLE LOCATION







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Final drawings for construction.



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P1	06.19.23	FOR CONSTRUCTION PERMIT	PND	PNC	CUSTOMER:	SIMIDAIN		CUSTOMER LOCATION:				
P2	07.20.23	REV.FOR CONSTRUCTION PERMIT	PND	PNC		JNDERSTRUCK	C / C&B HOLDIN	IGS	HAYI	EN, CO 81639		
0	08.02.23	FOR ERECTOR INSTALLATION	PND	PNC	PROJECT REFERENCE: THUNDERSTRUCK / C&B HOLDINGS							
					JOBSITE LOCA	JOBSITE COUNTY	:					
					HAYDEN, CO 81639					ROUTT		
					DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	IS	
					PND	PNC	08.02.23	кмо	11217-32005	D4		